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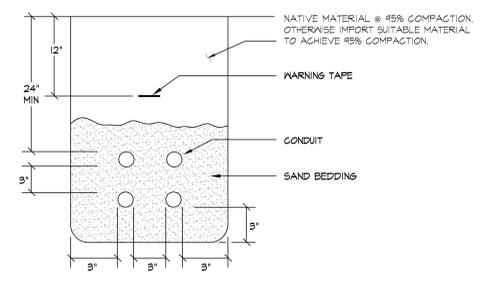
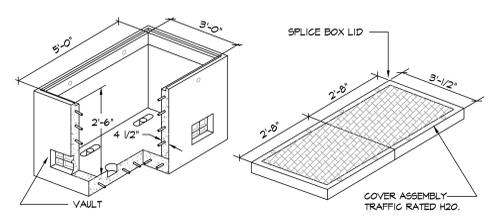
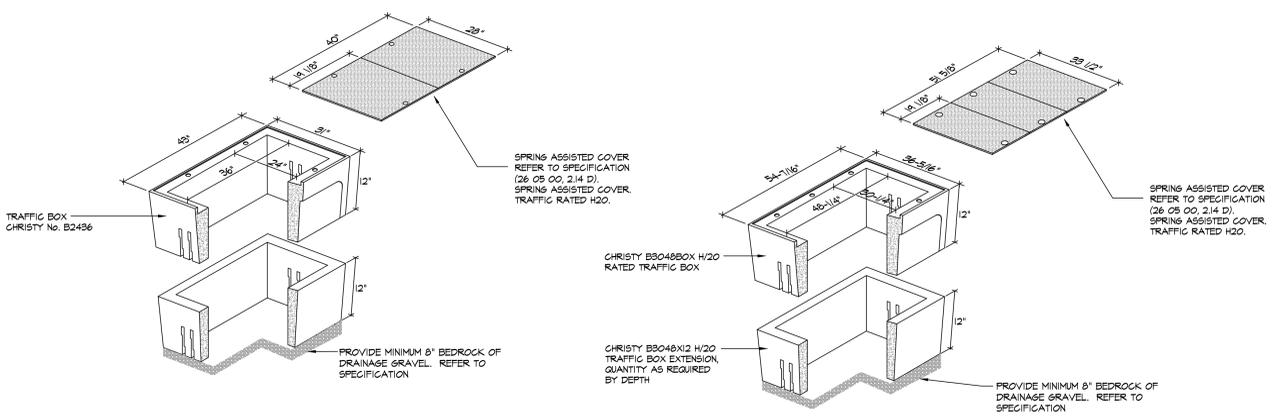
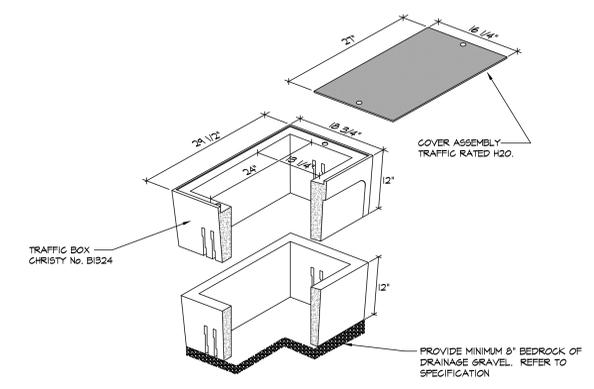
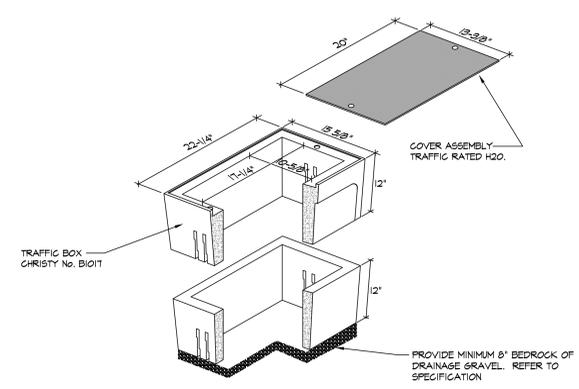
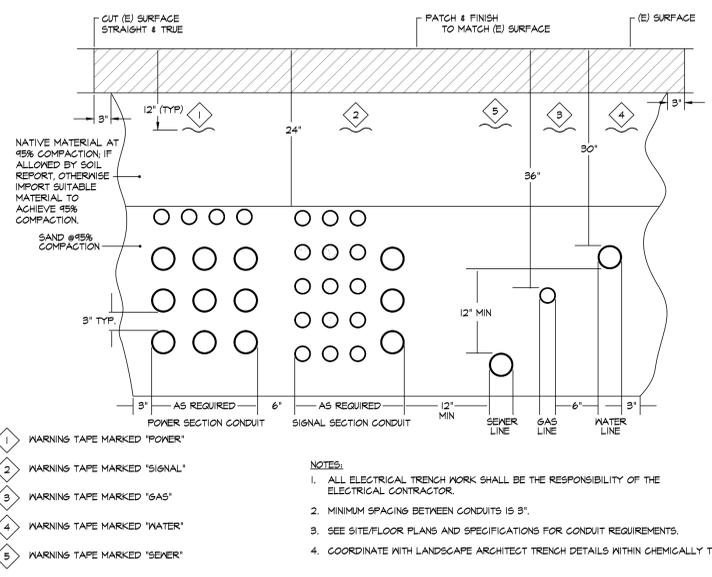
NO.	REVISIONS	DATE

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DATE ISSUED: 03/18/2024 SCALE: AS NOTED

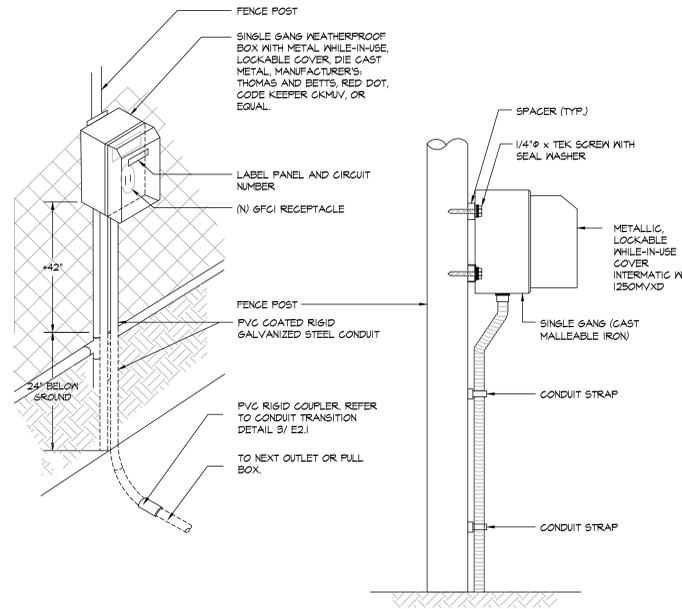
PROJ. NO.: 2309900

SHEET NO.: **E7.1** OF
ELECTRICAL DETAILS

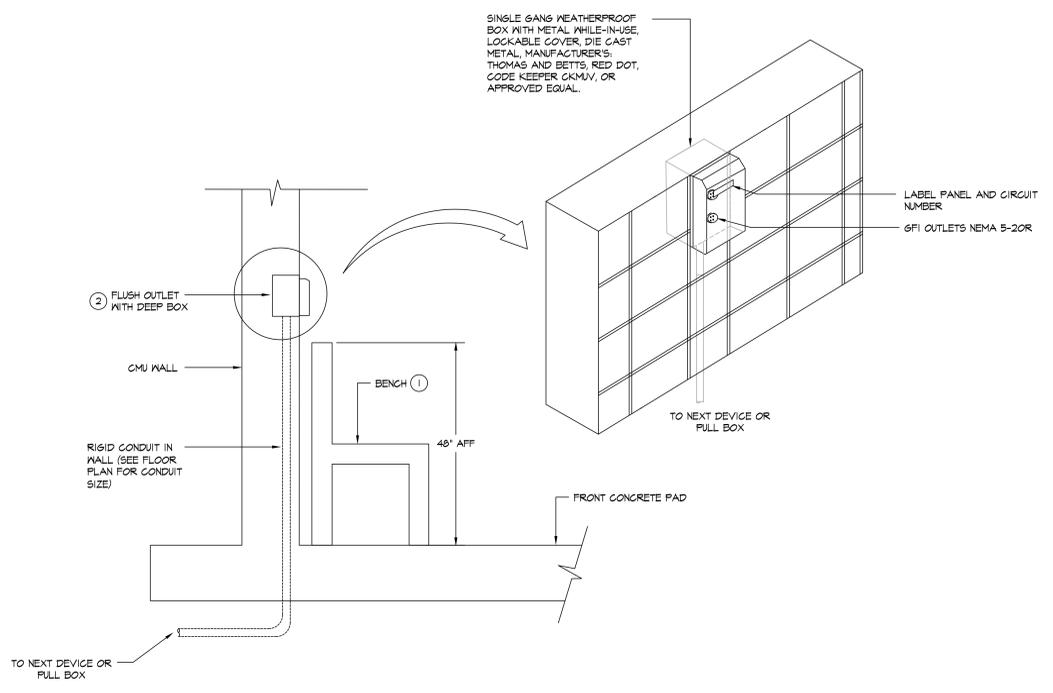


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1 RECEPTACLE MOUNTING
E7.2 NOT TO SCALE



NOTE:

- 1 CONTRACTOR TO COORDINATE WITH LANDSCAPE DRAWINGS TO FIND EXACT HEIGHT OF BENCH PRIOR TO ROUGH IN.
- 2 COORDINATE WITH DUGOUT CONTRACTOR (N) CMU WALL INSTALL BOXES AND CONDUIT CONCEALED IN WALL.

2 DUGOUT RECEPTACLE MOUNTING
E7.2 NOT TO SCALE

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J09 #0023095

KEY MAP

SHEET TITLE
ELECTRICAL DETAILS

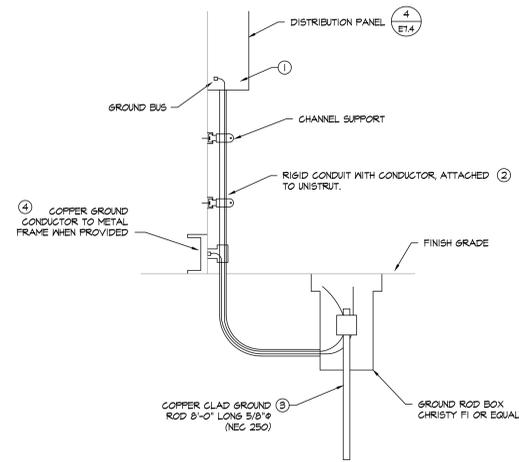
PROJECT NAME
**WEST CAMPUS
HIGH SCHOOL
BASEBALL & SOFTBALL
IMPROVEMENTS**

PROJECT ADDRESS
**5022 58TH STREET
SACRAMENTO, CA 95820**

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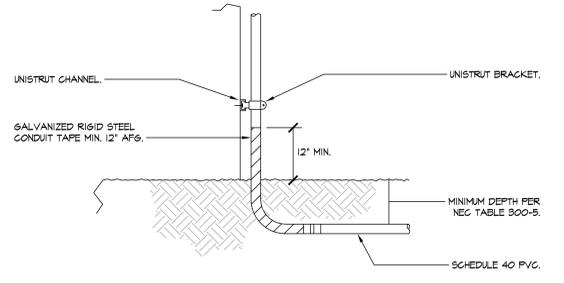
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PROJ. NO.: 2309900
SHEET NO.: **E7.2** OF
ELECTRICAL DETAILS



- ① SIZE OF CONDUCTORS SHALL COMPLY WITH NEC TABLE 250-66
- ② BOND SEPARATE CONDUCTORS FROM GROUND ROD TO ELECTRICAL PANEL AND TO METAL BUILDING FRAME (NEC 250-50). IN ADDITION TO DETAIL ABOVE, BOND THE ELECTRICAL GROUND TO NEAREST METALLIC COLD WATER PIPE (NEC 250-50)
- ③ CHECK RESISTANCE TO GROUND, IF RESISTANCE EXCEEDS 25 OHMS, INSTALL ADDITIONAL GROUND RODS AS REQUIRED (NEC 250-56)
- ④ ALL MODULES OF METAL FRAME BUILDINGS SHALL BE ELECTRICALLY BONDED TOGETHER. (BOLTING ONLY IS NOT ACCEPTABLE BONDING)

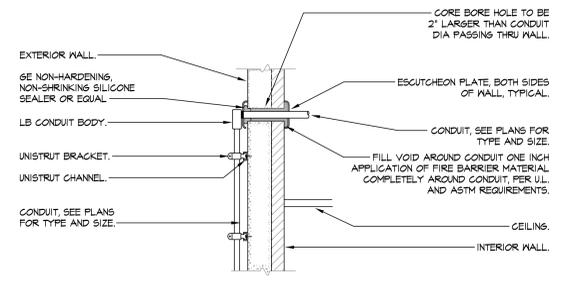
1 TYPICAL GROUND INSTALLATION
 E7.3 NOT TO SCALE

NOTE: GROUNDING TEST MUST BE BY INDEPENDENT LICENSED ELECTRICAL CONTRACTOR OR TESTING LABORATORY.



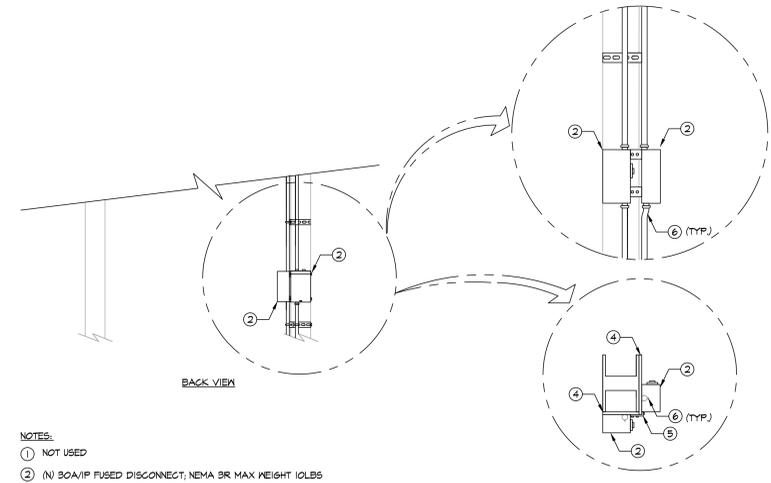
- NOTE:**
1. FOR WOOD STUD WALL: USE 3/8" LAG BOLT WITH MIN. 3/4" EMBEDMENT INTO STUDS. (ONE AT EACH END OF BRACKET)
 2. FOR CONCRETE WALL: 1/2" HILTI KWIK-BOLT T22 STAINLESS STEEL ANCHOR (ICC ESR-4266) WITH MINIMUM EMBEDMENT OF 3-5/8" IN 4" DEEP HOLE. 1/2" ANCHORS SHALL BE TORQUE-TESTED TO 40 FT-LBS, WHICH MUST BE ATTAINED WITHIN ONE-HALF TURN OF NUT AFTER FIRM CONTACT WITH ANCHOR WASHER. INSTALL ANCHOR PER CGO (HICAS), AND RECOMMENDATIONS IN MANUFACTURER'S ESR REPORT. ANCHOR INSTALLATIONS REQUIRE SPECIAL INSPECTION. (TYPICAL OF (4) PER SECTION)

2 UNDERGROUND CONDUIT RISER DETAIL
 E7.3 NOT TO SCALE



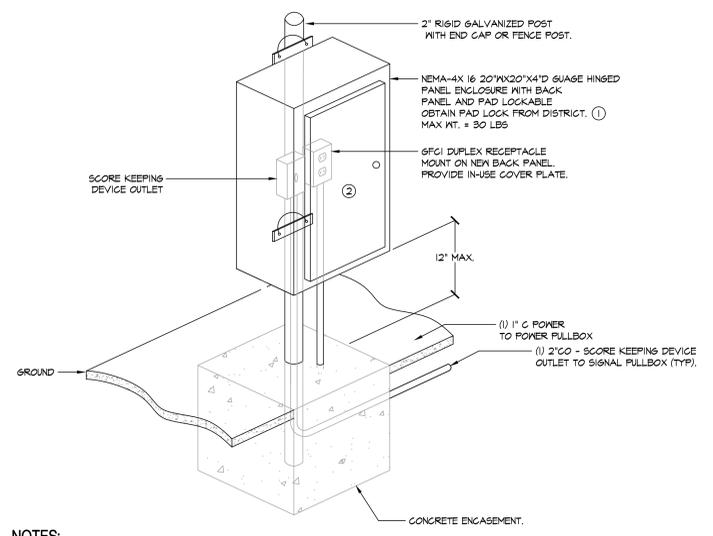
NOTE:
 PER U.L. FIRE RESISTANCE DIRECTORY SYSTEM M1002

3 CONDUIT WALL PENETRATION DETAIL
 E7.3 NOT TO SCALE



- NOTES:**
- ① NOT USED
 - ② (N) BOA/FP FUSED DISCONNECT, NEMA BR MAX WEIGHT 10LBS
 - ③ NOT USED
 - ④ (N) UNISTRUT MOUNTED ON H-BEAM
 - ⑤ (N) 3 HOLE FLUSH FITTING L - BRACKET
 - ⑥ (N) POWER CONDUIT

4 SCOREBOARD DISCONNECT PANEL MOUNTING
 E7.3 NOT TO SCALE



- NOTES:**
- ① PULL CAN SHALL BE PROVIDED WITH SEPARATORS TO DIVIDE POWER & SIGNAL. PROVIDE AS REQUIRED TO COMPLY WITH N.E.C. NEMA-4X PULL CAN SHALL BE APPROVED U.L. LISTED.
 - ② PROVIDE ENGRAVED NAME PLATE. IDENTIFY AS SCOREBOARD CONTROL. NAME PLATE SHALL BE PROVIDED PER SPECIFICATIONS.

5 METAL ENCLOSURE DETAIL
 E7.3 NOT TO SCALE

KEY MAP

SHEET TITLE

ELECTRICAL DETAILS

PROJECT NAME

**WEST CAMPUS
 HIGH SCHOOL
 BASEBALL & SOFTBALL
 IMPROVEMENTS**

PROJECT ADDRESS

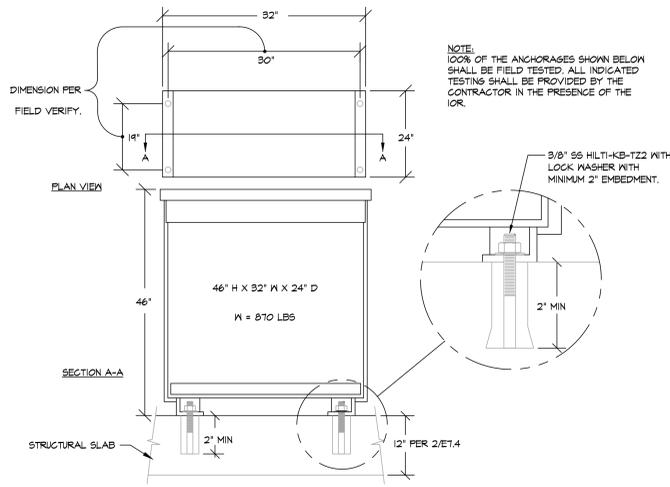
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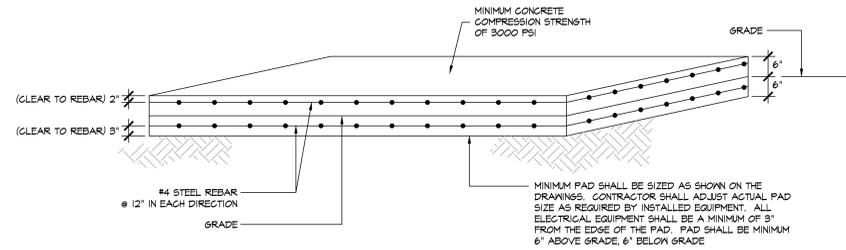
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DATE ISSUED 03/18/2024	SCALE
PROJ. NO. 2309900	
SHEET NO. E7.3	

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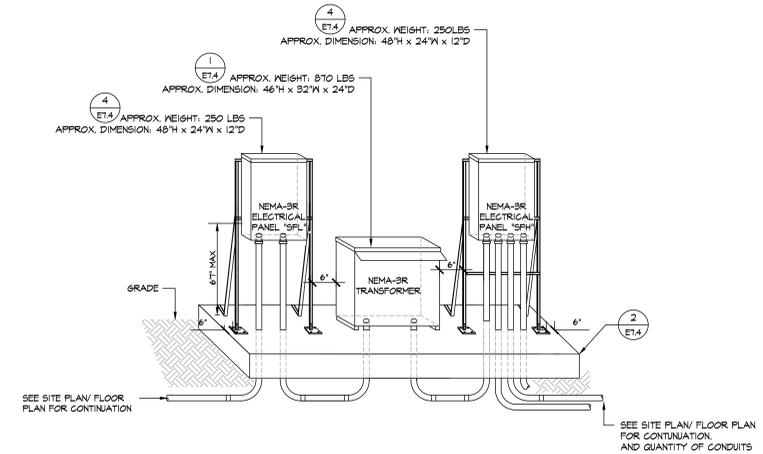
DISTRIBUTION TRANSFORMER INSTALLATION DETAIL

1
E7.4 NOT TO SCALE



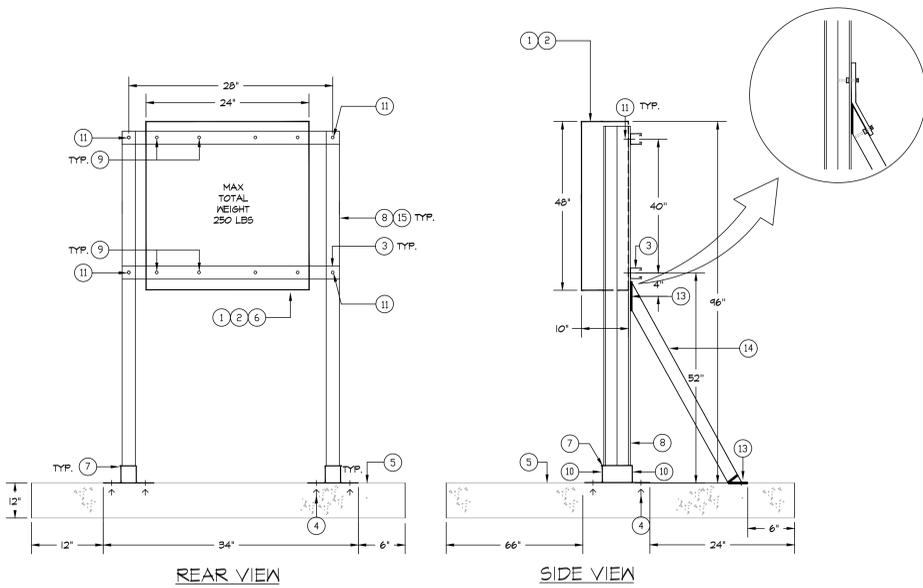
CONCRETE ELECTRICAL EQUIPMENT PAD

2
E7.4 NOT TO SCALE



NEMA 3R ELECTRICAL PANEL / TRANSFORMER / BREAKER ELEVATION DETAIL

3
E7.4 NOT TO SCALE



NOTES:

- 1 TYPE NEMA 4 LOCKABLE ENCLOSURE.
- 2 PANELBOARD.
- 3 PROVIDE UNISTRUT P1000 H5 CHANNEL THAT HAS A ROW OF ROUND HOLES MINIMUM 12 GA GALV STEEL.
- 4 PROVIDE STAINLESS STEEL 1/2" x 2-3/8" MINIMUM EMBEDMENT KNIK BOLT TZZ WEDGE ANCHOR (1CC-ES-ESR 4266), IN MINIMUM 2-5/8" DEEP HOLE. (4) ANCHOR BOLTS PER POST BASE.
- 5 CONCRETE PAD (SEE DETAIL 2/E7.4).
- 6 APPROX. DIMENSIONS OF ENCLOSURE 48"H x 24"W x 12"D.
- 7 PROVIDE UNISTRUT FLOOR SUPPORT P2073A POST BASE.
- 8 PROVIDE DOUBLE UNISTRUT P1001 MINIMUM 12 GA GALV STEEL.
- 9 PROVIDE HEX HEAD GAP SCREWS 3/8"x2" WITH HEX NUTS AND WASHERS. (4) GAP SCREWS ARE FOR ATTACHMENT OF PANEL TO REAR STRUTS. PROVIDE ONE GAP-SCREW NEAR EACH CORNER OF ENCLOSURE/PANEL.
- 10 PROVIDE (2) 1/2" GALV BOLTS FROM P2073A POST BASE INTO VERTICAL UNISTRUT P1001. PROVIDE EACH BOLT WITH P1001 NUT INSIDE STRUT. TYPICAL FOR BOTH P2073A POST BASE.
- 11 PROVIDE 1/2" GALV HEX HEAD MACHINE BOLT. BOLT SHALL BE ASTM A307, GRADE A TO BE FASTENERS AT EACH INTERSECTION. HORIZONTAL P1001 SINGLE UNISTRUT WILL CONNECT TO VERTICAL P1001 DOUBLE UNISTRUT OPEN SIDE WITH 1/2" DIAMETER BOLT AND NUT.
- 12 NOT USED.
- 13 UNISTRUT BRACKET. PROVIDE P1043 WITH 1/2" MB. 4 1/2" HILTI - KB - TZZ TO SLAB. MINIMUM EMBEDMENT KNIK BOLT TZZ WEDGE ANCHOR (1CC-ES-ESR 4266), IN MINIMUM 2-5/8" DEEP HOLE. LOCATE TOP OF BRACE AND BRACKET AS CLOSE TO HORIZONTAL STRUT AT BOTTOM OF PANELS AS POSSIBLE.
- 14 UNISTRUT BRACE, ONE P1000 BRACE AT EACH VERTICAL P1001, WITH MAXIMUM BRACE SLOP OF 2V:1H.
- 15 ALL UNISTRUT PARTS SHALL BE HOT DIPPED GALVANIZED.

GENERAL NOTES:

1. INSTALLATION OF ALL EXPANSION ANCHORS REQUIRES PERIODIC SPECIAL INSPECTION. ADDITIONALLY, TORQUE-TEST ALL 1/2 ANCHORS TO 40FT-LBS, WHICH MUST BE ATTENDED WITHIN ONE-HALF TURN OF NUT AFTER FIRM CONTACT WITH WASHER.
2. CONTRACTOR SHALL VERIFY EXACT SIZE OF NEMA-3R ENCLOSURE AND ADJUST UNISTRUT SUPPORT AS NEEDED TO FULLY SUPPORT ENCLOSURE.

ENCLOSED CIRCUIT BREAKER AND PANEL INSTALLATION ON UNISTRUT DETAIL

4
E7.4 SCALE: NOT TO SCALE

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JOB #023095

KEY MAP

SHEET TITLE

ELECTRICAL DETAILS

PROJECT NAME

WEST CAMPUS HIGH SCHOOL BASEBALL & SOFTBALL IMPROVEMENTS

PROJECT ADDRESS

5022 58TH STREET SACRAMENTO, CA 95820

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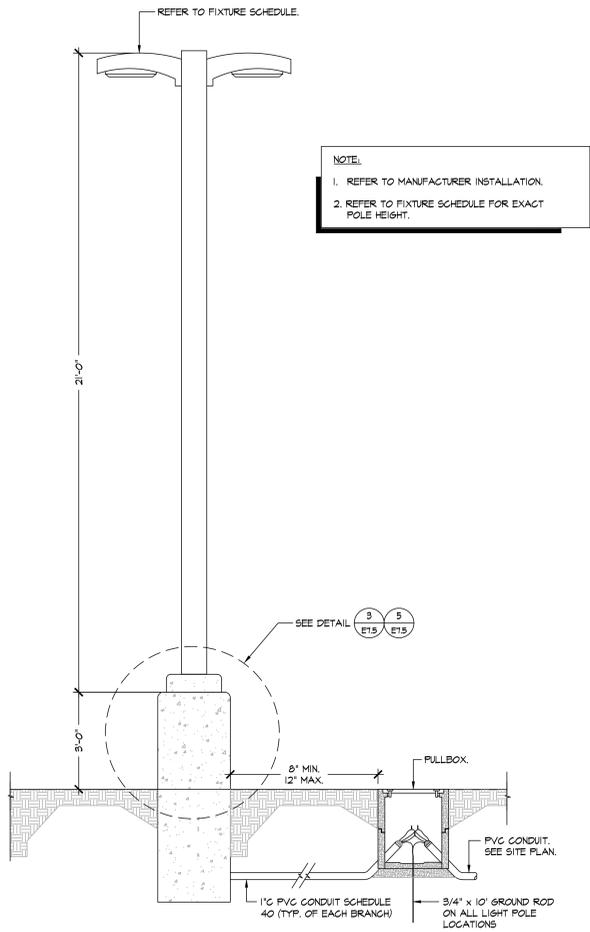
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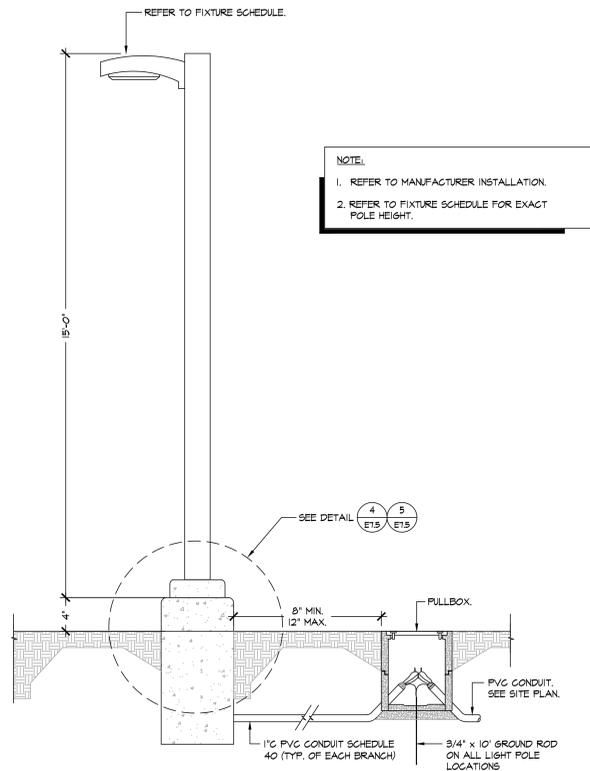
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ELECTRICAL DETAILS

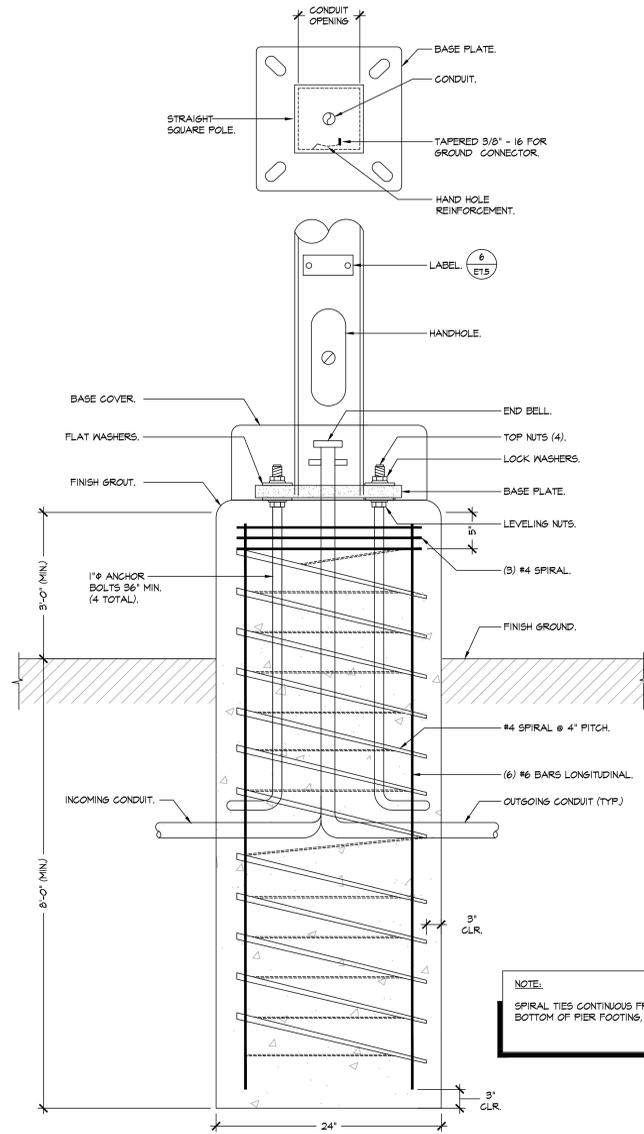
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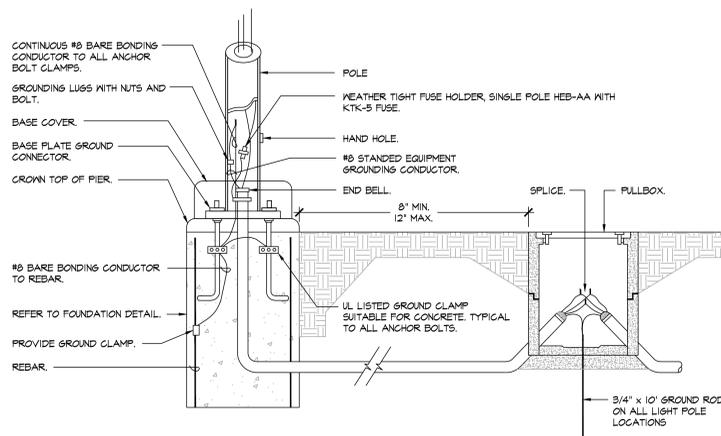
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E7.5 NOT TO SCALE



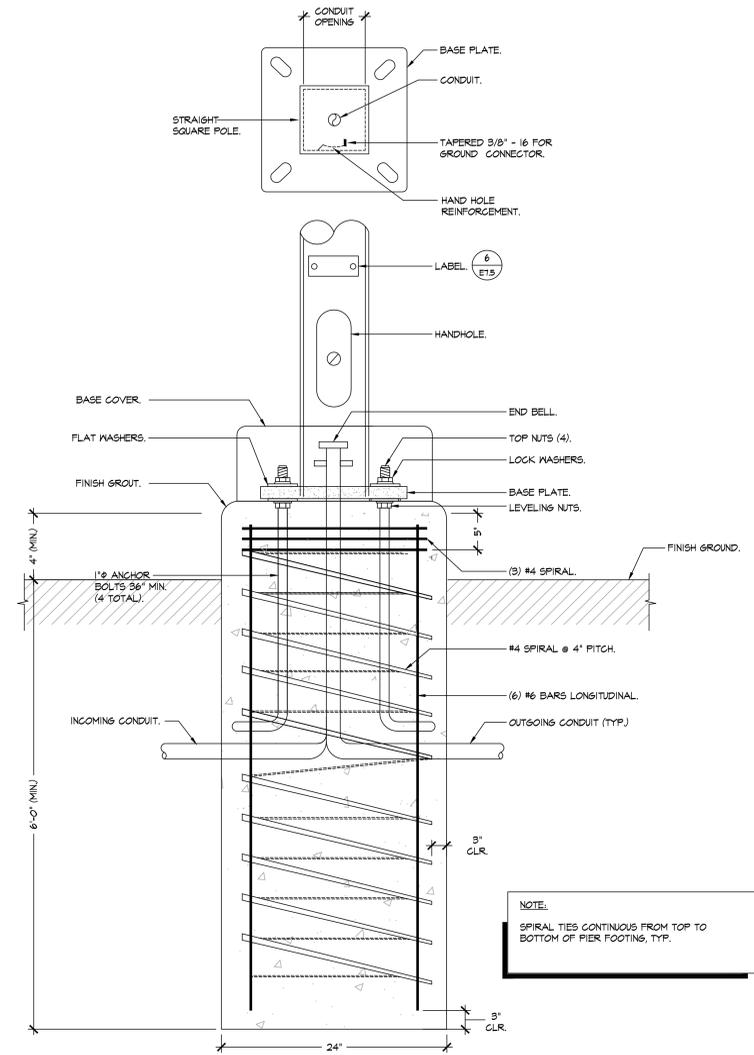
2 TYPICAL FLUSH LIGHT POLE DETAIL (PEDESTRIAN LIGHT)
E7.5 NOT TO SCALE



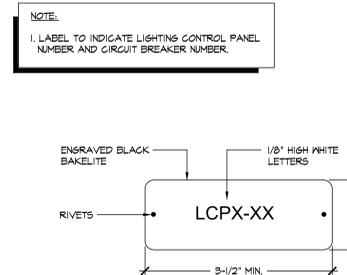
3 TYPICAL RAISED CONCRETE BASE ANCHOR BOLT & BASE PLATE DETAIL
E7.5 NOT TO SCALE



5 TYPICAL POLE BONDING / WIRING DETAIL (SECURITY LIGHT)
E7.5 NOT TO SCALE



4 TYPICAL FLUSH CONCRETE BASE ANCHOR BOLT & BASE PLATE DETAIL
E7.5 NOT TO SCALE



6 TYPICAL POST TAG
E7.5 NOT TO SCALE

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KEY MAP

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STRUCTURAL NOTES

GENERAL NOTES

- The following notes, typical details and schedules shall apply to all phases of this project unless otherwise shown or noted.
- Specific notes and details shall take precedence over general notes and typical details.
- All materials and workmanship shall conform to the minimum standards of the 2022 edition Title 24 of the California Building Code (CBC) and such other regulating agencies exercising authority over any portion of the work. The contractor shall have a current copy of the CBC on the job site.
- The "Contract or Construction Documents" shall consist of these notes, details, schedules, plans, and drawings.
- All specifications, including but not limited to materials and products, shall be those put forth in the "Contract or Construction Documents". No substitutions shall be permitted to be used or assumed to be used in the bidding or construction process without written approval by the Structural Engineer of Record.
- The contractor shall examine the "Contract or Construction Documents" and shall notify the Architect or Structural Engineer of Record of any discrepancies he may find before proceeding with the work.
- All information on existing conditions shown on drawings are based on best present knowledge available, but without guarantee of accuracy. The Contractor shall verify and be responsible for all dimensions and conditions at the site and shall notify the Architect or Structural Engineer of Record of any discrepancies between actual site conditions and information shown on or in the "Contract or Construction Documents" before proceeding with work.
- The Contractor shall immediately notify the Architect or Structural Engineer of Record of any condition which in his opinion might endanger the stability of the structure or cause distress of the structure.
- All work shall conform to the best practice prevailing in the various trades comprising work. The Contractor shall be responsible for coordinating the work of all trades.
- These "Contract or Construction Documents" represent the finished structure, and do not indicate the method of construction. The Contractor shall supervise and direct the work and shall be solely responsible for construction means, methods, techniques, sequences and procedures.
- Inspection and approval for fabricator's shops used for fabrication of structural load bearing members, components, materials or assemblies shall conform to CBC Section 1704A.2.5.
 - Labeling (as required or specified) shall be provided in accordance with CBC Section 1703A.5.
 - Evaluation and follow-up inspection services (as required or specified), shall conform to CBC Section 1703A.6.
- The Contractor shall provide temporary bracing and shoring for all structural members as required for structural stability of the structure during all phases of construction.
- The Contractor shall take all steps necessary to ensure proper alignment of the structure after the installation of all structural and finish materials. This shall include any necessary grading of the structure to determine final position of the completed work.
- Observation visits to the project site by field representatives of Architect and/or Structural Engineer of Record (support services) shall not include inspections of safety or protective measures, nor construction procedures, techniques or methods. Any support services performed by Architect or Structural Engineer of Record during any phase of construction, shall be distinguished from continuous and detailed inspection services (as required by any regulating governmental agency, e.g. the Authority Having Jurisdiction) provided by others. These support services, whether of material or work, are performed solely for the purpose of assisting in quality control and in achieving conformance with contract documents, but do not guarantee Contractor's performance and shall not be construed as supervision of construction.
- These notes, details, drawings and specifications (Contract or Construction Documents) do not carry necessary provisions for construction safety. These documents and all phases of construction hereby contemplated are to be governed, at all times, by applicable provisions of the current California Occupational Safety and Health Act.
- Where any conflict occurs between the requirements of federal, state and local laws, codes, ordinances, rules and regulations, the most stringent shall govern.
- Written dimensions shall have precedence over scaled dimensions.
- Drawings (notes, schedules, details and plans) shall have precedence over Structural Calculations.
- In the event that certain features of the construction are not fully shown on the drawings or called for in the General Notes or Specifications, then their construction shall be of the same character as for similar conditions that are shown or called for.
- ASTM designation and all standards refer to the latest amendments.
- These structural "Contract or Construction Documents" shall not be modified without prior written approval of the Structural Engineer of Record.
- Only structural working drawings approved by the Division of the State Architect are permitted to be used for construction on this project. All other drawings or documents are obsolete and are not permitted on the job site, nor shall they be used for any construction purposes. Contractors using unapproved drawings or documents are solely responsible for all work not performed in accordance with the "approved" drawings.
- A Division of the State Architect certified project inspector employed by the District (Owner) and approved by the Division of the State Architect shall provide continuous inspection of the work. The duties of the inspector are defined in Section 4-342, Part 1, Title 24 California Code of Regulations.

FOUNDATION NOTES

- Basis: See Structural Design Values Chart, Sheet SB0.1 Table B
- Unexpected soil conditions: Allowable values and foundation design are based upon the minimum values provided in Table 1806A.2 of the 2022 California Building Code. See SB0.1 for values
- Excavate to required depths and dimensions (as indicated in drawings), cut square and smooth with firm level bottoms. Care shall be taken not to over-excavate foundation at lower-elevation and prevent disturbing of soils around higher elevation.
- Footings shall be poured in neat excavations, without side forms whenever possible.
- Carry all foundations to required depths into compacted fill or natural soil (as per Structural Plans and Details).
- All foundation excavations shall be inspected and approved by the Inspector of Record or Geotechnical Engineer prior to forming and placement of reinforcing or concrete.
- Foundations shall not be poured until all required reinforcing steel, sleeves, inserts, conduits, pipes, etc. and formwork is properly placed and inspected by the Authority having Jurisdiction.
- The sides and bottoms of excavations which are to have concrete contact must be moistened several times just prior to pouring upon them.
- De-water footings, as required, to maintain dry working conditions.

REINFORCING STEEL

- All reinforcing steel shall be deformed intermediate grade bars conforming to ASTM A615, Grade 60 ($f_y = 60$ ksi) unless noted otherwise.
- Reinforcing steel shall not be welded, unless specifically noted otherwise.
- To hold reinforcing bars in their true position and prevent displacement, standard tie and anchorage devices must be provided. Placing of reinforcement shall conform to ACI 318-19 Section 26.6.2.
- Shop drawings for fabrication of any reinforcing steel shall be approved by Contractor and submitted to Project Specific Architect or Project Specific Structural Engineer of Record, for their review, prior to fabrication.
- Refer to typical details for minimum splice length and minimum radius of bend of reinforcing steel.
- All reinforcing steel splices shall be staggered 24", unless specifically noted or detailed otherwise.
- All reinforcing bar bends shall be made cold.
- Fabrication, erection and placement of reinforcing steel shall conform to Concrete Reinforcing Steel Institute of Standard Practice.
- Reinforcing steel shall be clean of rust, grease or other material likely to impair bond.

CONCRETE

- All concrete shall have a minimum ultimate compressive strength (f'_c) as outlined below at 28 days. All concrete shall be regular weight (unless specifically noted otherwise).
 - Concrete for footings: 4,500 psi $w/c = 0.45$ max.
- Maximum Fly Ash content shall be 15%, by weight, of total cementitious materials and shall conform to ASTM C618.
- All concrete work shall comply with CBC Chapter 19A and ACI 318-19 and latest edition of ACI Manual of Concrete Practice.
- Special Inspection (as required or specified) shall conform to CBC Chapter 17A.
- Cement shall be portland cement Type V and shall conform to ASTM C150.

- Aggregates shall conform to ASTM C33, provide aggregates from a single source.
- Water shall conform to ASTM C94 and be potable.
- Where not specifically detailed, the minimum concrete cover on reinforcing steel shall be:
 - Concrete cast against and permanently exposed to earth or weather: 3"
- All reinforcing steel, anchor bolts, dowels, inserts and any other hardware to be set in concrete shall be well secured in position prior to pouring of concrete.
- Vibrate all concrete as it is placed, with a mechanical vibrator operated by experienced personnel. The vibrator shall be used to consolidate the concrete, not transport it. Reinforcing and forms shall not be vibrated.
- Formwork design and removal shall conform to ACI 318-19 Section 26.11. Remove forms in accordance with the following minimum schedule:
 - Side forms of footings: Minimum 48 hours
 - Column and pier forms: 72 hours & 70% of design strength
- Concrete shall not free fall more than six feet. Use tremie, pump or other approved methods.
- Concrete shall be maintained in a moist condition for a minimum of 5 days after placement.
- The Contractor may use concrete admixtures as a construction means and methods to execute "Contract or Construction Documents". Use of admixture is solely the responsibility of the Contractor.
- Mix designs shall be prepared by an approved testing laboratory, signed by a licensed engineer and shall be submitted to the Project Specific Design Professional of Record for approval. SSG is not responsible for review or approval of site specific concrete mix design.
- Only one grade of concrete shall be allowed on project site at any one time
- Concrete strength shall be verified by standard cylinder tests (in accordance with CBC Section 1905A.1.16) made by an approved testing laboratory.
- Concrete placed when the air temperature has fallen to, or is expected to fall below 40° shall conform to ACI 318-19 Section 26.5.4, and ACI 306R-16.
- Concrete placed during hot weather shall conform to ACI 318-19 Section 26.5.5, and ACI 308R-14.
- Conduits and sleeves placed within structural concrete shall not be tied directly to structural reinforcement.
 - 1" concrete cover shall be maintained around reinforcement.
- No stakes shall be permitted within the footing section.
- Concrete shall reach minimum 75% design strength or cure for 3 days minimum prior to installation of steel columns and scoreboard components.

DRILLED CAISSON/PIER AND GRADE BEAM NOTES

- Excavations for drilled caissons/pier shall be performed in compliance with local grading codes and ordinances as well as CBC Chapters 18A and 33A.
- Provide Special Inspection in accordance with CBC Section 1705A.8 and Table 1705A.8.
- Excavations for all drilled caissons/piers shall be approved by the Project Geotechnical Engineer or Project Specific Inspector prior to placing of concrete.
- Reinforcement for drilled caissons/pier shall be approved by the Structural Engineer of Record prior to placing in caisson/pier excavation.
- De-water caisson/pier footings and building excavation as required to maintain dry working conditions.
- Caisson/piers are to be poured within 24 hours after completion of drilling operation. Shoring requirements shall be determined by contractor. Contractor shall provide fall protection and safety barriers at and near the drilled hole as required by OSHA and the Authority Having Jurisdiction.
- The Contractor shall be responsible for all shoring, bracing, etc. necessary to support cut and/or fill banks, and existing structures during excavation, and the forming and placement of concrete.
- Bottom of caissons/piers shall be thoroughly cleaned prior to placement of concrete.

STRUCTURAL STEEL AND WELDING

- All structural steel construction shall conform to AISC 360-16 and AISC 341-16.
 - Fabrication of all structural steel shall be done in the shop of an approved fabricator. Inspection and approval for fabricator's shops used for fabrication of structural load bearing members, components, materials or assemblies shall conform to CBC Section 1704A.2.5.
- All structural steel shall conform to the following specifications:
 - Angles, channels, plates, bars, rounds, and other miscellaneous shapes: Shall conform to ASTM A36 and shall have a minimum yield stress (F_y) of 36 ksi.
 - Wide-flange shapes: Shall conform to ASTM A992 and shall have a minimum yield stress (F_y) of 50 ksi.
 - Structural tubes: Shall be ASTM A500, Grade C, and shall have a min. yield stress (F_y) of 50ksi.
- All structural steel fasteners shall conform to the following specifications:
 - Bolts shall conform to ASTM A307
 - Anchor Bolts shall conform to ASTM F1554, Grade as noted in drawings
 - Carbon steel nuts shall conform to ASTM A563
 - Stainless steel nuts shall conform to ASTM F594.
 - Washers shall conform to ASTM F436
- Special Inspection shall be provided for all structural steel and welding, in accordance with CBC Chapter 17A.
- All structural steel shall be fabricated, erected and welded in accordance with AISC Specifications for Structural Steel Buildings (AISC 360-16) and Code of Standard Practice for Steel Buildings and Bridges (AISC 303-16).
- All welding shall be done by qualified and certified welders.
- Shop drawings for the fabrication of any structural steel shall be approved by the Contractor and submitted to Project Specific Architect or Project Specific Structural Engineer of Record for their review, prior to fabrication.
- No holes other than those specifically detailed shall be allowed through structural steel members. Burning of holes is not permitted.
- All welding shall conform to 'AWS D1.1' specifications for welding. (E-70XX Electrodes).
- Where fillet weld size is not indicated, use 'AWS' minimum size based on the thickness of the thinner part being welded, as specified in AISC Specifications for Structural Steel Buildings (AISC 360-10), Section J2.2.
- All butt welds to be complete joint penetration, unless specifically noted otherwise.
- Welder qualification requirements, welding procedure and welding electrodes for all structural steel (except structural sheet steel, see steel decking) shall conform to CBC Sections 1705A.2.1 and 2204A.1.
- Provide 3" minimum concrete cover around all structural steel below grade.
- Structural steel embedded into concrete shall be uncoated.
- Structural steel shall be hot-dip galvanized (minimum ASTM A123 or A153 Class D) or painted with zinc-rich primer, undercoat, and finish coat; or equivalent paint system.
- All exposed steel fasteners, including cast-in-place anchor bolts/rods, shall be stainless steel (Type 304 minimum), hot-dip galvanized (ASTM A153, Class D minimum or ASTM F2329), or protected with corrosion-preventative coating that demonstrated no more than 2% of red rust in minimum 1,000 hours of exposure in salt spray test per ASTM B117. Zinc plated fasteners do not comply with this requirement.

ABBREVIATIONS

A.B.	Anchor Bolt	HORIZ.	Horizontal
ABV.	Above	HSS	Hollow Steel Section
ACI	American Concrete Institute	HT	Height
ADJ.	Adjacent	ICC	International Building Code
AHJ	Division of the State Architect	ICC	International Code Council
AISC	American Institute of Steel Construction	ID	Inside Diameter
AOR	Architect of Record	IN.	Inch, Inches
APPROX.	Approximately	INT.	Interior
ASCE	American Society of Civil Engineers	ksi	Kips per Square Inch
ARCH.	Architect, Architecture	LL	Live Load
ASTM	American Society of Testing and Materials	MAX.	Maximum
ATR	All Thread Rod	MB	Machine Bolt
AWS	American Welding Society	MFR.	Manufactured, Manufacturer
B.O.	Bottom of _____	MIN.	Minimum
BOT.	Bottom	MPH	Miles per Hour
b/t	Between	N/R	Not Required
CAC	California Administrative Code	N.T.S.	Not to Scale
CBC	California Building Code	o.c.	On Center
CIP	Cast-in-place	OD	Outside Diameter
CJP	Complete Joint Penetration	o.c.	On Center
CL	Centerline	OD	Outside Diameter
CLR.	Clear	PEN.	Penetration
COL.	Column	PI	Plate
CONC.	Concrete	PIP	Partial Joint Penetration
CONN.	Connection	psi	Pounds per Square Inch
CONST.	Construction	PSF	Pounds per Square Foot
CONT.	Continue, Continuous	REBAR	Reinforcing Bar
Ø	Diameter	REINF.	Reinforcement
DBL.	Double	REQD	Required
DET.	Detail	S.F.	Square Feet
DI	Dead Load	SHT.	Sheet
DSA	Division of State Architect	SIM.	Similar
DWGS.	Drawings	SMS	Sheet Metal Screw
EA.	Each	SQ.	Square
E.F.	Each Face	STAGTD	Staggered
ELEC.	Electric, Electrical	STD.	Standard
ELEV.	Elevation	STL	Steel
EMBED.	Embedded, Embedment	SEOR	Structural Engineer of Record
EOR	Engineer of Record	T&B	Top and bottom
EQUIP.	Equipment	THR'D	Threaded
E.S.	Each Side	T.O.	Top of _____
E.W.	Each Way	TYP.	Typical
EXT.	Exterior	U.N.O.	Unless Noted Otherwise
FAB.	Fabricated	VERT.	Vertical
FDN.	Foundation	VIF	Verify in Field
F.G.	Finish Grade	w/	With
F.O.	Face of _____	w/c	Water/Cement Ratio
FRMG.	Framing	WSS	Welded Steel Stud
FT.	Foot-Feet	WT.	Weight
FTG.	Footing		
G.A.	Gauge		
GALV.	Galvanized		
GEOR	Geotechnical Engineer of Record		

POST INSTALLED ANCHOR & TESTING

- All post-installed anchors are to be tension tested with the exception that torque testing is allowed if the anchors are specifically designed as torque controlled
- Test quantity of post-installed anchors as noted below:

Application	Quantity
Non-structural (Equipment Anchorage, etc.)	50%
Structural	100%
- Apply proof test loads to anchors without removing the nut if possible. If not, remove nut and install a threaded coupler to the same tightness of the original nut using a torque wrench and apply load.
- All tests shall be performed in the presence of the inspector.
- Reaction loads from test fixtures may be applied close to the anchor being tested, provided the anchor is not restrained from withdrawing or restricted from a concrete shear cone type failure mechanism.
- Test equipment is to be calibrated by an approved testing laboratory in accordance with standard recognized procedures.
- The following criteria apply for the acceptance of installed anchors:
 - Hydraulic ram method: anchors tested with a hydraulic jack or spring loaded devices shall maintain the test load for a minimum of 15 seconds and shall exhibit no discernible movement during the tension test, e.g. as evidenced by loosening of the washer under the nut.
 - Torque wrench method: anchors tested with a calibrated torque wrench must attain the manufacturer's recommended torque within 1/2 turn of the nut.
 - Wedge or sleeve type: one-quarter turn of the nut from 3/8" sleeve anchor only.
 - Threaded type: one-quarter turn of the screw after initial seating of the screw head.
- If any anchor fails testing, test all anchors of the same type not previously tested until twenty consecutive anchors pass, then resume the initial test frequency. If the anchors are used for the support and bracing of non-structural components (pipe, duct or conduit), the twenty shall be only those anchors installed by the same trade.
- Test loads per ICC ESR, IAPMO, OR UES report
- When installing drilled-in anchors and/or powder driven pins in existing non-prestressed reinforced concrete, use care and caution to avoid cutting or damaging the existing reinforcing bars. When installing them by using prestressed concrete (pre- or post-tensioned) locate the prestressed tendons by into a non-destructive method prior to installation. Exercise extreme care and caution to avoid cutting or damaging the tendons during installation. Maintain a minimum clearance of one inch between the reinforcement and the drilled-in anchor and/or pin.

ANCHOR TORQUE TEST VALUES

Anchor Diameter	CONCRETE		MASONRY	
	HILTI KB TZ 2	SIMPSON STRONG BOLT 2	HILTI KB TZ 2	SIMPSON STRONG BOLT 2
ESR-4266	ESR-3037	ESR-4561	ER-240	
3/8"	30 ft-lb	30 ft-lb	15 ft-lb	20 ft-lb
1/2"	50 ft-lb	60 ft-lb	25 ft-lb	35 ft-lb
5/8"	40 ft-lb	90 ft-lb	30 ft-lb	55 ft-lb
1"	110 ft-lb	150 ft-lb	50 ft-lb	100 ft-lb

If the manufacturer's recommended installation torque is less than the test torque noted in the table, the manufacturer's recommended installation torque should be used in lieu of the tabulated values.

See manufacturer's ESR report for Maximum Impact Wrench Torque Rating.

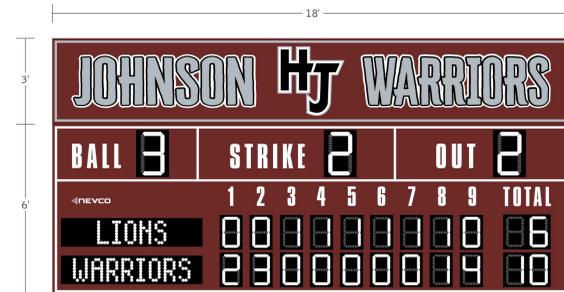
NOTE: FOR TESTING & SPECIAL INSPECTIONS SEE FORM DSA 103 SUBMITTED SEPARATELY

HIRAM JOHNSON HIGH SCHOOL, SACRAMENTO, CA

PROOF #58153C-PR

PROOF INCLUDES:

- Model 1408-ETN Baseball Softball LED Scoreboard
18W x 61" x 8"D
Scoreboard Color: #73 Maroon
Digit Color: White
Electronic Team Name Color: White
- Non-Illuminated Sign
18W x 31H



SIGNATURE OF APPROVAL

DATE

This rendering is for conceptual purposes only. It may not be to exact scale or specifications and should not be used for installation purposes. Every effort has been made to make it as accurate as possible. Beams and/or pilasters are for illustration only. Engineering specifications may require changes in the quantity, size and/or shape of beams and pilasters to meet installation requirements. Nevco assumes no obligations or liability regarding the viability or applicability of existing structures. THIS DRAWING IS THE PROPERTY OF NEVCO INC. AND SHALL NOT BE REPRODUCED, COPIED, SHARED OR DISTRIBUTED WITH ANYONE OTHER THAN THE INTENDED STAFF OR CLIENT OF THE PROPOSED PROJECT WITHOUT THE EXPRESSED PERMISSION OF NEVCO INC.



PC SEOR SEAL DATE SIGNED: 08.09.2023

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301 East Harris Avenue, Greenville, Illinois 62246

Phone: (618) 664-0960
www.nevco.com



PRE-CHECK (PC) DOCUMENT CODE: 2022

A separate project application for construction is required.

WEST CAMPUS HS, SCOREBOARD ASSEMBLY

BHEET INFORMATION

DATE: 08.09.2023

DRAWN: JMK

CHECKED: MEP

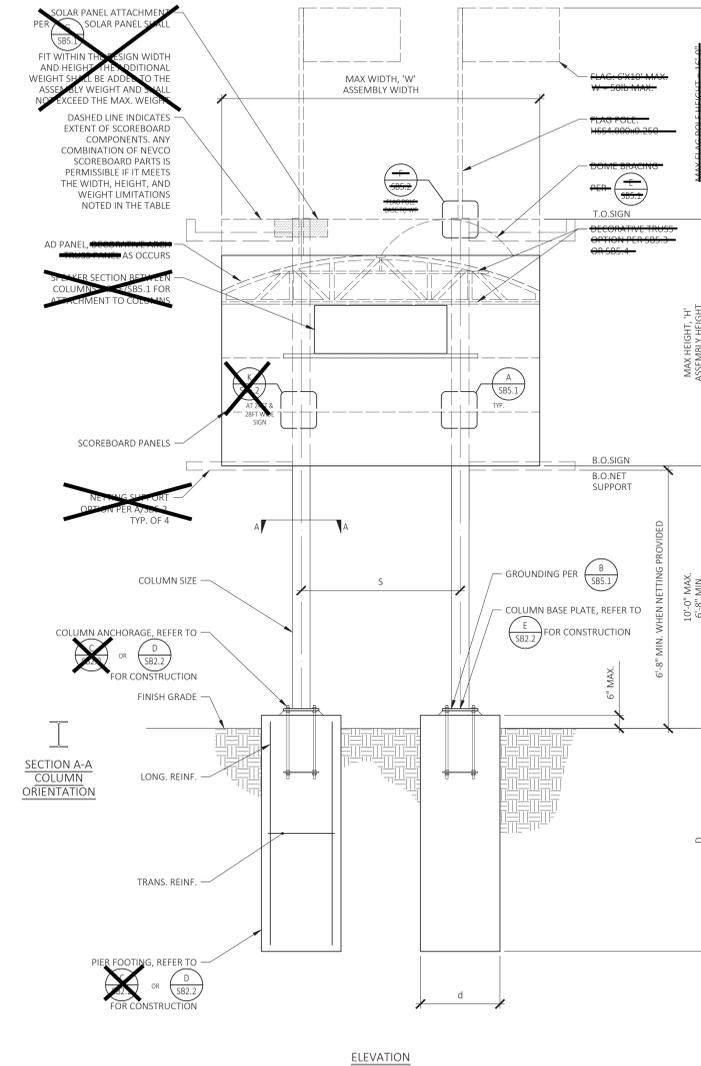
SSG JOB #: S23109

SHEET: SB0.2

STRUCTURAL NOTES & SPECIAL INSPECTIONS

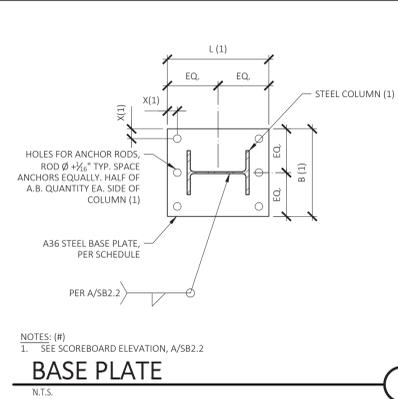
ASSEMBLY WIDTH, W	CHECK OPTION THIS APPLICATION	ASSEMBLY CRITERIA		TWO COLUMN ASSEMBLY														
		MAX. WEIGHT	ASSEMBLY HEIGHT, H	COLUMN SPACING, S	COLUMN SIZE	COLUMN SIZE W/O FLAG	PIER FOOTING CRITERIA (2)		BASE PLATE			ANCHOR RODS						
						PIER DIAMETER, d	DEPTH, D	LONG. REINF.	TRANS. REINF. (1)	THICKNESS, t	WIDTH, B	LENGTH, L	WELD	QUANTITY & DIAMETER	GRADE	EDGE DISTANCE, X	GROUT HEIGHT	EMBED
8'-0"		770 lbs.	≤ 8'-0"	8'-0"	W8x24	36"Ø	7'-0"	8-#6	#4 @ 42" o.c.	1 1/2"	20"	24"	3/8"	(4) - 1 1/2"	F1554-GR.36	2 1/2"	2"	48"
		1,160 lbs.	≤ 12'-0"	8'-0"	W10x33	36"Ø	8'-0"	8-#6	#4 @ 42" o.c.	1 1/2"	20"	20"	3/8"	(4) - 1 1/2"	F1554-GR.36	2 1/2"	2"	48"
		1,540 lbs.	≤ 16'-0"	8'-0"	W12x40	36"Ø	9'-0"	8-#6	#4 @ 42" o.c.	1 1/2"	20"	20"	3/8"	(4) - 1 1/2"	F1554-GR.55	2 1/2"	2"	48"
		1,920 lbs.	≤ 20'-0"	8'-0"	W14x61	36"Ø	10'-0"	8-#8	#4 @ 6" o.c.	1 1/2"	24"	24"	3/8"	(4) - 1 1/2"	F1554-GR.55	2 1/2"	2"	48"
9'-0"		870 lbs.	≤ 8'-0"	8'-0"	W8x24	36"Ø	7'-3"	8-#6	#4 @ 42" o.c.	1"	20"	20"	3/8"	(4) - 1 1/2"	F1554-GR.36	2 1/2"	2"	48"
		1,300 lbs.	≤ 12'-0"	8'-0"	W10x33	36"Ø	8'-3"	8-#6	#4 @ 42" o.c.	1 1/2"	20"	20"	3/8"	(4) - 1 1/2"	F1554-GR.36	2 1/2"	2"	48"
		1,730 lbs.	≤ 16'-0"	8'-0"	W12x40	36"Ø	9'-3"	8-#6	#4 @ 42" o.c.	1 1/2"	20"	20"	3/8"	(4) - 1 1/2"	F1554-GR.55	2 1/2"	2"	48"
		2,160 lbs.	≤ 20'-0"	8'-0"	W14x61	36"Ø	10'-3"	8-#8	#4 @ 6" o.c.	1 1/2"	24"	24"	3/8"	(4) - 1 1/2"	F1554-GR.55	2 1/2"	2"	48"
10'-0"		960 lbs.	≤ 8'-0"	8'-0"	W8x24	36"Ø	7'-6"	8-#6	#4 @ 42" o.c.	1"	20"	20"	3/8"	(4) - 1 1/2"	F1554-GR.36	2 1/2"	2"	48"
		1,440 lbs.	≤ 12'-0"	8'-0"	W10x33	36"Ø	8'-6"	8-#6	#4 @ 42" o.c.	1 1/2"	20"	20"	3/8"	(4) - 1 1/2"	F1554-GR.36	2 1/2"	2"	48"
		1,920 lbs.	≤ 16'-0"	8'-0"	W12x40	36"Ø	9'-6"	8-#6	#4 @ 42" o.c.	1 1/2"	24"	24"	3/8"	(4) - 1 1/2"	F1554-GR.105	2 1/2"	2"	48"
		2,400 lbs.	≤ 20'-0"	8'-0"	W14x61	36"Ø	10'-6"	8-#8	#4 @ 6" o.c.	1 1/2"	24"	24"	3/8"	(4) - 1 1/2"	F1554-GR.105	2 1/2"	2"	48"
12'-0"		1,100 lbs.	≤ 8'-0"	8'-0"	W10x33	36"Ø	8'-0"	8-#6	#4 @ 42" o.c.	1 1/2"	20"	20"	3/8"	(4) - 1 1/2"	F1554-GR.36	2 1/2"	2"	48"
		1,730 lbs.	≤ 12'-0"	8'-0"	W12x40	36"Ø	9'-3"	8-#6	#4 @ 42" o.c.	1 1/2"	24"	24"	3/8"	(4) - 1 1/2"	F1554-GR.55	2 1/2"	2"	48"
		2,310 lbs.	≤ 16'-0"	8'-0"	W14x61	36"Ø	10'-3"	8-#8	#4 @ 6" o.c.	1 1/2"	24"	24"	3/8"	(6) - 1 1/2"	F1554-GR.55	2 1/2"	2"	48"
		2,880 lbs.	≤ 20'-0"	8'-0"	W16x77	36"Ø	11'-3"	8-#8	#4 @ 6" o.c.	1 1/2"	24"	24"	3/8"	(6) - 1 1/2"	F1554-GR.55	2 1/2"	2"	48"
16'-0"		1,540 lbs.	≤ 8'-0"	8'-0"	W10x33	36"Ø	8'-9"	8-#6	#4 @ 42" o.c.	1 1/2"	20"	20"	3/8"	(4) - 1 1/2"	F1554-GR.55	2 1/2"	2"	48"
		2,070 lbs.	≤ 12'-0"	8'-0"	W12x45	36"Ø	10'-3"	8-#6	#4 @ 42" o.c.	1 1/2"	24"	24"	3/8"	(6) - 1 1/2"	F1554-GR.55	2 1/2"	2"	48"
		3,080 lbs.	≤ 16'-0"	8'-0"	W14x61	36"Ø	12'-0"	8-#8	#4 @ 6" o.c.	1 1/2"	24"	24"	3/8"	(6) - 1 1/2"	F1554-GR.55	2 1/2"	2"	48"
		3,840 lbs.	≤ 20'-0"	8'-0"	W16x77	36"Ø	12'-0"	12-#8	#4 @ 6" o.c.	1 1/2"	24"	24"	3/8"	(6) - 1 1/2"	F1554-GR.105	2 1/2"	2"	48"
18'-0"	X	1,730 lbs.	≤ 8'-0"	10'-0"	W12x35	36"Ø	9'-0"	8-#6	#4 @ 42" o.c.	1 1/2"	20"	20"	3/8"	(4) - 1 1/2"	F1554-GR.36	2 1/2"	2"	48"
		2,600 lbs.	≤ 12'-0"	10'-0"	W14x43	36"Ø	10'-0"	8-#8	#4 @ 6" o.c.	1 1/2"	24"	24"	3/8"	(4) - 1 1/2"	F1554-GR.55	2 1/2"	2"	48"
		3,420 lbs.	≤ 16'-0"	10'-0"	W16x77	36"Ø	10'-0"	12-#8	#4 @ 6" o.c.	1 1/2"	24"	24"	3/8"	(6) - 1 1/2"	F1554-GR.55	2 1/2"	2"	48"
		4,320 lbs.	≤ 20'-0"	10'-0"	W18x130	36"Ø	10'-0"	12-#8	#4 @ 6" o.c.	1 1/2"	24"	24"	3/8"	(6) - 1 1/2"	F1554-GR.55	2 1/2"	2"	48"
24'-0"		2,310 lbs.	≤ 8'-0"	14'-0"	W14x43	36"Ø	9'-9"	8-#6	#4 @ 42" o.c.	1 1/2"	24"	24"	3/8"	(4) - 1 1/2"	F1554-GR.55	2 1/2"	2"	48"
		3,460 lbs.	≤ 12'-0"	14'-0"	W16x67	36"Ø	11'-6"	8-#8	#4 @ 6" o.c.	1 1/2"	24"	24"	3/8"	(6) - 1 1/2"	F1554-GR.55	2 1/2"	2"	48"
		4,610 lbs.	≤ 16'-0"	14'-0"	W18x130	36"Ø	11'-6"	12-#8	#4 @ 6" o.c.	1 1/2"	24"	24"	3/8"	(6) - 1 1/2"	F1554-GR.55	2 1/2"	2"	48"
		5,760 lbs.	≤ 20'-0"	14'-0"	W20x135	36"Ø	13'-3"	12-#8	#4 @ 6" o.c.	1 1/2"	24"	24"	3/8"	(6) - 1 1/2"	F1554-GR.55	2 1/2"	2"	48"
28'-0"		6,920 lbs.	≤ 24'-0"	14'-0"	W18x130	36"Ø	14'-6"	12-#8	#4 @ 6" o.c.	2"	30"	30"	CIP	(6) - 2"	F1554-GR.105	3"	2"	48"
		8,070 lbs.	≤ 28'-0"	14'-0"	W18x158	36"Ø	14'-6"	12-#8	#4 @ 6" o.c.	2"	30"	36"	CIP	(6) - 2"	F1554-GR.105	4"	2"	48"
		2,690 lbs.	≤ 8'-0"	14'-0"	W14x43	36"Ø	11'-3"	8-#8	#4 @ 6" o.c.	1 1/2"	24"	24"	3/8"	(4) - 1 1/2"	F1554-GR.55	3"	2"	48"
		4,040 lbs.	≤ 12'-0"	14'-0"	W16x67	36"Ø	12'-9"	8-#8	#4 @ 6" o.c.	2"	24"	24"	3/8"	(4) - 1 1/2"	F1554-GR.55	3"	2"	48"
		5,380 lbs.	≤ 16'-0"	14'-0"	W18x143	36"Ø	14'-6"	12-#8	#4 @ 6" o.c.	2"	24"	24"	3/8"	(6) - 1 1/2"	F1554-GR.55	3"	2"	48"
		6,720 lbs.	≤ 20'-0"	14'-0"	W18x175	36"Ø	14'-6"	12-#8	#4 @ 6" o.c.	2"	24"	30"	CIP	(6) - 2"	F1554-GR.105	3"	2"	48"
		8,070 lbs.	≤ 24'-0"	14'-0"	W18x175	36"Ø	15'-9"	12-#8	#4 @ 6" o.c.	2"	24"	36"	CIP	(6) - 2"	F1554-GR.105	4"	2"	48"
		9,410 lbs.	≤ 28'-0"	14'-0"	W18x175	36"Ø	16'-6"	14-#8	#4 @ 6" o.c.	3"	24"	36"	CIP	(6) - 2"	F1554-GR.105	4"	2"	48"

NOTES: (R)
 1. CONTRACTOR OPTION TO PROVIDE TIES OR SPIRAL REINFORCING. SEE C/SB2.2 FOR THE OPTION, SEE D/SB2.2 FOR SPIRAL OPTION
 2. CONTRACTOR IS RESPONSIBLE FOR CASING PIERS AND DRILLING SEQUENCING TO PROTECT PIER EXCAVATION

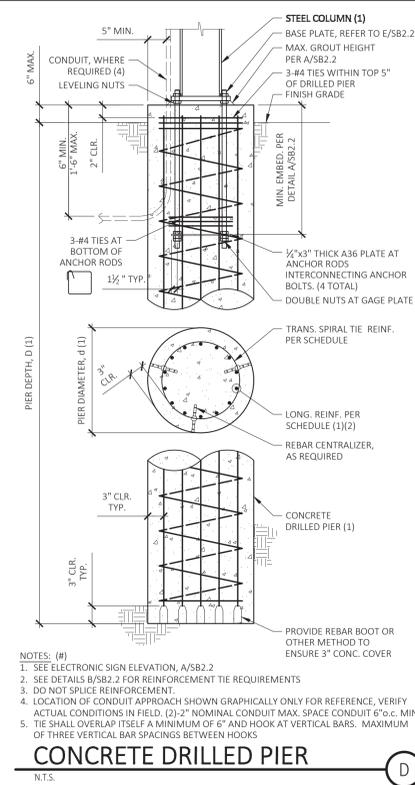


TWO COLUMN SCOREBOARD INSTALLATION

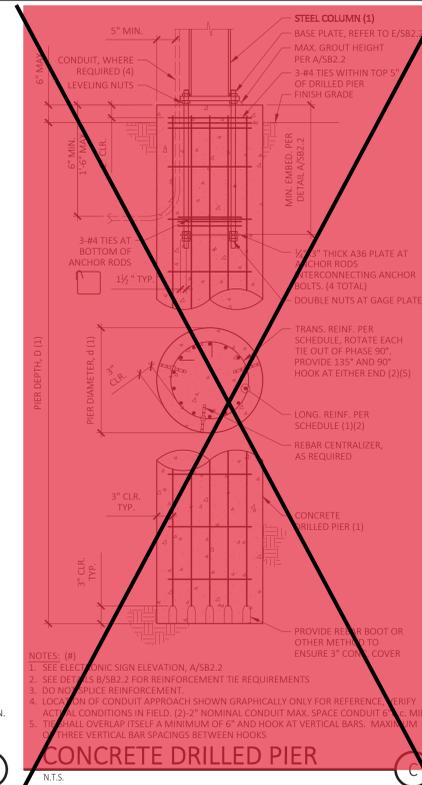
N.T.S.



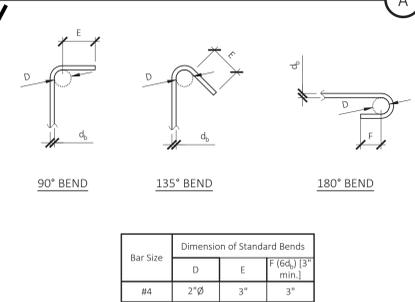
NOTES: (R)
 1. SEE SCOREBOARD ELEVATION, A/SB2.2
 N.T.S.



NOTES: (R)
 1. SEE ELECTRONIC SIGN ELEVATION, A/SB2.2
 2. SEE DETAILS B/SB2.2 FOR REINFORCEMENT TIE REQUIREMENTS
 3. DO NOT SPLICE REINFORCEMENT.
 4. LOCATION OF CONDUIT APPROACH SHOWN GRAPHICALLY ONLY FOR REFERENCE. VERIFY ACTUAL CONDITIONS IN FIELD. (2) 2" NOMINAL CONDUIT MAX. SPACE CONDUIT 6" MIN.
 5. TIE SHALL OVERLAP ITSELF A MINIMUM OF 6" AND HOOK AT VERTICAL BARS. MAXIMUM OF THREE VERTICAL BAR SPACINGS BETWEEN HOOKS
 CONCRETE DRILLED PIER
 N.T.S.



NOTES: (R)
 1. SEE ELECTRONIC SIGN ELEVATION, A/SB2.2
 2. SEE DETAILS B/SB2.2 FOR REINFORCEMENT TIE REQUIREMENTS
 3. DO NOT SPLICE REINFORCEMENT.
 4. LOCATION OF CONDUIT APPROACH SHOWN GRAPHICALLY ONLY FOR REFERENCE. VERIFY ACTUAL CONDITIONS IN FIELD. (2) 2" NOMINAL CONDUIT MAX. SPACE CONDUIT 6" MIN.
 5. TIE SHALL OVERLAP ITSELF A MINIMUM OF 6" AND HOOK AT VERTICAL BARS. MAXIMUM OF THREE VERTICAL BAR SPACINGS BETWEEN HOOKS
 CONCRETE DRILLED PIER
 N.T.S.



TIE AND STIRRUP BENDS
 N.T.S.

IDENTIFICATION STAMP
 DIV. OF THE STATE ARCHITECT
 APP: 02-121908 INC.
 REVIEWED FOR
 SS FLS ACS
 DATE: 05/01/2024

SSG
 structural engineers

REGISTERED PROFESSIONAL ENGINEER
 STATE OF CALIFORNIA
 No. 5405

DATE SIGNED: 08.09.2023
 PC SEOR REAL

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 THANK YOU FOR YOUR INTEREST IN NEVCO SCOREBOARD PRODUCTS

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 APP: 04-12237 PC
 REVIEWED FOR
 SS FLS ACS CG
 DATE: 09/20/2023

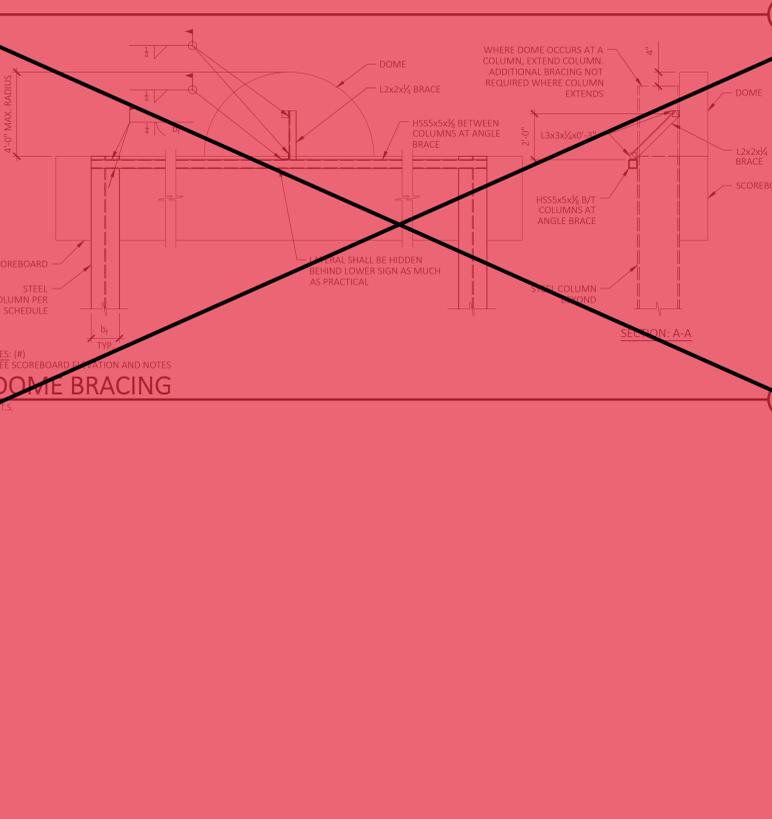
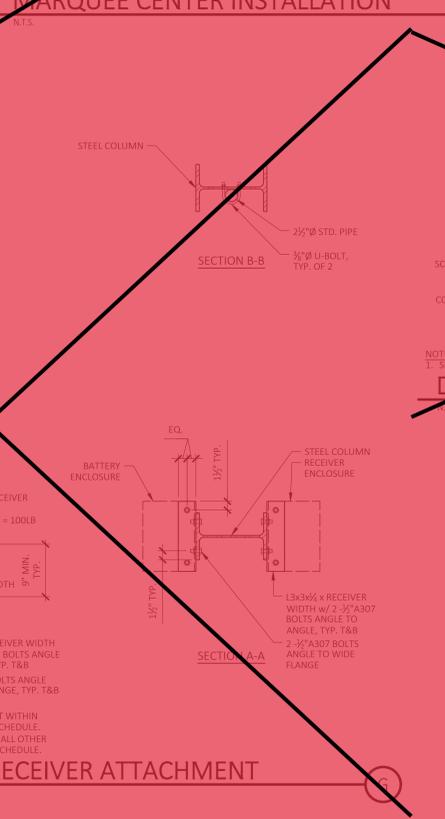
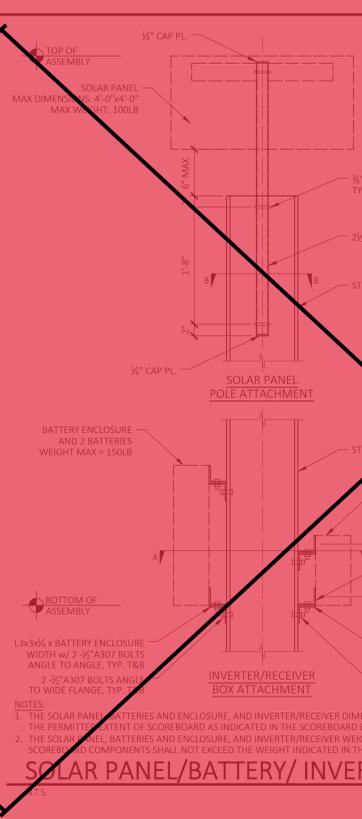
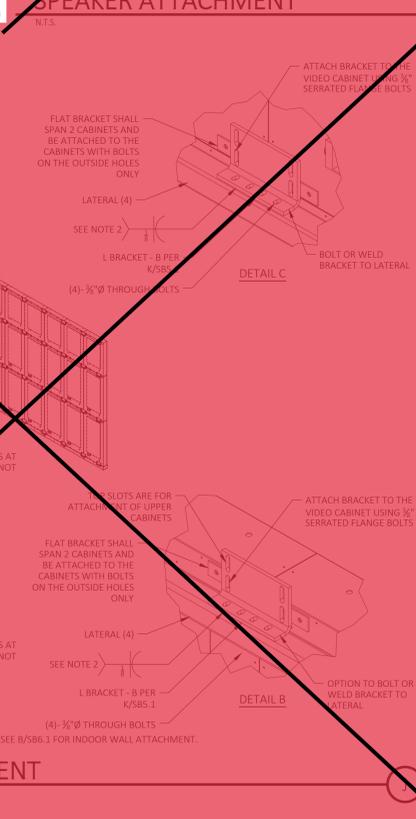
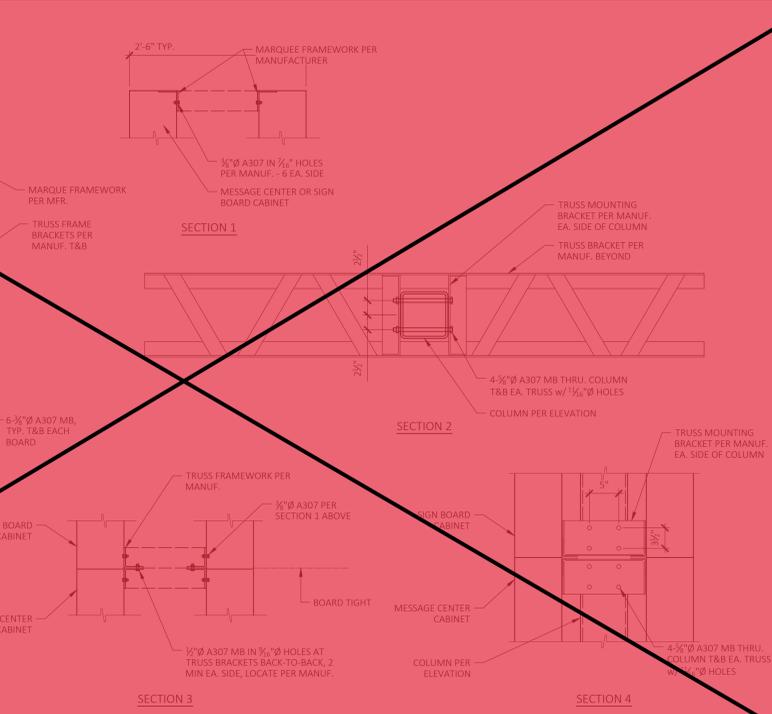
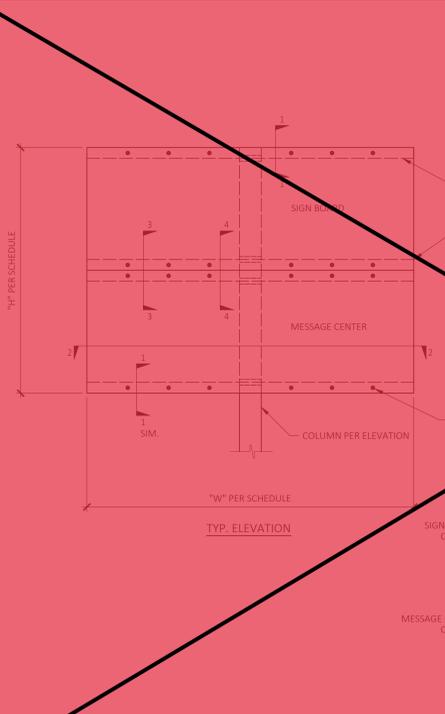
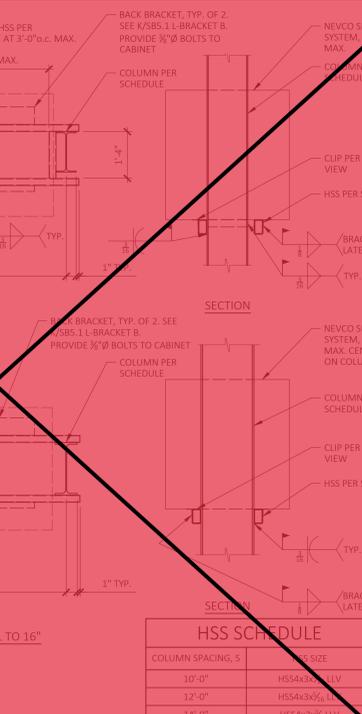
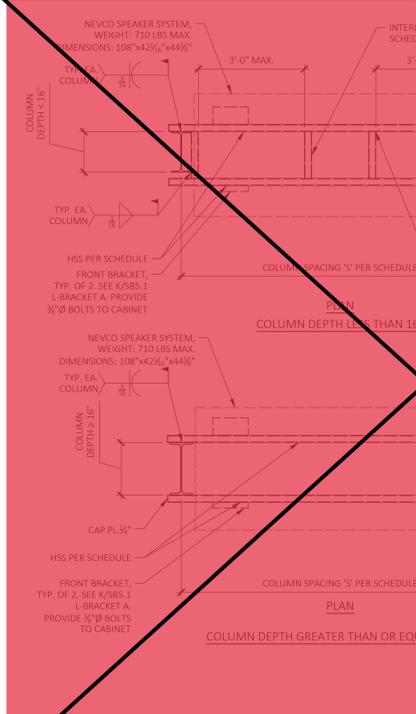
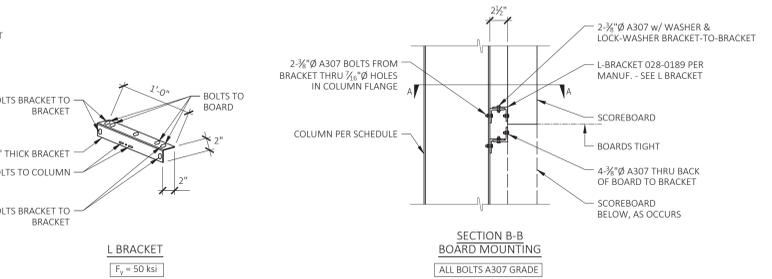
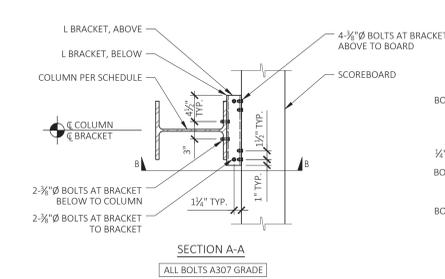
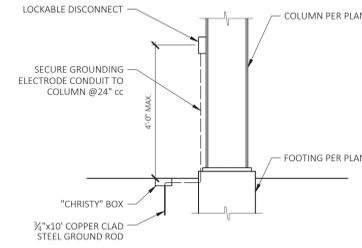
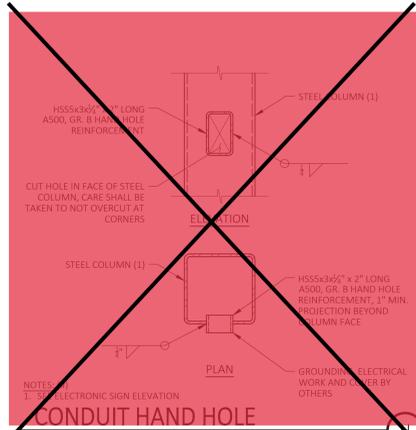
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 CODE: 2022

A separate project application for construction is required.

**WEST CAMPUS HS,
 SCOREBOARD ASSEMBLY**

TWO COLUMN
 CAISSON -
 BOLTED

SHEET INFORMATION
 DATE: 08.09.2023
 DRAWN: JMK
 CHECKED: MEP
 SSG JOB #: S23109
 SHEET: SB2.2



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REVIEWED FOR
SS FLS ACS
DATE: 05/01/2024

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PROFESSIONAL ENGINEER
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SS FLS ACS CG
DATE: 09/20/2023

PRE-CHECK (PC) DOCUMENT
CODE: 2022

A separate project application for construction is required.

**WEST CAMPUS HS
SCOREBOARD ASSEMBLY**

**ATTACHMENT
DETAILS**

SHEET INFORMATION

DATE: 08.09.2023
DRAWN: JMK
CHECKED: MEP
SSG JOB #: S23109
SHEET: SB5.1

ROMTEC

18240 NORTH BANK ROAD - ROSEBURG, OR 97470
 (541)-496-3541 FAX (541)-496-0803



PROJECT INFORMATION

PROJECT NAME WEST CAMPUS
 PROJECT I.D. WES01
 MODEL # 2022 SIERRA II 16'-8" W/ MECH RM
 SITE ADDRESS 5022 58TH ST
 CITY / STATE SACRAMENTO, CALIFORNIA

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G3	DESIGN CRITERIA AND CODE SUMMARY
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A1.2	ADA CLEARANCES
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E1	ELECTRICAL SCHEDULE
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E5-E12	TITLE 24 DOCUMENTS

REV.	DATE	BY	DESCRIPTION
8	04/19/24	CR	G0,S9.1,S10.1,S10.2,S10.3
7	04/15/24	CR	G0,A1.1,A1.3,A2.1,A2.2,A5.2,S8.5,S9.1,S10.1,S10.2,R1
6	03/19/24	CR	G0,S8.5
5	03/14/24	CR	G0,P1,M1,E1
4	02/27/24	CR	G0,G3,A1.3,S7.1,S7.2,S9.1,S10.1,S10.2,S10.3,R1,R2,M1,E5-E12
3	02/15/24	CR	G0,G2,A1.1,A1.2,A2.1,A2.2,A3.1,A3.2,A4.1,A5.2,S7.1,S7.2,S8.1,S8.2,S8.4,S9.1,R1,P1,P2,E1,E2,E3,E4
2	12/07/23	CR	G0,G1,G2,A3.1,A5.2,S7.1,S7.2,S7.3,S8.2,S8.4,S10.1,S10.2
1	11/07/2023	CR	E4

REVISION SCHEDULE

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PROJECT: 2022 SIERRA II COMPACT 16'-8" W/ MECH RM
 PLAN SET# WES01
 DATE: 11/01/2023
 REVISIONS
 REV. DATE BY
 5 03-14-2024 CR
 6 03-19-2024 CR
 7 04-15-2024 CR
 8 04-19-2024 CR
 DRAWN BY: CR

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 949.305.1150
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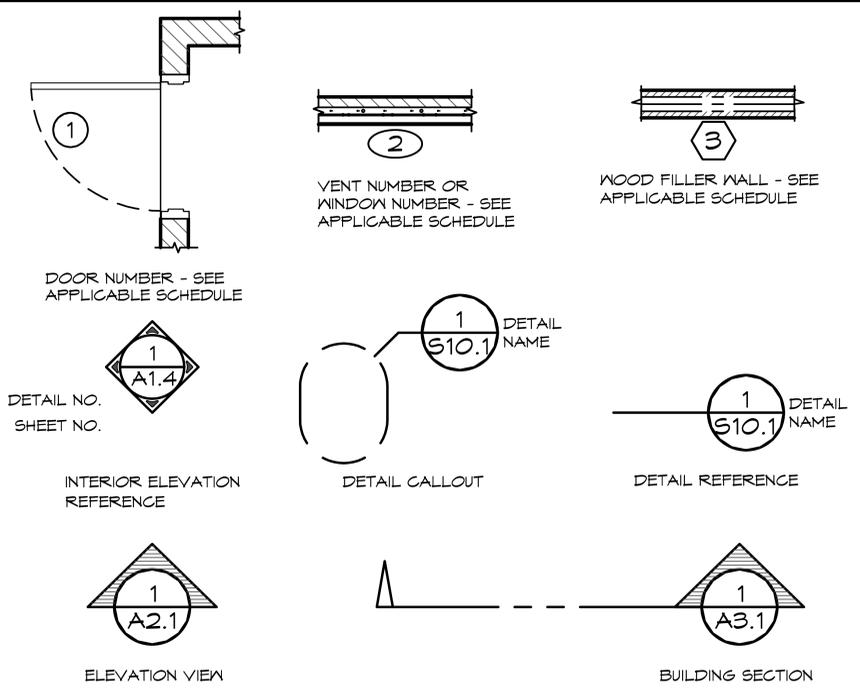
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WEST CAMPUS
 SACRAMENTO, CALIFORNIA
 SHEET TITLE: TITLE SHEET
 REVISION & SHEET SCHEDULE

SHEET NO.

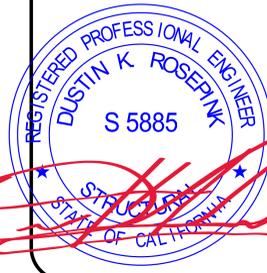
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SYMBOL LEGEND



ABBREVIATIONS

AB	ANCHOR BOLT	ND	NAPKIN DISPOSAL
AFF	ABOVE FINISHED FLOOR	NTS	NOT TO SCALE
ATS	AUTOMATIC TRANSFER SWITCH	OC	ON CENTER
BN	BOUNDARY NAIL	OCEW	ON CENTER EACH WAY
BOT	BOTTOM	OSB	ORIENTED STRAND BOARD
BP	BREAKER PANEL	P	PHOTO EYE
CJ	CONTROL JOINT	PCC	PORTLAND CEMENT COMPANY
CL	CENTER LINE	PEN	PANEL EDGE NAILING
CO	CLEAN OUT	PL	PLATE
CMU	CONCRETE MASONRY UNIT	PSF	POUNDS PER SQUARE FOOT
db	NOMINAL BAR DIAMETER	PSI	POUNDS PER SQUARE INCH
DD	DIAPER DECK	PT	PRESSURE TREATED
DIA	DIAMETER	PTD	PAPER TOWEL DISPENSER
DISC	DISCONNECT	PV	PHOTO VOLTAIC
EM	ELECTRIC METER	R4S	ROUGH FOUR SIDES
EN	END NAIL	REQD	REQUIRED
EW	EACH WAY	RO	ROUGH OPENING
FD	FLOOR DRAIN	S4S	SURFACED FOUR SIDES
FF	FINISHED FLOOR	SCH	SCHEDULE
FG	FINISHED GRADE	SD	SOAP DISPENSER
FN	FIELD NAIL	SIP	STRUCTURAL INSULATED PANEL
FRP	FIBERGLASS REINFORCED PANEL	SJ	SAW JOINT
GB	GRAB BAR	SM	SHEET METAL
GLB	GLUE LAMINATED BEAM	SN	SHEAR NAILING
HB	HOSE BIBB	SS	STAINLESS STEEL
HD	HAND DRYER	SST	STRUCTURAL STEEL TUBE
HM	HOLLOW METAL (DOOR)	TBD	TO BE DETERMINED
HTR	HEATER	T&B	TOP & BOTTOM
HYP	HYPOTENUSE	T&G	TONGUE & GROOVE
I.S.	INSTALLER SUPPLIED	TLT	TOILET
KSI	KIPS PER SQUARE INCH	TP	TOILET PAPER DISPENSER
L	STRUCTURAL STEEL ANGLE	TS	TIMER SWITCH
LAV	LAVATORY	TSCD	TOILET SEAT COVER DISPENSER
LF	LIGHT FIXTURE	TYP	TYPICAL
MBP	MAIN BREAKER PANEL	UNO	UNLESS NOTED OTHERWISE
MD	MAIN DISCONNECT	VB	VAPOR BARRIER
MIN	MINIMUM	VTR	VENT THROUGH ROOF
MIR	MIRROR	WH	WATER HEATER
MO	MASONRY OPENING	WWM	WOVEN WIRE MESH
MR	METAL ROOFING	W/	WITH
MS	MILD STEEL		



GENERAL NOTES

- THIS PROJECT SHALL COMPLY WITH ALL 2022 CALIFORNIA BUILDING CODES AND STANDARDS IDENTIFIED ON SHEET G2. ALL WORK SHALL MEET OR EXCEED INDUSTRY STANDARDS FOR MATERIALS, WORKMANSHIP, ETC.
- CONTRACTOR SHALL REVIEW THE DRAWINGS THOROUGHLY BEFORE PROCEEDING WITH ANY WORK. ANY DISCREPANCIES FOUND WITHIN THESE DOCUMENTS SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF ROMTEC. CONTRACTOR SHALL NOT PROCEED WITH ANY WORK HE KNOWS TO BE IN CONFLICT WITH OTHER WORK, OR IS NOT APPROVED BY CODE, UNTIL RESOLVED BY ROMTEC OR THE ENGINEER/ARCHITECT.
- CONTRACTOR SHALL MAINTAIN GENERAL LIABILITY INSURANCE AND WORKER'S COMP. INSURANCE AS PER SPECIFIC STATE MINIMUM REQUIREMENTS.
- FOOTINGS SHALL BE CONSTRUCTED ON UNDISTURBED NATIVE SOIL OR ENGINEER APPROVED FILL. CONTRACTOR TO VERIFY ASSUMED SOIL BEARING CAPACITY NOTED ON SHEET G2. SHOULD SOIL NOT MEET OR EXCEED THE ASSUMED SOIL BEARING CAPACITY, CONTRACTOR TO MODIFY SOIL CONDITIONS TO SATISFY CRITERIA OR NOTIFY THE STRUCTURAL ENGINEER TO REVISE DESIGN PER CONDITIONS ENCOUNTERED. BACKFILL AROUND BUILDING TO PROVIDE SLOPE AWAY FROM BUILDING NOT LESS THAN A 5% SLOPE FOR A MINIMUM DISTANCE OF 10' FROM THE BUILDING, PER 2022 CBC 1804A.3. REFER TO GEOTECHNICAL REPORT BY UES, No. 4630.2300077.0016, DATED NOVEMBER 1, 2023.
- A. CAST-IN-PLACE CONCRETE: 3000 PSI MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS 4" +/- 1" SLUMP, WITH MAX 1" AGGREGATE, AND ALL MATERIALS IN ACCORDANCE WITH ACI 318 STANDARD. FINE BROOM FINISH INTERIOR SURFACES AND EXTERIOR SLABS. JOINTS REQUIRED IN FLAT WORK, SEE FOUNDATION DETAILS FOR REQUIREMENTS.
 B. CMU BLOCKS "MEDIUM WEIGHT DENSITY" ARE MANUFACTURED TO ASTM C90-16 STANDARDS WITH A MIN COMPRESSIVE STRENGTH $F_m = 2000$ PSI. ALL CMU BLOCKS MUST BE FULLY GROUTED IN 5 FT MAXIMUM LIFTS AND NOT BE WETTED. THE MORTAR TO BE USED SHALL BE TYPE S 2000 PSI MORTAR CONFORMING TO ASTM C270.
 MASONRY (CONCRETE) GROUT: 2500 PSI MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS 9" +/- 1" SLUMP, WITH MAX 1/2" AGGREGATE, AND TESTED IN ACCORDANCE TO MEET ACI 318. FINE OR COURSE GROUT MAY BE USED IN ACCORDANCE WITH 2022 CBC. CONSOLIDATE GROUT AT THE TIME OF PLACEMENT. CONSOLIDATE POURS EXCEEDING 12 IN. IN HEIGHT BY MECHANICAL VIBRATION, AND RECONSOLIDATE BY MECHANICAL VIBRATION AFTER INITIAL WATER LOSS AND SETTLEMENT HAS OCCURRED. CONSOLIDATION AND RECONSOLIDATION ARE NORMALLY ACHIEVED WITH A MECHANICAL VIBRATOR. A LOW VELOCITY VIBRATOR WITH A 3/4 IN. HEAD IS USED.
- ANCHOR AND MACHINE BOLTS SHALL BE ASTM A307, UNLESS NOTED OTHERWISE BOLTS SHALL BE INSTALLED PER TURN-OF-NUT INSTALLATION METHOD REQUIRED TURNS FOR PRE-TENSIONING FROM SNUG-TIGHT, U.N.O. IN THIS PLANSET OR BY ANCHOR, BOLT OR FASTENER MANUFACTURER. SCREWS AND MACHINE BOLT CALLOUTS ARE MINIMUM SIZE SIZE ALLOWED, ACTUAL SIZE MAY VARY. STEEL PLATES & SHAPES SHALL BE ASTM A36, $F_y = 36$ ksi. CONCRETE REINFORCING STEEL (REBAR): ASTM A615 60 ksi. (GRADE 60). WOOD FRAMING SHALL BE #2 & BTR DOUGLAS FIR, UNO. GLU-LAM BEAMS SHALL BE GRADE 24F-V4 OR AS STATED IN NOTE #10.
- QUESTIONS CONCERNING MATERIALS OR CONSTRUCTION CONTACT ROMTEC TECHNICAL ASSISTANCE AT: 541-496-3541
- ROMTEC SCOPE SUPPLY AND DESIGN SUBMITTAL (SSDS) IDENTIFY SPECIFIC MODEL, MANUFACTURER & BRAND OF ALL PLUMBING AND ELECTRICAL FIXTURES AND ACCESSORIES. REFER TO THE SSDS FOR SPECIFIC LIST OF ITEMS SUPPLIED BY ROMTEC, ANY ITEMS NOT LISTED IN THE SSDS IS ASSUMED SUPPLIED BY THE INSTALLER.
- THE OWNER / CONTRACTOR MAY EXERCISE DISCRETION IN SELECTING THE FINAL LOCATION FOR NON-DIMENSIONED ACCESSORIES AND FIXTURES (E.G., LIGHTS, COMFORT HEATERS, ETC.)
- GLUE LAMINATED BEAMS SHALL BE DOUGLAS FIR-LARCH, U.N.O. WITH 1-1/2" OUTER AND CORE LAMINATIONS AND SHALL CONFORM TO THE "STANDARD SPECIFICATIONS FOR STRUCTURAL GLUED LAMINATED TIMBER", AITC/A.P.A.-E.W.S. #117, ANSI/AITC A-190.1 AND ALL APPROVED SUPPLEMENTS THEREOF.
 GLUE LAMINATED BEAM SHALL HAVE THE FOLLOWING GRADES (U.N.O. ON PLANS):
 - FOR SIMPLY SUPPORTED BEAMS.....COMBINATION 24F-V4
 - FOR CANTILEVERED BEAMS OR BEAMS CONTINUOUS OVER SUPPORTS.....COMBINATION 24F-V8 (20F-V12 FOR AC/AC)
 BEAMS SHALL CONFORM TO A.P.A.-E.W.S. OR A.I.T.C. INDUSTRIAL APPEARANCE GRADE, U.N.O.
 MOISTURE CONTENT OF THE LUMBER AT THE TIME OF GLUING SHALL NOT BE MORE THAN 16% WITH A MAX VARIATION OF 5% IN ANY BEAM. BEAMS SHALL BEAR LEGIBLE A.P.A.-E.W.S. OR A.I.T.C. GRADE STAMP. IF GRADE STAMP ILLEGIBLE OR REQUIRED BY BUILDING OFFICIALS, A "CERTIFICATE OF INSPECTION" BY AN APPROVED INSPECTION AGENCY SHALL BE SUBMITTED TO THE BUILDING DEPARTMENT PRIOR TO ERECTION.

NOTE: ARCHITECT/ENGINEER IS NOT RESPONSIBLE FOR ANY SITE DESIGN OR ENGINEERING AND WILL NOT BE HELD ACCOUNTABLE OR LIABLE FOR ANY ISSUES RELATED TO THIS SITE. IT IS THE OWNER'S RESPONSIBILITY TO ACCURATELY LOCATE THIS BUILDING, SET FLOOR AND ADJACENT ELEVATIONS, DETERMINE SITE IS SUITABLE FOR CONSTRUCTION, VERIFY ALL UTILITIES, ETC.

RECYCLE
 RECYCLE ALL USED SHIPPING MATERIALS AND LEFT OVER BUILDING MATERIALS

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WEST CAMPUS
 SACRAMENTO, CALIFORNIA
 PROJECT: 2022 SIERRA II COMPACT 16'-8" W/ MECH RM

PLAN SET#	NES01	
DATE:	11/01/2023	
REVISIONS		
REV.	DATE:	BY:
1	10-23-2023	CR
2	12-07-2023	CR
4	02-27-2024	CR

DRAWN BY: CR

SHEET NO. **G1**

CODES AND STANDARDS

2022 CALIFORNIA BUILDING CODE (CBC), TITLE 24 PART 2 (BASED ON 2021 IBC)
 2022 CALIFORNIA ELECTRICAL CODE, TITLE 24 PART 3 (BASED ON 2020 NFPA, NEC)
 2022 CALIFORNIA MECHANICAL CODE, TITLE 24 PART 4 (BASED ON 2021 UMC)
 2022 CALIFORNIA PLUMBING CODE, TITLE 24 PART 5 (BASED ON 2021 UPC)
 2022 CALIFORNIA BUILDING ENERGY CODE, TITLE 24 PART 6
 2022 CALIFORNIA FIRE CODE, TITLE 24 PART 9, (BASED ON 2021 IFG)
 2022 CALIFORNIA GREEN BUILDING STANDARDS CODE

ACI AMERICAN CONCRETE INSTITUTE, ACI 318-19, "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE"
 TMS THE MASONRY SOCIETY, TMS 402-16, "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES"
 AISC AMERICAN INSTITUTE OF STEEL CONSTRUCTION
 "STEEL CONSTRUCTION MANUAL, 15TH EDITION"

CODE SUMMARY:

OCCUPANCY CLASS.: U
 CONSTRUCTION: VB
 AREA: 222 FT²
 AREA ALLOWABLE: 5500 FT²
 HEIGHT: 1 STORY
 HEIGHT ALLOWABLE: 1 STORY
 OCCUPANT LOAD: 4

DESIGN LOADS

ROOF: LIVE LOAD 20 PSF
 ROOF: DEAD LOAD 15 PSF

CBC SEISMIC DESIGN CATEGORY D
 DESIGN WIND SPEED (ULTIMATE) 95 MPH
 EXPOSURE C
 ALLOWABLE SOIL BEARING 2000 PSF

PER GEOTECHNICAL ENGINEERING REPORT
 BY UES, DATED NOVEMBER 1, 2023.

SEISMIC DESIGN DATA:

RISK CATEGORY: II
 IMPORTANCE FACTOR: 1.0
 SS: 0.546
 S1: 0.247
 SITE CLASS: D
 SMS: 0.744
 SM1: 0.494
 SDS: 0.496
 SD1: 0.329
 SEISMIC DESIGN CATEGORY: D
 R = 5
 BASE SHEAR: V = 0.099 W

WIND DESIGN :

RISK CATEGORY: II
 WIND SPEED = 95 MPH
 EXPOSURE: C
 INTERNAL PRESSURE COEFF = ±0.18

SPECIAL INSPECTIONS (TMS 402-16)

MINIMUM VERIFICATION	REQUIRED FOR QUALITY ASSURANCE (a)			REFERENCE FOR CRITERIA
	LEVEL 1	LEVEL 2	LEVEL 3	TMS 602
PRIOR TO CONSTRUCTION, VERIFICATION OF COMPLIANCE OF SUBMITTALS	R	R	R	ART. 1.5
PRIOR TO CONSTRUCTION, VERIFICATION OF f_m AND f_{AAC} , EXCEPT WHERE SPECIFICALLY EXEMPT BY THE CODE.	NR	R	R	ART. 1.4 B
DURING CONSTRUCTION, VERIFICATION OF SLUMP FLOW AND VISUAL STABILITY INDEX (VSI) WHEN SELF-CONSOLIDATING GROUT IS DELIVERED TO THE PROJECT SITE.	NR	R	R	ART. 1.5 & 1.6.3
DURING CONSTRUCTION, VERIFICATION OF f_m AND f_{AAC} FOR EVERY 5,000 sq. ft. (465 sq.m).	NR	NR	R	ART. 1.4 B
DURING CONSTRUCTION, VERIFICATION OF PROPORTIONS OF MATERIALS AS DELIVERED TO THE PROJECT SITE FOR PREMIXED OR PREBLENDED MORTAR, PRESTRESSING GROUT, AND GROUT OTHER THAN SELF-CONSOLIDATING GROUT.	NR	NR	R	ART. 1.4 B

(a) R=REQUIRED, NR=NOT REQUIRED

TABLE 4 -- MINIMUM SPECIAL INSPECTION REQUIREMENTS

MINIMUM SPECIAL INSPECTION	REQUIRED FOR QUALITY ASSURANCE (a)			REFERENCE FOR CRITERIA	
	LEVEL 1	LEVEL 2	LEVEL 3	TMS 402	TMS 602
1. AS MASONRY CONSTRUCTION BEGINS, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE:					
A. PROPORTION OF SITE-PREPARED MORTAR	NR	P	P		ART. 2.1, 2.6 A, & 2.6 C
B. GRADE AND SIZE OF PRESTRESSING TENDONS AND ANCHORAGES	NR	P	P		ART. 2.4 B & 2.4 H
C. GRADE, TYPE AND SIZE OF REINFORCEMENT, CONNECTORS, ANCHOR BOLTS, AND PRESTRESSING TENDONS AND ANCHORAGES	NR	P	P		ART. 3.4 & 3.6 A
D. PRESTRESSING TECHNIQUE	NR	P	P		ART. 3.6 B
E. PROPERTIES OF THIN-BED MORTAR FOR AAC MASONRY	NR	C(b)/P(c)	C		ART. 2.1 C.1
F. SAMPLE PANEL CONSTRUCTION	NR	P	C		ART. 2.1 C.1
2. PRIOR TO GROUTING, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE:					
A. GROUT SPACE	NR	P	C		ART. 3.2 D & 3.2 F
B. PLACEMENT OF PRESTRESSING TENDONS AND ANCHORAGES	NR	P	P	SEC. 10.8 & 10.9	ART. 2.4 & 3.6
C. PLACEMENT OF REINFORCEMENT, CONNECTORS, AND ANCHOR BOLTS	NR	P	C	SEC. 6.1, 6.3.1, 6.3.6, & 6.3.7	ART. 2.4 & 3.6
D. PROPORTIONS OF SITE-PREPARED GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS	NR	P	P		ART. 2.6 B & 2.4 G.1.b
3. VERIFY COMPLIANCE OF THE FOLLOWING DURING CONSTRUCTION:					
A. MATERIALS AND PROCEDURES WITH THE APPROVED SUBMITTALS	NR	P	P		ART. 1.5
B. PLACEMENT OF MASONRY UNITS AND MORTAR JOINT CONSTRUCTION	NR	P	P		ART. 3.3 B
C. SIZE AND LOCATION OF STRUCTURAL MEMBERS	NR	P	P		ART. 3.3 F
D. TYPE, SIZE AND LOCATION OF ANCHORS, INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES OR OTHER CONSTRUCTION.	NR	P	C	SEC. 1.2.1(e), 6.2.1 & 6.3.1	
E. WELDING OF REINFORCEMENT	NR	C	C	SEC. 6.1.6.1.2	
F. PREPARATION, CONSTRUCTION, AND PROTECTION OF MASONRY DURING COLD WEATHER (TEMPERATURE BELOW 40°F(4.4°C)) OR HOT WEATHER (TEMPERATURE ABOVE 90°F(32.2°C))	NR	P	P		ART. 1.8 C & 1.8 D
G. APPLICATION AND MEASUREMENT OF PRESTRESSING FORCE	NR	C	C		ART. 3.6 B
H. PLACEMENT OF GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS IS IN COMPLIANCE	NR	C	C		ART. 3.5 & 3.6 C
I. PLACEMENT OF AAC MASONRY UNITS AND CONSTRUCTION OF THIN-BED MORTAR JOINTS	NR	C(b)/P(c)	C		ART. 3.3 B.9 & 3.3 F.1.b
4. OBSERVE PREPARATION OF GROUT SPECIMENS, MORTAR SPECIMENS, AND/OR PRISMS	NR	P	C		ART. 1.4 B.2.a.3, 1.4 B.2.b.3, 1.4 B.2.c.3, 1.4 B.3, & 1.4 B.4

(a) FREQUENCY REFERS TO THE FREQUENCY OF INSPECTION, WHICH MAY BE CONTINUOUS DURING THE LISTED TASK OR PERIODICALLY DURING THE LISTED TASK, AS DEFINED IN THE TABLE.
 NR=NOT REQUIRED, P=PERIODIC, C=CONTINUOUS
 (b) REQUIRED FOR THE FIRST 5000 SQUARE FEET (465 SQUARE METERS) OF AAC MASONRY.
 (c) REQUIRED AFTER THE FIRST 5000 SQUARE FEET (465 SQUARE METERS) OF AAC MASONRY.



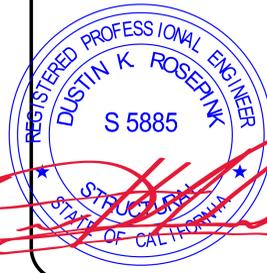
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PROJECT: 2022 SIERRA II COMPACT 16'-8" W/ MECH RM
 PLAN SET# NES01
 DATE: 11/01/2023
 REVISIONS
 REV. DATE BY
 1 10-29-2023 CR
 2 12-07-2023 CR
 3 02-15-2024 CR
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WEST CAMPUS
 SACRAMENTO, CALIFORNIA
 SHEET TITLE: DESIGN CRITERIA AND CODE SUMMARY



California Building Code 2022 (Vol 1 & 2)

1705A.3 Concrete Construction

Special inspections and tests of concrete construction shall be performed in accordance with this section and Table 1705A.3.

Exception: Special inspections and tests shall not be required for concrete patios, driveways and sidewalks, on grade.

**TABLE 1705A.3
 REQUIRED SPECIAL INSPECTIONS AND TESTS OF CONCRETE CONSTRUCTION**

TYPE	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION	REFERENCED STANDARD ^a	CBC REFERENCE
1. Inspect <i>and</i> test reinforcement, including prestressing tendons, and verify placement.				
a. Reinforcement in special moment frames, boundary elements of special structural walls and coupling beams.	X	—	ACI 318: Ch. 20, 25.2, 25.3, 25.5.1, 25.6.1—25.6.3, 26.13.1, 26.13.3.2, 26.13.3.3	1705A.3.9, 1908A.1, 1910A.2, 1910A.3, [DSA-SS/CC] 1909.2.4, 1909.2.5, 1909.4.1
b. All other reinforcement	—	X		
2. Reinforcing bar welding:				
a. Verify weldability of reinforcing bars other than ASTM A706.	—	X	AWS D1.4	1705A.3.1, 1903A.8
b. Inspect single-pass fillet welds, maximum $\frac{5}{16}$ "; not defined in 2.d or 2.e.	—	X		
c. Inspect all other welds.	X	—		
d. Reinforcing steel resisting flexural and axial forces in intermediate and special moment frames, and boundary elements and coupling beams of special structural walls of concrete and shear reinforcement.	X	—	ACI 318: 18.2.8, 25.5.7, 26.6.4, 26.13.1.4, 26.13.3.2, 26.13.3.3	
e. Shear reinforcement.	X	—		
3. Inspect anchors cast in concrete.	—	X	ACI 318: 17.8.2, 26.7.2, 26.8.2, 26.13.1, 26.13.3.3	—
4. Inspect <i>and</i> test anchors post-installed in hardened concrete members. ^b				
a. Adhesive anchors installed in horizontally or upwardly inclined orientations to resist sustained tension loads.	X	—	ACI 318: 17.8.2.4, 26.7.2, 26.13.1, 26.13.3.2 ACI 318: 17.8.2, 26.7.2, 26.13.1, 26.13.3.3	1705A.3.8, 1910A.5, [DSA-SS/CC] 1909.2.7
b. Mechanical anchors and adhesive anchors not defined in 4.a.	—	X		1705A.3.8, 1910A.5, [DSA-SS/CC] 1909.2.7
5. Verify use of required design mix.	—	X	ACI 318: Ch. 19, 26.4, 26.13.3.2	1903A.5, 1903A.6, 1903A.7, 1904A.1, 1904A.2, 1910A.1, [DOSHDP 1 & 4] 1908A.1, [DSA-SS/CC] 1909.2.1, 1909.2.2, 1909.2.3
6. Prior to <i>and</i> during concrete placement, fabricate specimens for strength tests, perform slump and air content tests, and determine the temperature of the concrete.	X	—	ASTM C31, ASTM C172, ACI 318: 26.4, 26.5, 26.12	1705A.3.5, 1705A.3.6, 1705A.3.9, 1905A.1.17, [DSA-SS/CC] 1909.3.9
7. Inspect concrete and shotcrete placement for proper application techniques.	X	—	ACI 318: 26.5, 26.13, ACI 506: 3.4	1705A.3.9, 1905A.1.15, 1905A.1.16, [DSA-SS/CC] 1909.3.7, 1909.3.8
8. Verify maintenance of specified curing temperature and techniques.	—	X	ACI 318: 26.5.3-26.5.5, 26.13.3.3	
9. Inspect prestressed concrete for:				
a. Application of prestressing forces; and	X	—	ACI 318: 26.10.2, 26.13.1, 26.13.3.2	1705A.3.4
b. Grouting of bonded prestressing tendons.	X	—		
10. Inspect erection of precast concrete members.	—	X	ACI 318: 26.9, 26.13.1, 26.13.3.3	—
11. For precast concrete diaphragm connections or reinforcement at joints classified as moderate or high deformability elements (MDE or HDE) in structures assigned to Seismic Design Category D, E or F, inspect such connections and reinforcement in the field for:			ACI 318: 26.13.1.3	—
a. Installation of the embedded parts	X	—	ACI 550.5	
b. Completion of the continuity of reinforcement across joints.	X	—		
c. Completion of connections in the field.	X	—		
12. Inspect installation tolerances of precast concrete diaphragm connections for compliance with ACI 550.5.	—	X	ACI 318: 26.13.1.3	—
13. Verify in-situ concrete strength, prior to stressing of tendons in post-tensioned concrete and prior to removal of shores and forms from beams and structural slabs.	—	X	ACI 318: 26.10.2, 26.11.2, 26.13.3.3	
14. Inspect formwork for shape, location and dimensions of the concrete member being formed.	—	X	ACI 318: 26.11.1.2(b), 26.13.3.3	1908A.3, [DSA-SS/CC] 1909.4.3

For Sl: 1 Inch = 25.4 mm.

a. Where applicable, see Section 1705A.13.

b. Specific requirements for special inspection shall be included in the research report for the anchor issued by an approved source in accordance with 17.8.2 in ACI 318, or other qualification procedures. Where specific requirements are not provided, special inspection requirements shall be specified by the registered design professional and shall be approved by the building official prior to the commencement of the work.

c. Installation of all adhesive anchors in horizontal and upwardly inclined positions shall be performed by an ACI/CRSI Certified Adhesive Anchor Installer, except where the design tension on the anchors is less than 130 pounds and those anchors are clearly noted on the approved construction documents or where the anchors are shear dowels across cold joints in slabs on grade where the slab is not part of the lateral force-resisting system.

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PROJECT: 2022 SIERRA II COMPACT 16'-8" W/ MECH RM
**WEST CAMPUS
 SACRAMENTO, CALIFORNIA**
 SHEET TITLE: DESIGN CRITERIA AND CODE SUMMARY

PLAN SET#
WES01

DATE:
11/01/2023

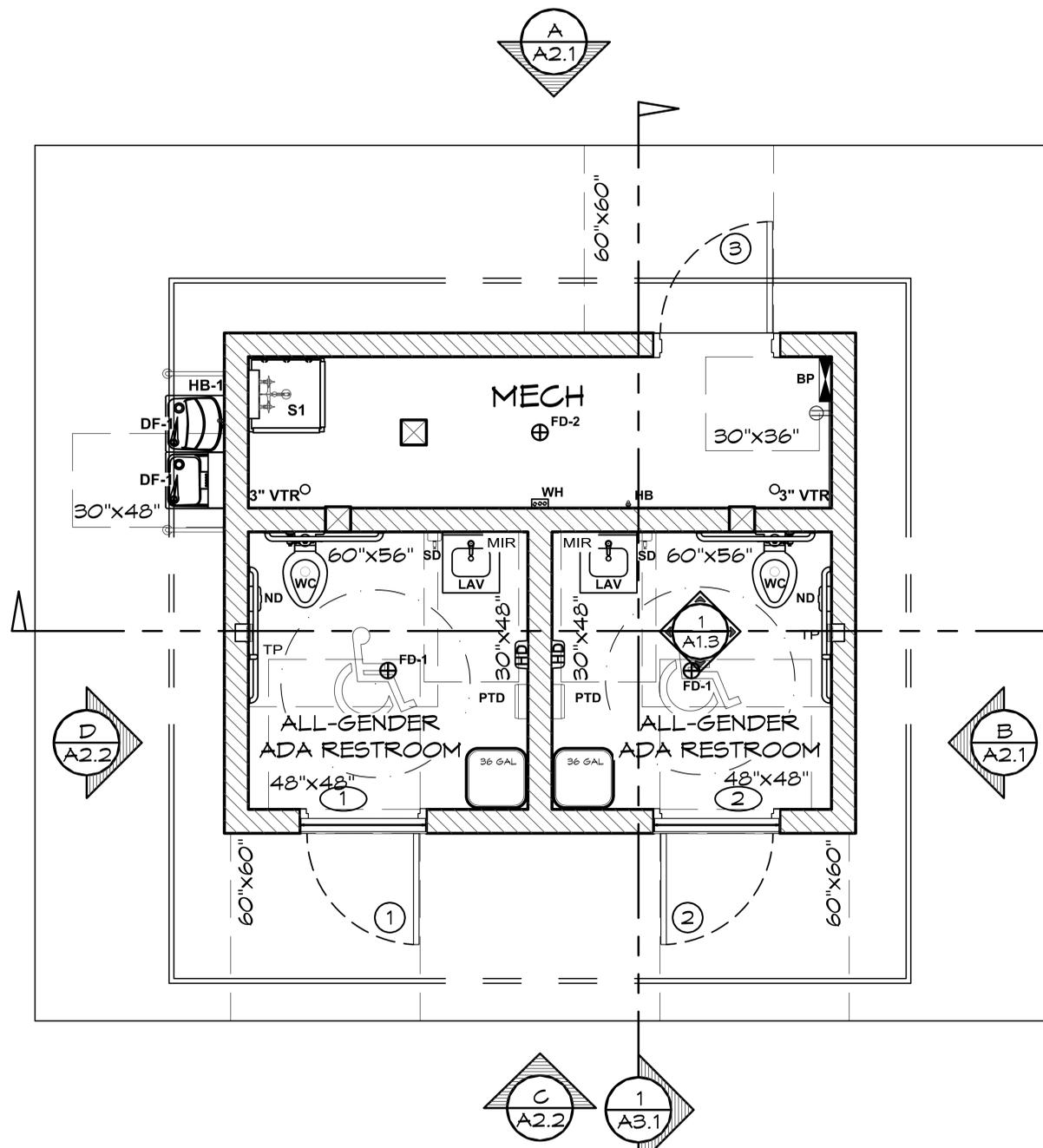
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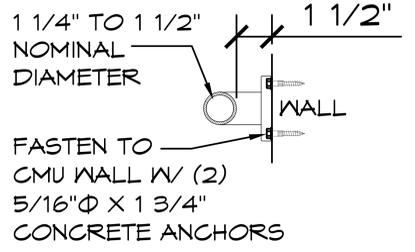
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SHEET NO.

G3



INSTALLED GRAB BARS MUST MEET 250# MINIMUM LOAD & GRAB BARS SHALL NOT ROTATE WITHIN THEIR FITTINGS PER CBC 2016 11B-609.

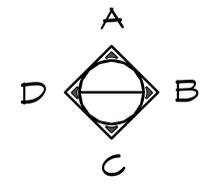


2 GRAB BAR
 SCALE: 1 1/2" = 1'-0"

INSTALL GRAB BARS PER CBC 11B-604.5

- MINIMUM LENGTH FOR THE GRAB BAR AT THE SIDE OF THE TOILET AS 42".
- THE 42" GRAB BAR AS LOCATED 12" MAXIMUM FROM THE REAR WALL.
- THE 42" GRAB BAR EXTENDING 54" MINIMUM FROM THE REAR WALL WITH THE FRONT END POSITIONED 24" MINIMUM IN FRONT OF THE WATER CLOSET.
- A MINIMUM LENGTH FOR THE GRAB BAR BEHIND THE TOILET AS 36".
- THE 36" GRAB BAR EXTENDING A MINIMUM OF 12" ON ONE SIDE OF THE TOILET.
- THE 36" GRAB BAR EXTENDING A MINIMUM OF 24" ON THE OTHER SIDE OF THE TOILET.
- THE GRAB BAR SHALL HAVE GRIPPING SURFACES WITH A DIAMETER OF 1-1/4 INCH TO 2 INCH LOCATED WITH A SPACE BETWEEN THE WALL AND GRAB BAR OF 1-1/2 INCH PER CBC 11B-609 .2.1.

1 ADA RESTROOM FIXTURE CLEAR FLOOR AREA
 SCALE: 1/4" = 1'-0"



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PROJECT: 2022 SIERRA II COMPACT 16'-8" W/ MECH RM

WEST CAMPUS
 SACRAMENTO, CALIFORNIA

SHEET TITLE: ADA CLEAR SPACE

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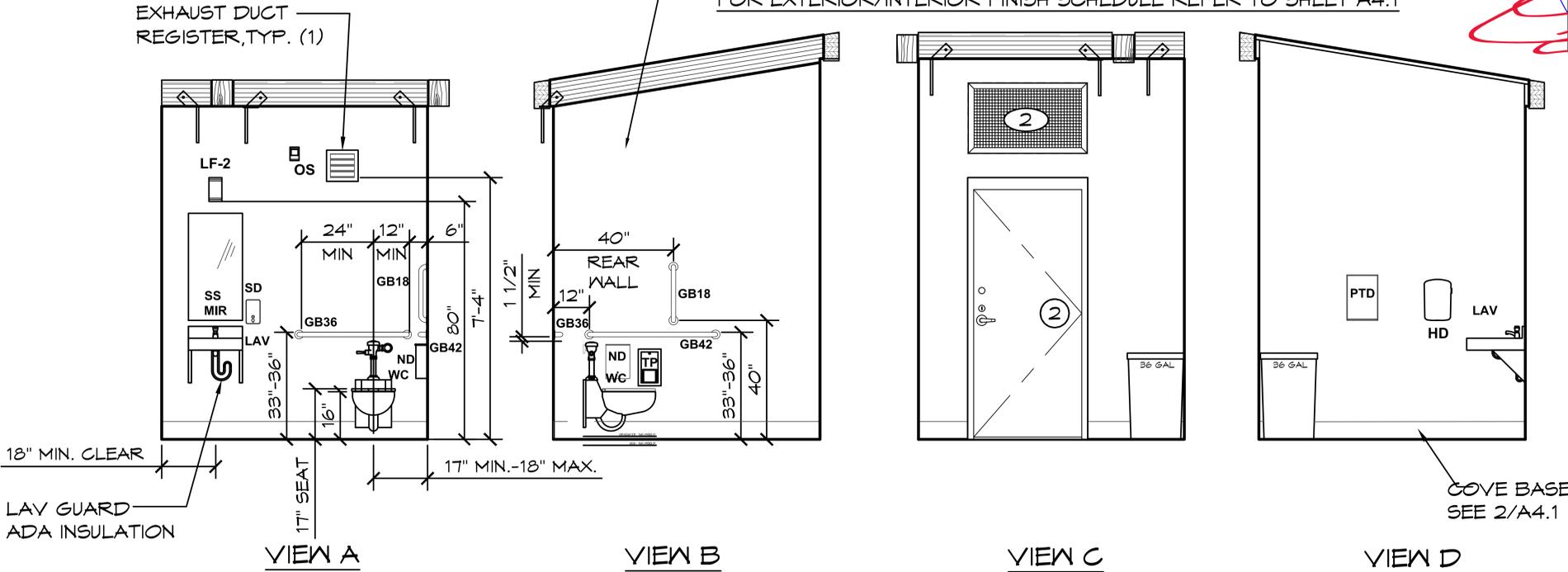


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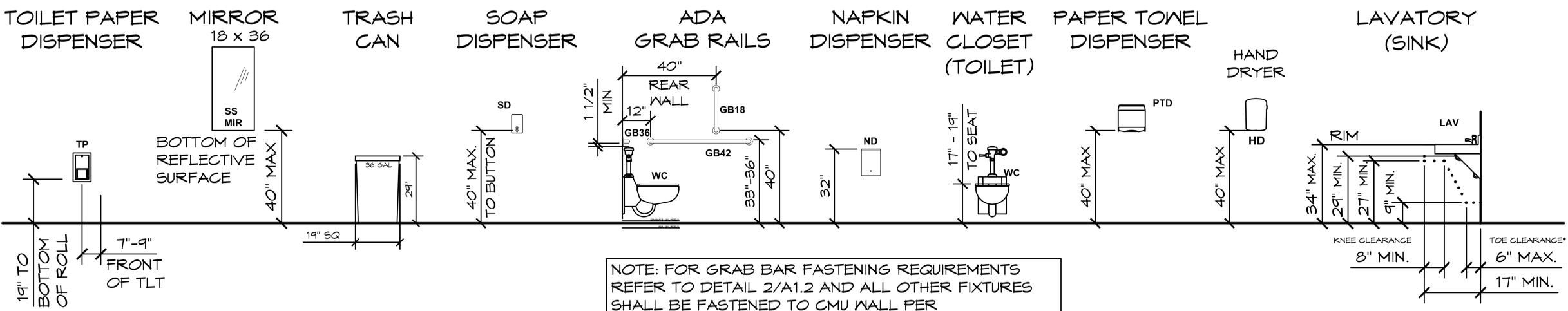
A1.2



WALLS AND PARTITIONS WITHIN 2 FEET OF SERVICE SINKS, URINALS AND WATER CLOSETS SHALL HAVE SMOOTH, HARD, NONABSORBENT SURFACE, TO A HEIGHT OF NOT LESS THAN 4 FEET ABOVE THE FLOOR PER CBC 2022 1210.2.2
 FLOORS AND WALL BASES SHALL HAVE A SMOOTH, HARD, NONABSORBENT VERTICAL BASE THAT EXTENDS UPWARD ONTO THE WALLS NOT LESS THAN 4 INCHES PER CBC 2022 1210.2.1.
 FOR EXTERIOR/INTERIOR FINISH SCHEDULE REFER TO SHEET A4.1



1 ADA RESTROOM INTERIOR ELEVATION VIEWS
 SCALE: 1/4" = 1'-0"



2 ADA RESTROOM FIXTURE DETAILS
 SCALE: 1/4" = 1'-0"

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PROJECT: 2022 SIERRA II COMPACT 16'-8" W/ MECH RM

WEST CAMPUS SACRAMENTO, CALIFORNIA

SHEET TITLE: INTERIOR ELEVATIONS

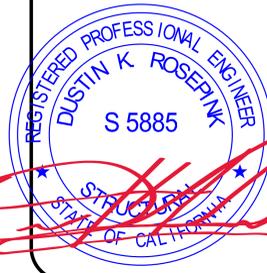
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EXHAUST FAN ROOF CAP W/
 6" COLLAR & BIRD SCREEN.
 SEAL PENETRATION W/
 METAL ROOF VENT SEALING
 ADAPTER MULTI-FLASH, TYP.

3" VTR W/ VENT
 BOOT, TYP. (2)

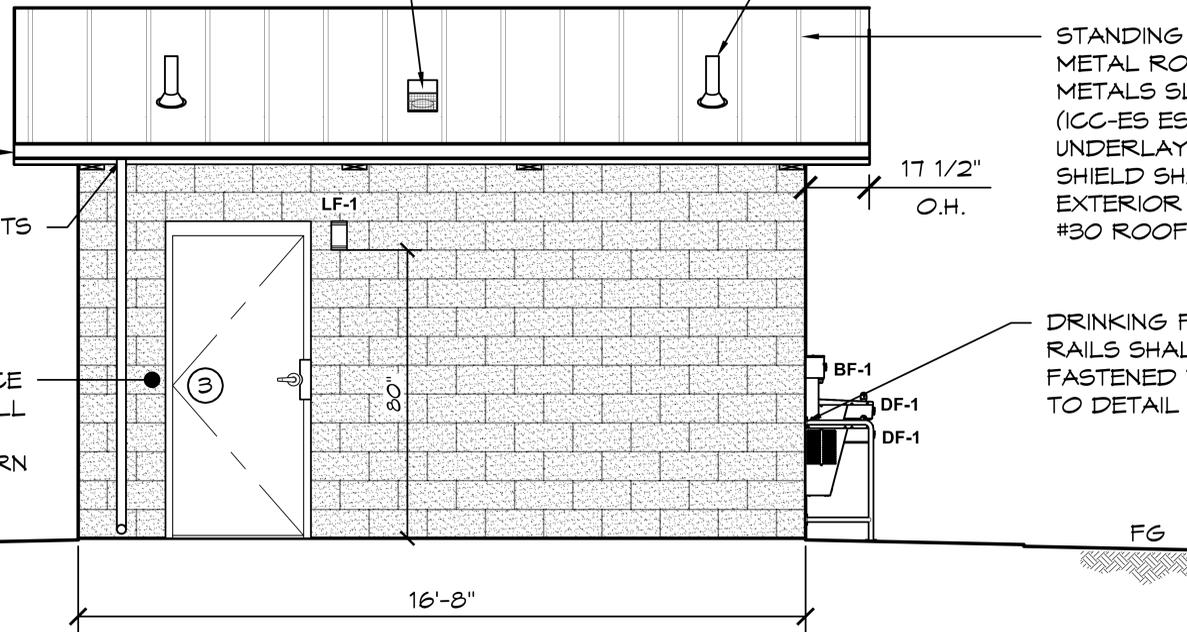
STANDING SNAP LOCKING SEAM
 METAL ROOFING CUSTOM-BILT
 METALS SL-1750-18" WIDTH PANEL
 (ICC-ES ESL-1491), OVER
 UNDERLAYMENT - ICE & WATER
 SHIELD SHALL EXTEND 24" PAST
 EXTERIOR WALL AT PERIMETER WITH
 #30 ROOFING FELT IN FIELD, TYP.

2X6 CEDAR
 FASCIA, TYP

GUTTERS & DOWNSPOUTS
 SHALL BE SECURELY
 FASTENED TO CMU
 (SUPPLIED BY
 INSTALLER), TYP.

8" X 16" CMU SPLIT FACE
 CONCRETE BLOCK WALL
 FULLY REINFORCED
 RUNNING BOND PATTERN

FG



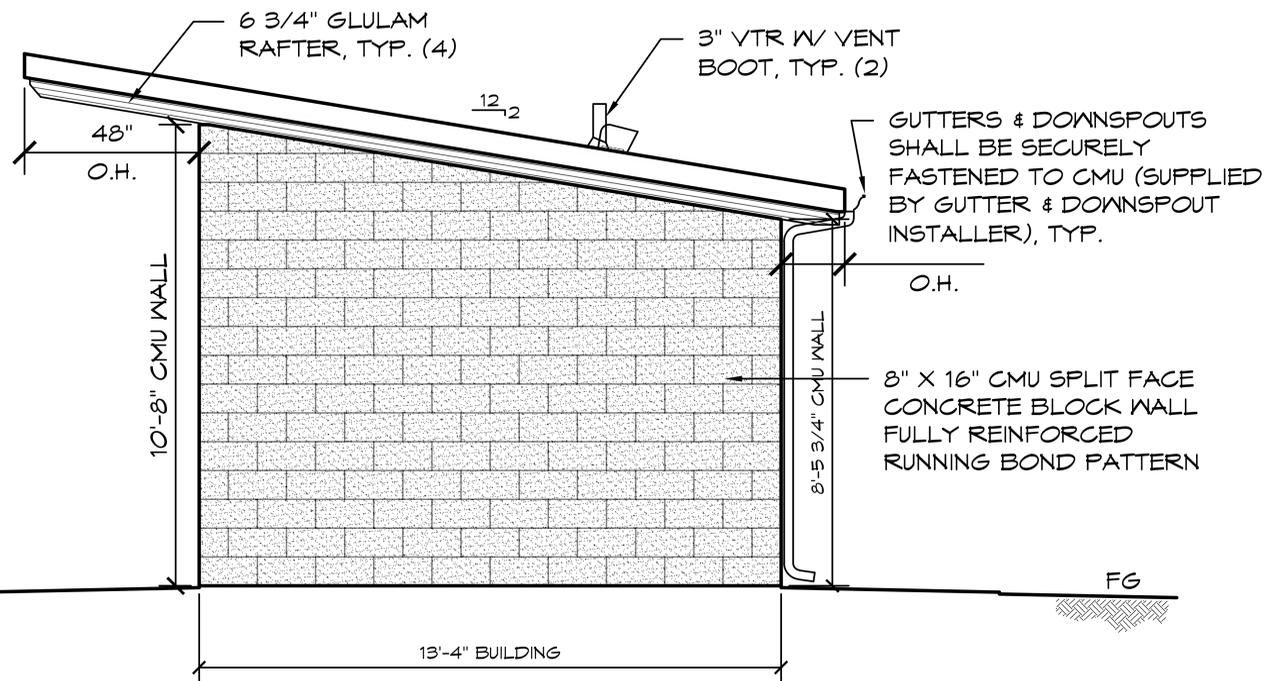
A ELEVATION VIEW
 SCALE: 1/4" = 1'-0"

FOR EXTERIOR/INTERIOR FINISH
 SCHEDULE REFER TO SHEET A4.1

NOTES:

- ③ 1. SEE SHEET A5.1 FOR DOOR LEGEND.

DRINKING FOUNTAIN GUARD
 RAILS SHALL BE SECURELY
 FASTENED TO CMU - REFER
 TO DETAIL 2/A1.2, TYP.



B ELEVATION VIEW
 SCALE: 1/4" = 1'-0"

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PROJECT: 2022 SIERRA II COMPACT 16'-8" W/ MECH RM
 WEST CAMPUS
 SACRAMENTO, CALIFORNIA
 SHEET TITLE: EXTERIOR ELEVATIONS

PLAN SET#
NES01

DATE:
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3	02-15-2024	CR
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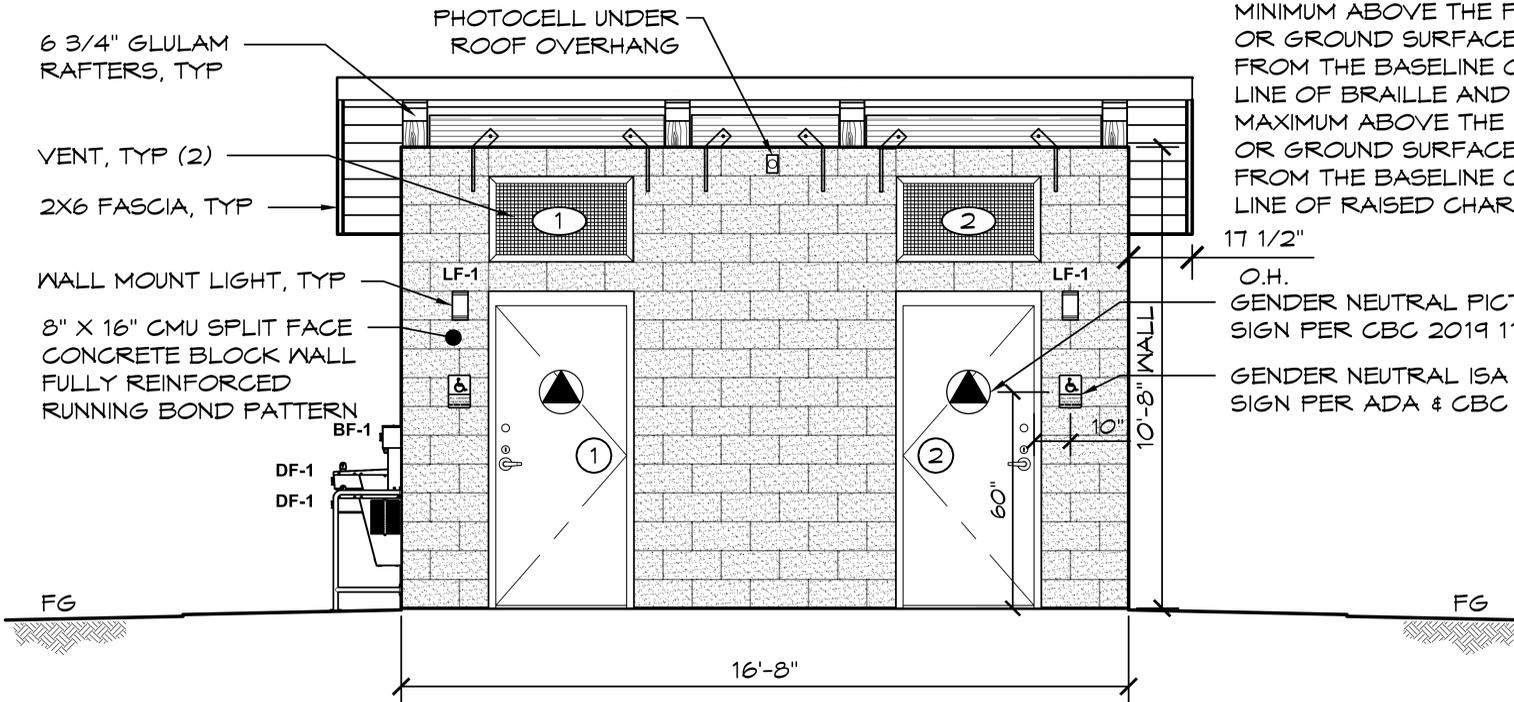
SHEET NO.

A2.1



SIGNS WITH RAISED CHARACTERS AND BRAILLE SHALL BE LOCATED 48" MINIMUM ABOVE THE FINISHED FLOOR OR GROUND SURFACE, MEASURED FROM THE BASELINE OF THE LOWEST LINE OF BRAILLE AND 60 INCHES MAXIMUM ABOVE THE FINISHED FLOOR OR GROUND SURFACE, MEASURED FROM THE BASELINE OF THE HIGHEST LINE OF RAISED CHARACTERS.

FOR EXTERIOR/INTERIOR FINISH SCHEDULE REFER TO SHEET A4.1



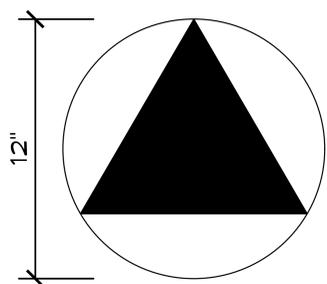
O.H.
 GENDER NEUTRAL PICTORIAL ADA SIGN PER CBC 2019 11B-703.7.2.6
 GENDER NEUTRAL ISA WITH BRAILLE SIGN PER ADA & CBC 2019 11B-703

NOTES:

- ① ② 1. SEE SHEET A5.1 FOR DOOR LEGEND.
- ① ② 2. SEE SHEETS A6.1 FOR VENT LEGEND.

C ELEVATION VIEW

SCALE: 1/4" = 1'-0"



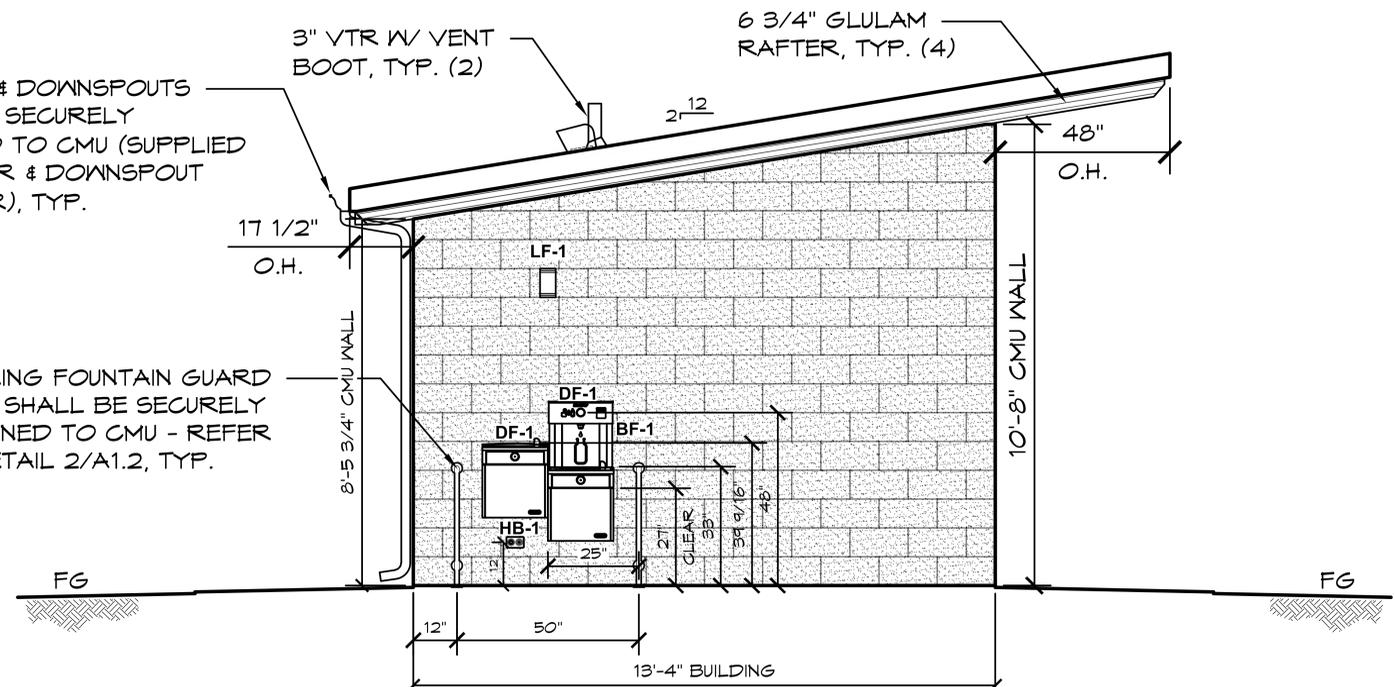
NOTE:
 ALL GENDER SANITARY FACILITIES SIGN SHALL BE IDENTIFIED BY A CIRCLE, 1/4" THICK AND 12" INCHES IN DIAMETER WITH 1/4" THICK TRIANGLE SUPERIMPOSED ON THE CIRCLE AND WITHIN THE 12" DIAMETER



NOTES:
 A) CHARACTERS ON SIGNS SHALL BE RAISED 1/32" INCH MINIMUM AND SHALL BE 'SANS SERIF' UPPERCASE CHARACTERS ACCOMPANIED BY GRADE II BRAILLE (WHERE REQUIRED). RAISED CHARACTERS SHALL BE A MINIMUM OF 5/8" INCH AND A MAXIMUM OF 2" INCHES HEIGHT.
 B) CONTRACTED GRADE II BRAILLE SHALL BE USED WHENEVER BRAILLE IS REQUIRED. DOTS SHALL BE 1/10" INCH ON CENTERS IN EACH CELL WITH A 2/10" INCH SPACE BETWEEN CELLS, MEASURED FROM THE SECOND COLUMN OF DOTS IN THE FIRST CELL TO THE FIRST COLUMN OF DOTS IN THE SECOND CELL. DOTS SHALL BE A RAISED A MINIMUM OF 1/40" (0.025) INCH ABOVE THE BACKGROUND.

GUTTERS & DOWNSPOUTS SHALL BE SECURELY FASTENED TO CMU (SUPPLIED BY GUTTER & DOWNSPOUT INSTALLER), TYP.

DRINKING FOUNTAIN GUARD RAILS SHALL BE SECURELY FASTENED TO CMU - REFER TO DETAIL 2/A1.2, TYP.



D ELEVATION VIEW

SCALE: 1/4" = 1'-0"

1 ADA SIGNS DETAIL

SCALE: 3" = 1'-0"

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WEST CAMPUS
 SACRAMENTO, CALIFORNIA
 PROJECT: 2022 SIERRA II COMPACT 16'-8" W/ MECH RM

PLAN SET#
NES01

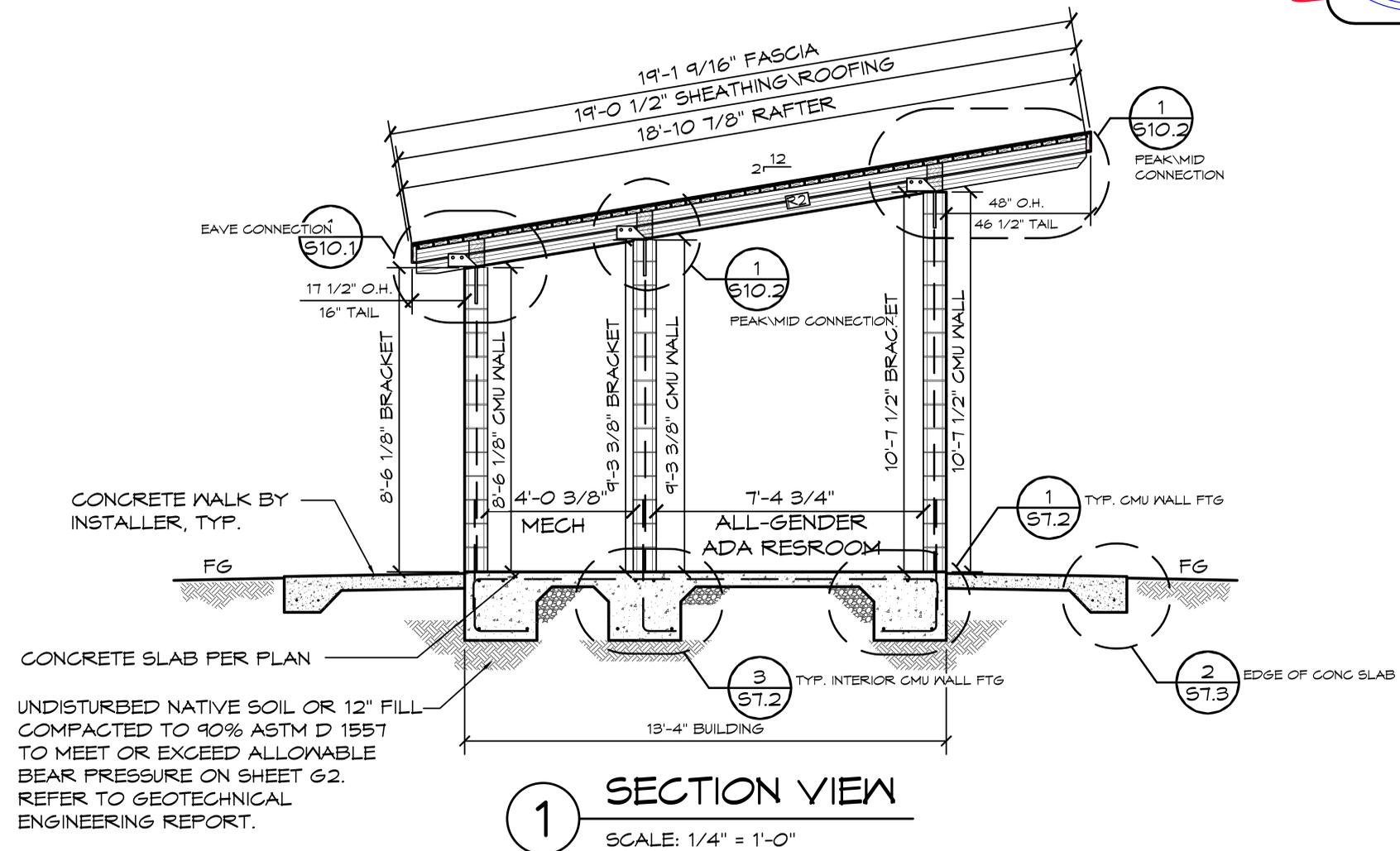
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1 SECTION VIEW
 SCALE: 1/4" = 1'-0"

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PROJECT: 2022 SIERRA II COMPACT 16'-8" W/ MECH RM
 WEST CAMPUS
 SACRAMENTO, CALIFORNIA
 SHEET TITLE: BUILDING SECTIONS

PLAN SET# NES01
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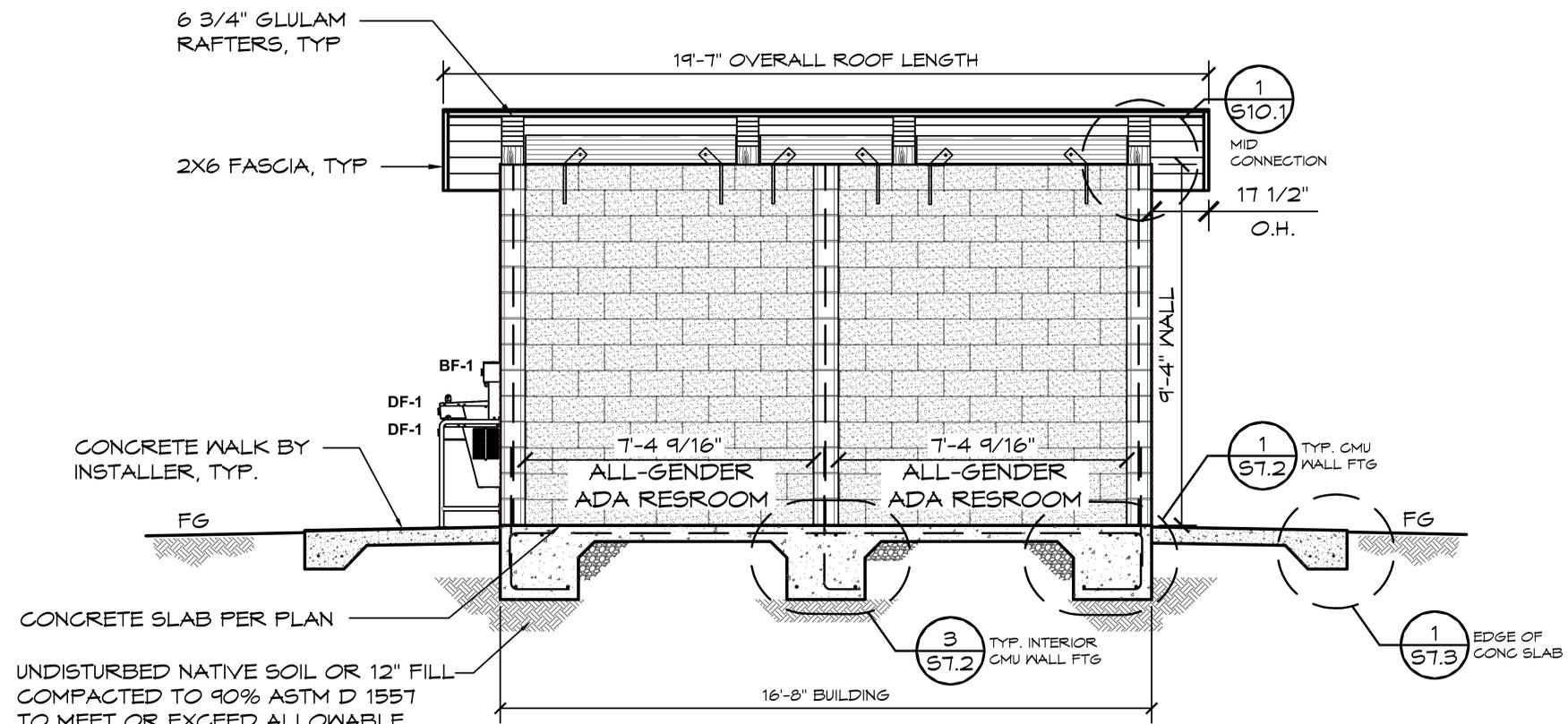
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1 SECTION VIEW
 SCALE: 1/4" = 1'-0"

CONCRETE WALK BY INSTALLER, TYP.
 FG
 CONCRETE SLAB PER PLAN
 UNDISTURBED NATIVE SOIL OR 12" FILL
 COMPACTED TO 90% ASTM D 1557
 TO MEET OR EXCEED ALLOWABLE
 BEAR PRESSURE ON SHEET G2.
 REFER TO GEOTECHNICAL
 ENGINEERING REPORT.

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PROJECT: 2022 SIERRA II COMPACT 16'-8" W/ MECH RM
**WEST CAMPUS
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SHEET NO. **A3.2**