

# CIVIL ABBREVIATIONS AND LEGEND

CO

CR

DWG

**ESMT** 

**HDPE** 

STD

W/

W/O

POST INDICATOR VALVE

PUBLIC UTILITY EASEMENT

STORM DRAIN MANHOLE

SANITARY SEWER MANHOLE

TRENCH DRAIN CATCH BASIN

TOP OF RAMP ELEVATION

TOP OF RETAINING WALL

TOP OF WALK ELEVATION

UNLESS OTHERWISE NOTED

SUBGRADE ELEVATION

REINFORCED CONCRETE PIPE

MANHOLE RIM ELEVATION (SOLID COVER)

REDUCED PRESSURE BACKFLOW PREVENTER

POLYVINYL CHLORIDE

PROPERTY LINE

RIGHT OF WAY

STORM DRAIN

SANITARY SEWER

SCHEDULE

**STANDARD** 

SIDEWALK

UTILITY

WATER

WITHOUT

WATER VALVE

WITH

TELEPHONE

TOP OF CURB

TRENCH DRAIN

TELEPHONE POLE

TOP OF SEAT WALL

VITRIFIED CLAY PIPE

UNDERGROUND

POWER POLE

**ABBREVIATIONS** NOTE: NOT ALL ABBREVIATIONS MAY BE USED ON THESE PLANS. AGGREGATE BASE ASPHALTIC CONCRETE AREA DRAIN ASSESSOR'S PARCEL NUMBER AIR RELEASE VALVE AGGREGATE SUB-BASE BLOW-OFF VALVE **BUTTERFLY VALVE** BACK OF WALK **CENTERLINE** CATCH BASIN CLASS CORRUGATED METAL PIPE CATV CABLE TELEVISION **CLEANOUT** COMM COMMUNICATION CONC. CONCRETE CONST. CONSTRUCT CURB RETURN CONCRETE SURFACE DOUBLE CHECK VALVE DOUBLE DETECTOR CHECK VALVE DECOMPOSED GRANITE DROP INLET PAD = 99.33DIAMETER DUCTILE IRON PIPE DRAWING DOWNSPOUT ELECTRIC EDGE OF PAVEMENT EASEMENT **EXISTING** FIRE SERVICE LINE FIRE DEPARTMENT CONNECTION FLOWLINE SANITARY SEWER FORCE MAIN FINISHED FLOOR ELEVATION FIRE HYDRANT GRATE ELEVATION GRADE ELEVATION GATE VALVE HOSE BIBB HEADER BOARD HIGH DENSITY POLYETHYLENE PIPE HIGH POINT PIPE INVERT ELEVATION JOINT UTILITY POLE LINEAL FEET LIP OF GUTTER LEFT **MOWSTRIP** NOT TO SCALE OVERHEAD PORTLAND CEMENT CONCRETE PLANTER DRAIN

#### <u>LEGEND</u> NOTE: NOT ALL SYMBOLS MAY BE USED ON THESE PLANS. PROPOSED GRADING & DRAINAGE SYMBOLS: 8" SD STORM DRAIN LINE (SIZE AND FLOW SHOWN) STORM DRAIN MANHOLE (SDMH) ——— CATCH BASIN (CB) ——— DROP INLET (DI) —— AREA DRAIN (AD) PLANTER DRAIN (PD) OR FLOOR DRAIN (FD) STORM DRAIN CLEANOUT ELEVATION FINISHED FLOOR ELEVATION BUILDING PAD ELEVATION CONCRETE SIDEWALK GRADED DIRECTION FOR DRAINAGE FLOW $\longrightarrow$ ---- SWALE TREE TO BE REMOVED RETAINING WALL PROPOSED SANITARY SEWER SYMBOLS: 6" SS SANITARY SEWER LINE (SIZE AND FLOW SHOWN) SANITARY SEWER MANHOLE (SSMH) SEWER CLEANOUT FLUSHER BRANCH PROPOSED WATER SYMBOLS: 8" RW RECLAIMED WATER LINE & SIZE 8" IRR IRRIGATION SERVICE LINE & SIZE 8" NP NON POTABLE WATER LINE & SIZE 8" SP FIRE SPRINKLER SERVICE LINE & SIZE <del>───</del> GATE VALVE ———M——— WATER METER → → → FH FIRE HYDRANT ASSEMBLY FIRE DEPARTMENT CONNECTION DETECTOR CHECK VALVE

DOUBLE DETECTOR CHECK VALVE

BACKFLOW PREVENTER

AIR RELEASE VALVE + SIZE

BLOW-OFF VALVE + SIZE

REDUCED PRESSURE

BUTTERFLY VALVE

POST INDICATOR VALVE

#### **DEMOLITION GENERAL NOTES**

- IN THE EVENT THAT ANY UNUSUAL CONDITIONS NOT COVERED BY THE GEOTECHNICAL INVESTIGATION REPORT OR ARE ENCOUNTERED DURING GRADING OPERATIONS THE GEOTECHNICAL ENGINEER AND THE ARCHITECT SHALL BE IMMEDIATELY NOTIFIED FOR DIRECTIONS.
- 2. NO BURNING OR BLASTING SHALL BE PERMITTED.
- ADDITIONAL DEMOLITION INFORMATION MAY BE SHOWN ON THE GRADING, DRAINAGE, AND UTILITY PLANS, AND THOSE PLANS PREPARED BY OTHER DISCIPLINES FOR THIS PROJECT.
- 4. ALL DEMOLISHED ITEMS SHALL BE DISPOSED OF OFFSITE AT A SUITABLE, LEGAL, DUMP SITE OR OTHER FACILITY.
- 5. ALL DISPOSED OF MATERIALS SHALL BE RECYCLED IF POSSIBLE
- 6. THE TYPES, LOCATIONS, SIZES AND/OR DEPTHS OF EXISTING UNDERGROUND UTILITIES AS SHOWN IN THESE PLANS WERE OBTAINED FROM SOURCES OF VARYING RELIABILITY. THE CONTRACTOR IS CAUTIONED THAT ONLY ACTUAL EXCAVATION WILL REVEAL THE TYPES, EXTENT, SIZES, LOCATIONS, AND DEPTHS OF SUCH UNDERGROUND UTILITIES. A REASONABLE EFFORT HAS BEEN MADE TO LOCATE AND DELINEATE ALL KNOWN UNDERGROUND UTILITIES. HOWEVER, WARREN CONSULTING ENGINEERS CAN ASSUME NO RESPONSIBILITY FOR THE COMPLETENESS OR ACCURACY OF ITS DELINEATION OF SUCH UNDERGROUND UTILITIES, NOR FOR THE EXISTENCE OF OTHER BURIED OBJECTS OR UTILITIES WHICH MAY BE ENCOUNTERED BUT WHICH ARE NOT SHOWN ON THESE DRAWINGS. THE CONTRACTOR OR ANY SUBCONTRACTOR FOR THIS CONTRACT SHALL NOTIFY THE DISTRICT TWO (2) WORKING DAYS IN ADVANCE OF PERFORMING ANY EXCAVATION WORK IN ORDER TO VERIFY TO THE GREATEST EXTENT POSSIBLE THE EXISTING UTILITY LINES, CONFLICTS AND PROPOSED UTILITY CONNECTION POINTS.
- 7. THE SCHOOL DISTRICT SHALL HAVE SALVAGE RIGHTS TO ANY DEMOLISHED ITEMS SHOWN HEREON. THE CONTRACTOR SHALL GIVE THE DISTRICT NOTICE 7 DAYS PRIOR TO THE START OF DEMOLITION. THE DISTRICT SHALL MOVE ANY RETAINED ITEMS OUT OF THE CONTRACTORS WORK AREA, UNLESS ANOTHER ARRANGEMENT IS MADE WITH THE CONTRACTOR. ANY REMAINING ITEMS BECOME THE PROPERTY OF THE CONTRACTOR AND SHALL BE REMOVED FROM THE SITE. ANY ITEMS NOT SHOWN FOR REMOVAL SHALL REMAIN AND SHALL BE PROTECTED FROM DAMAGE DURING CONSTRUCTION TO A REASONABLE
- 8. EXISTING UTILITY STRUCTURES IN AREAS OF NEW PAVING SHALL BE REMOVED AND REPLACED WITH NEW BOX/COVER AT NEW GRADE UNLESS SPECIFICALLY NOTED OTHERWISE.
- 9. ITEMS OUTSIDE THE LIMITS OF DEMOLITION SHALL REMAIN AND BE PROTECTED FROM DAMAGE DURING CONSTRUCTION.
- 10. EXISTING UTILITY STRUCTURES AND PIPING NOT SHOWN ON DEMOLITION PLAN TO BE REMOVED SHALL REMAIN AND BE PROTECTED.

#### UTILITY VERIFICATION NOTE

PRIOR TO THE START OF CONSTRUCTION, VERIFY AND POTHOLE ALL UTILITY POINTS OF CONNECTION FOR LOCATION DEPTH. AND SIZE, IF CONFLICT IS FOUND, CONTACT THE ENGINEER IMMEDIATELY FOR

#### IRRIGATION DEMOLITION NOTE

WITHIN LANDSCAPE AREAS TO BE DEMOLISHED THERE MAY BE EXISTING IRRIGATION LINES NOT SHOWN ON THIS PLAN. CONTRACTOR SHALL REMOVE LATERAL LINES AND HEADS ENCOUNTERED. MAIN LINES AND CONTROL WIRES MAY ONLY BE REMOVED PROVIDED THAT ROUTING IS KNOWN AND REMOVAL WILL NOT DEACTIVATE AN IRRIGATION SYSTEMS INTENDED TO REMAIN. IF CONFLICT IS FOUND, CONTACT THE ENGINEER FOR DIRECTION.

#### **GENERAL NOTES:**

APPLICATION.

 THE TYPES, LOCATIONS, SIZES, AND/OR DEPTHS OF EXISTING UNDERGROUND UTILITIES AS SHOWN ON THESE PLANS WERE OBTAINED FROM SOURCES OF VARYING RELIABILITY THE CONTRACTOR IS CAUTIONED THAT ONLY ACTUAL EXCAVATION WILL REVEAL THE TYPES, EXTENT, SIZES, LOCATIONS AND DEPTHS OF SUCH UNDERGROUND UTILITIES. A REASONABLE EFFORT HAS BEEN MADE TO LOCATE AND DELINEATE ALL KNOWN UNDERGROUND UTILITIES. HOWEVER. WARREN CONSULTING ENGINEERS CAN ASSUME NO RESPONSIBILITY FOR THE COMPLETENESS OR ACCURACY OF ITS DELINEATION OF SUCH UNDERGROUND UTILITIES, NOR FOR THE EXISTENCE OF OTHER BURIED OBJECTS OR UTILITIES WHICH MAY BE ENCOUNTERED BUT WHICH ARE NOT SHOWN ON THESE PLANS. THE CONTRACTOR OR ANY SUBCONTRACTOR FOR THIS CONTRACT SHALL NOTIFY MEMBERS OF UNDERGROUND SERVICE ALERT (USA) TWO (2) WORKING DAYS IN ADVANCE OF

PERFORMING ANY EXCAVATION WORK BY CALLING TOLL FREE 1—800—227—2600, OR 811.



- . WARREN CONSULTING ENGINEERS, INC. (WCE) ASSUMES NO RESPONSIBILITY FOR ERRORS IN PHYSICAL LOCATION OF IMPROVEMENTS, HORIZONTAL OR VERTICAL, IF STAKED BY OTHERS. IN ADDITION, ANY SUCH ERRORS IN PHYSICAL LOCATION MAY AFFECT THE INTENDED DESIGN OF SUCH IMPROVEMENTS AND WCE CANNOT BE HELD RESPONSIBLE FOR SUCH CONDITIONS WHICH ARE A RESULT OF ERRORS IN SURVEYING, OR IMPROPER CONSTRUCTION.
- 3. IF SUBSURFACE CULTURAL RESOURCES, REMAINS, AND/OR ARTIFACTS ARE UNCOVERED DURING PROJECT CONSTRUCTION, ALL WORK IN THE VICINITY SHALL BE STOPPED UNTIL SUCH ITEMS CAN BE ASSESSED BY AN APPROPRIATE MEMBER OF THE COUNTY ENVIRONMENTAL IMPACT SECTION STAFF.
- 4. CONTRACTOR AGREES THAT HE/SHE SHALL ASSUME SOLE AND COMPLETE RESPONSIBILITY FOR JOB SITE CONDITIONS DURING THE COURSE OF CONSTRUCTION OF THIS PROJECT, INCLUDING SAFETY OF ALL PERSONS AND PROPERTY: THAT THIS REQUIREMENT SHALL APPLY CONTINUOUSLY AND SHALL NOT BE LIMITED TO NORMAL WORKING HOURS: AND THAT THE CONTRACTOR SHALL DEFEND, INDEMNIFY AND HOLD THE OWNER AND ENGINEER HARMLESS FROM ANY AND ALL LIABILITY, REAL OR ALLEGED, IN CONNECTION WITH THE PERFORMANCE OF WORK ON THIS PROJECT, EXCEPTING FOR LIABILITY ARISING FROM THE SOLE NEGLIGENCE OF THE OWNER OR ENGINEER.
- 5. THE CONTRACTOR SHALL OBTAIN AN EXCAVATION PERMIT FROM THE STATE OF CALIFORNIA DEPARTMENT OF INDUSTRIAL SAFETY FOR ALL EXCAVATIONS OF 5 FEET OR MORE IN DEPTH.
- 6. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO MAKE ALL NECESSARY PRE-BID AND PRE-CONSTRUCTION SITE INSPECTION, AND/OR OBSERVATIONS ON THE SITE TO PRE-DETERMINE ALL HIS/HER MEANS AND METHODS NECESSARY TO COMPLETE THE IMPROVEMENTS SHOWN ON THESE PLANS AND PER THE PROJECT SPECIFICATIONS. IT IS THE CONTRACTORS RESPONSIBILITY TO DETERMINE, AND INCLUDE IN HIS/HER CONTRACT, ALL MEANS AND METHODS NECESSARY TO PERFORM A COMPLETE AND ACCEPTABLE JOB.
- '. WHERE IMPROVEMENTS LIE WITHIN AN EXISTING DEVELOPED AREA, CONTRACTOR SHALL USE CAUTION WHEN ACCESSING THE SITE THROUGH THESE EXISTING IMPROVEMENTS. IT IS THE CONTRACTORS RESPONSIBILITY TO PROTECT ANY SUCH EXISTING IMPROVEMENTS OUTSIDE THE PROJECT BOUNDARY. OR EXISTING IMPROVEMENTS WITHIN THE BOUNDARY WHICH ARE TO REMAIN. PROPER PRECAUTIONS SHALL BE PROVIDED AND MAINTAINED THROUGHOUT CONSTRUCTION. ANY DAMAGE SHALL BE REPAIRED OR REPLACED TO THE SATISFACTION OF THE
- 8. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO KEEP DETAILED RECORDS OF MINOR CHANGES OR ADJUSTMENTS MADE DURING CONSTRUCTION (WHICH WERE NOT FORMALLY ISSUED). UPON PROJECT COMPLETION, THESE RECORDS AND/OR INFORMATION SHALL BE PROVIDED TO THE OWNER AND WARREN CONSULTING ENGINEERS, INC. UNLESS AN OFFICIAL "AS-BUILT" SET OF PLANS IS A REQUIREMENT OF THE CONTRACT. IF AS-BUILT PLANS ARE A REQUIREMENT OF THE CONTRACT, REFER TO SPECIFICATIONS FOR AS-BUILT DELIVERABLE REQUIREMENTS.
- 9. IN VEHICULAR PATHWAYS, EXISTING ASPHALTIC AND/OR CONCRETE SURFACES SHALL BE CUT TO A NEAT AND STRAIGHT LINE, PARALLEL OR PERPENDICULAR TO THE VEHICULAR TRAVELED PATH. THIS IS TYPICALLY THE ROADWAY CENTERLINE, BUT MAY VARY. THAT SAWCUT EDGE SHALL BE PROTECTED FROM DAMAGE DURING CONSTRUCTION SO A CLEAN EDGE REMAINS FOR PATCH BACK.. IF EDGE IS DAMAGED, A NEW SAW CUT WILL BE REQUIRED. THE EXPOSED EDGE SHALL BE "TACKED" WITH EMULSION PRIOR TO PAVING.
- 10. NO BURNING OR BLASTING SHALL BE ALLOWED ONSITE UNLESS SPECIFICALLY ADDRESSED ON PLANS, OR SPECIFICALLY APPROVED AND COORDINATED WITH THE ARCHITECT, ENGINEER, AND LOCAL AGENCY OR OTHER ADMINISTRATIVE AUTHORITY.
- 11. SUBGRADE AND RESULTING FINISHED GRADE SHALL BE CONSTRUCTED SMOOTH AND UNIFORM BETWEEN SPOT ELEVATIONS, CONTOURS OR OTHER STRUCTURE ELEVATIONS SHOWN ON GRADING OR OTHER PLANS. NO MOUNDS, RUTS, DEPRESSIONS OR OTHER GRADING DEFICIENCIES WILL BE ALLOWED UNLESS SPECIFICALLY SHOWN ON PLANS.
- 12. ON NEW WATER SYSTEMS, SERVICE LATERALS SHALL BE MADE USING APPROPRIATE "TEE" AND "WYE" FITTINGS. SADDLE TAPS WILL ONLY BE ALLOWED WHEN MAKING CONNECTIONS TO EXISTING WATER MAINS.
- 13. CURING COMPOUND SHALL BE APPLIED IN A CONTINUOUS SOLID WET FLOWING COAT. ANY "SPOTTY" APPLICATIONS SHALL BE RECOATED IMMEDIATELY. APPLICATION SHALL BE INSPECTED BY PROJECT INSPECTOR DURING
- 14. EMBEDMENT OF FEATURES IN CONCRETE PAVING, CURBS, OR WALLS, SUCH AS SQUARE OR ROUND TUBING, POSTS, OR COLUMNS, STEEL BOLTED PLATES, OR OTHER STRUCTURES, SHALL REQUIRE ADDITIONAL SCORE OR EXPANSION JOINTS TO PREVENT UNCONTROLLED CRACKING. THOSE ADDITIONAL JOINTS MAY OR MAY NOT BE SPECIFICALLY SHOWN ON PLANS BUT SHALL BE PROVIDED BY THE CONTRACTOR.
- 15. EMBEDMENT OF FEATURES IN CONCRETE PAVING, CURBS, OR WALLS, SUCH AS SQUARE OR ROUND TUBING, POSTS, OR COLUMNS, STEEL BOLTED PLATES, OR OTHER STRUCTURES, SHALL REQUIRE A MINOR ADJUSTMENT OF REBAR WITHIN CONCRETE TO ALLOW FOR SUCH STRUCTURE. THAT REBAR ADJUSTMENT MAY NOT BE SPECIFICALLY SHOWN
- 16. NO MORE THAN 1 GALLON OF WATER PER YARD OF CONCRETE CAN BE ADDED TO THE TRUCK AFTER ARRIVAL TO PROJECT SITE. THE ADDITION OF WATER CAN ONLY BE ADDED UNDER THE SUPERVISION OF THE CONCRETE INSPECTOR OR LABORATORY TECHNICIAN.
- 17. WHEN PUMPING CONCRETE FOR PLACEMENT, ABSOLUTELY NO WATER IS TO BE ADDED TO PUMP HOPPER. ANY WATER ADDED TO HOPPER WILL BE REASON FOR CONCRETE REJECTION AT THE CONTRACTORS EXPENSE.
- 18. ALL CONTRACTION/CONSTRUCTION JOINTS "CJ" SHALL BE 1/4 THE SLAB THICKNESS DEEP, BUT NO LESS THAN 1" FOR CONTROLLING OF CRACKING. CONTRACTOR SHALL EXERCISE CAUTION WHEN FINAL TROWELING OF CONCRETE SO AS NOT TO FILL IN THESE JOINTS WITH CONCRETE CREAM. ANY CRACKS OUTSIDE OF JOINTS WHICH WERE CONSTRUCTED LESS THAN 1" DEEP, SHALL BE CAUSE FOR CONCRETE SLAB(S) TO BE REMOVED AND REPLACE AT
- 19. ANY SCREED BOARDS SET WITHIN CONCRETE SLABS SHALL BE AN "OVERHEAD SCREED" SO THERE IS NO INTERFERENCE WITH THE PLACEMENT AND ALIGNMENT OF SLAB REINFORCING.
- 20. 3-1/2" FELT JOINTS WILL NOT BE ACCEPTED. PROVIDE A FULL 4" FELT JOINT FOR 4" SLAB CONSTRUCTION, AND A 6" FELT JOINT FOR A 6" SLAB SLAB CONSTRUCTION.
- 21. SHOULD ANY SHRINKAGE CRACKS OCCUR OUTSIDE OF EITHER THE EXPANSION JOINTS OR CRACK CONTROL JOINTS, THEN THE CONCRETE SLAB SHALL BE SAWCUT AT THE NEAREST JOINTS ON EACH SIDE OF THE CRACK AND THE CONCRETE SECTION SHALL BE, REMOVED AND REPLACED. NEW CONCRETE SHALL BE DOWELED INTO EXISTING CONCRETE PER DRAWING DETAIL.
- 22. ALL AREAS DISTURBED BY GRADING OPERATIONS WHETHER SHOWN ON THE DRAWINGS OR NOT SHALL BE HYDRO SEEDED UNLESS OTHERWISE NOTED. HYDRO SEEDING SHALL CONFORM TO LOCAL CITY/COUNTY STANDARDS.
- 23. REPAIR OR PATCHING OF GALVANIZED METALS, SUCH AS AFTER WELDING GALVANIZED COMPONENTS, SHALL BE MADE USING A ZINC COMPOSITION "HOT STICK" APPLICATION PER ASTM A 780-01. GALVANIZING PAINTS WILL NOT BE ALLOWED.

#### **GENERAL PAVING SURFACE NOTES:**

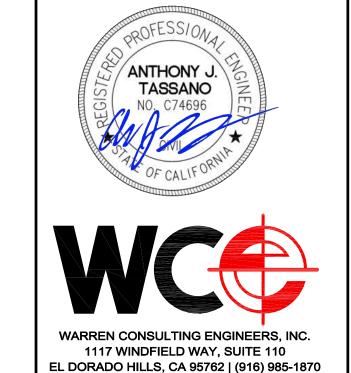
- 1. PROVIDE EQUIVALENT OF MEDIUM BROOM FINISH AT SLOPES UP TO 5.99%, TYPICAL. PROVIDE EQUIVALENT OF HEAVY BROOM FINISH AT SLOPES 6% AND GREATER. REFER TO SPECIFICATIONS.
- 2. ALL NEW PEDESTRIAN WALKWAYS (NON-RAMP) SHALL BE SLOPED NO GREATER THAN 2.0%, AND NO LESS THAN 0.75% IN ANY DIRECTION, UNLESS SPECIFICALLY LABELED OTHERWISE. ALL CONCRETE SHALL MEET THE FOLLOWING SLOPE REQUIREMENTS:
- NO GREATER THAN 5% SLOPE IN THE DIRECTION OF TRAVEL. - NO GREATER THAN 2% SLOPE CROSSING THE DIRECTION OF TRAVEL.

#### CIVIL SHEET INDEX

CO.1 CIVIL GENERAL NOTES AND ABBREVIATIONS

NO GREATER THAN 2% SLOPE IN ANY DIRECTION IN COURTYARD OR PLAZA AREAS.

- C1.1 DEMOLITION PLAN
- C2.1 GRADING AND PAVING PLAN
- C3.1 DETAILS AND SECTIONS



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**CIVIL GENERAL NOTES AND ABBREVIATIONS** 

PROJECT NO. 3/21/2022

= WATER BOX = IRRIGATION CONTROL VALVE = FIRE HYDRANT = BACKFLOW PREVENTER

= SPRINKLER = HOSE BIBB -OH-E- = OVERHEAD ELECTRIC LINE

---E = UNDERGROUND ELECTRIC LINE —————— = UNDERGROUND ELECTRIC LINE (RECORD INFORMATION)

— — E — — = UNDERGROUND ELECTRIC LINE (UNDERGROUND LOCATING) = ELECTRIC MANHOLE = UTILITY POLE (WITH GUY WIRE)

= ELECTRIC METER = ELECTRIC BOX = STREET LIGHTING BOX

 $\square$   $\square$   $\square$   $\square$   $\square$  = LIGHT STANDARD □ □ □ □ = SIGNAL LIGHT = FLOOD LIGHT = ELECTRICAL OUTLET

---G---= GAS LINE (RECORD INFORMATION) --G--= GAS LINE (UNDERGROUND LOCATING)

= GAS MANHOLE = GAS VALVE = GAS METER --- T --- = TELEPHONE LINE

---T---= TELEPHONE LINE (RECORD INFORMATION) -- T -- = TELEPHONE LINE (UNDERGROUND LOCATING) = STORM DRAIN BOX

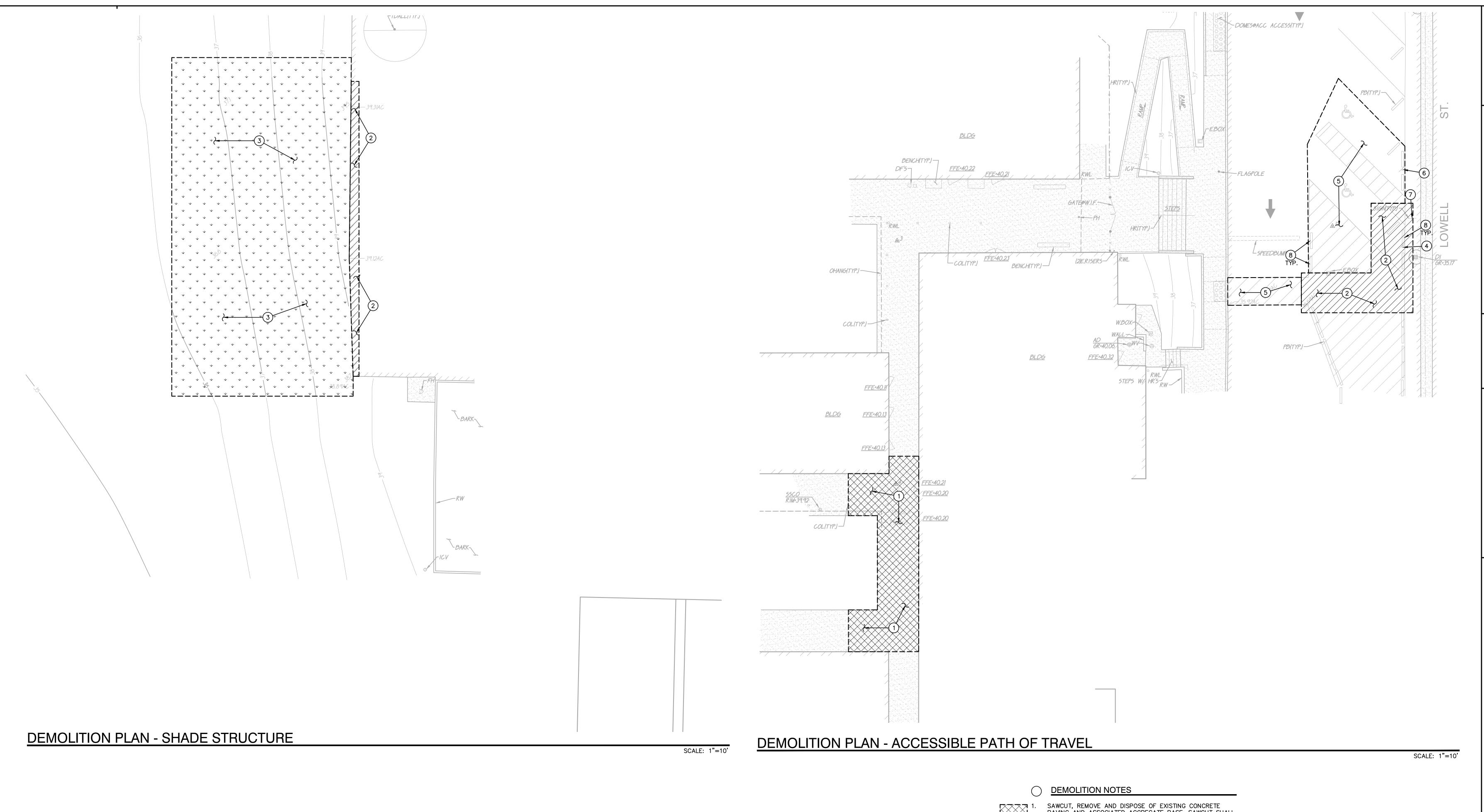
= TRAFFIC SIGNAL BOX

TBM LIST

NUMBER DESCRIPTION NORTHING EASTING ELEV 2 CPS CHISELED "+" 5219.36 5000.00 35.91 CPS CHISELED "+" 4997.13 4917.27 39.94 CPS CHISELED "+" 4936.01 4916.89 40.03 4833.42 4880.14 39.53 CPS MAGNAIL 12 CPS CHISELED "+" 5602.74 5333.28 33.89 15 CPS PK+WASHER 4827.07 5034.29 35.07 16 CPS CHISELED "+" 4858.53 4759.47 39.95 17 CPS PK+WASHER 5000.79 5026.55 35.42 18 CPS CHISELED "+" 5008.76 4756.93 40.04

19 CPS CHISELED "+" 4934.01 4760.33 39.93 20 CPS CHISELED "+" 5083.12 4770.18 39.94 21 CPS CHISELED "+" 5154.79 4781.83 39.91

22 CPS CHISELED "+" 5185.35 4662.12 40.62



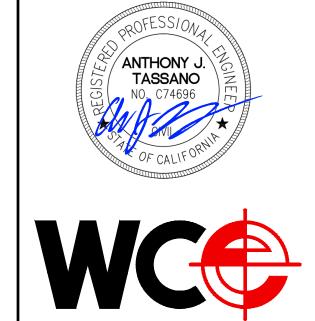
SAWCUT, REMOVE AND DISPOSE OF EXISTING CONCRETE PAVING AND ASSOCIATED AGGREGATE BASE. SAWCUT SHALL BE A NEAT STRAIGHT LINE, MAINTAIN CLEAN, STRAIGHT CUT EDGE UNTIL NEW PAVING IS PLACED.

SAWCUT, REMOVE AND DISPOSE OF EXISTING ASPHALT PAVING AND ASSOCIATED AGGREGATE BASE. SAWCUT SHALL BE A NEAT STRAIGHT LINE, MAINTAIN CLEAN, STRAIGHT CUT EDGE UNTIL NEW PAVING IS PLACED.

REMOVE AND DISPOSE OF EXISTING LANDSCAPING, TURF AND ASSOCIATED IRRIGATION PIPING/SPRINKLERS WITHIN AREAS OF WORK. CUT AND CAP ANY MAINLINES NEAR WHERE THEY ENTER THE BOUNDARY OF THE PROJECT. MARK ALL CAPPED LINES WITH AN IRRIGATION VALVE BOX. ALL EXISTING IRRIGATION AREAS OUTSIDE THE PROJECT WORK AREA SHALL BE PRESERVED AND OPERATIONAL. INTEGRITY SHALL BE MAINTAINED WITH PROPER SPRINKLER COVERAGE TO TURF AREAS TO REMAIN.

- 4. REMOVE AND DISPOSE OF EXISTING SIGN, POST AND ASSOCIATED FOOTINGS.
- 5. BLACK OUT EXISTING STRIPING.
- 6. CUT POST FLUSH WITH PAVEMENT AND REMOVE. GROUT FILL POST HOLE.
- 7. EXISTING SIGN TO REMAIN.
- 8. REMOVE AND SALVAGE EXISTING PARKING BUMPER.





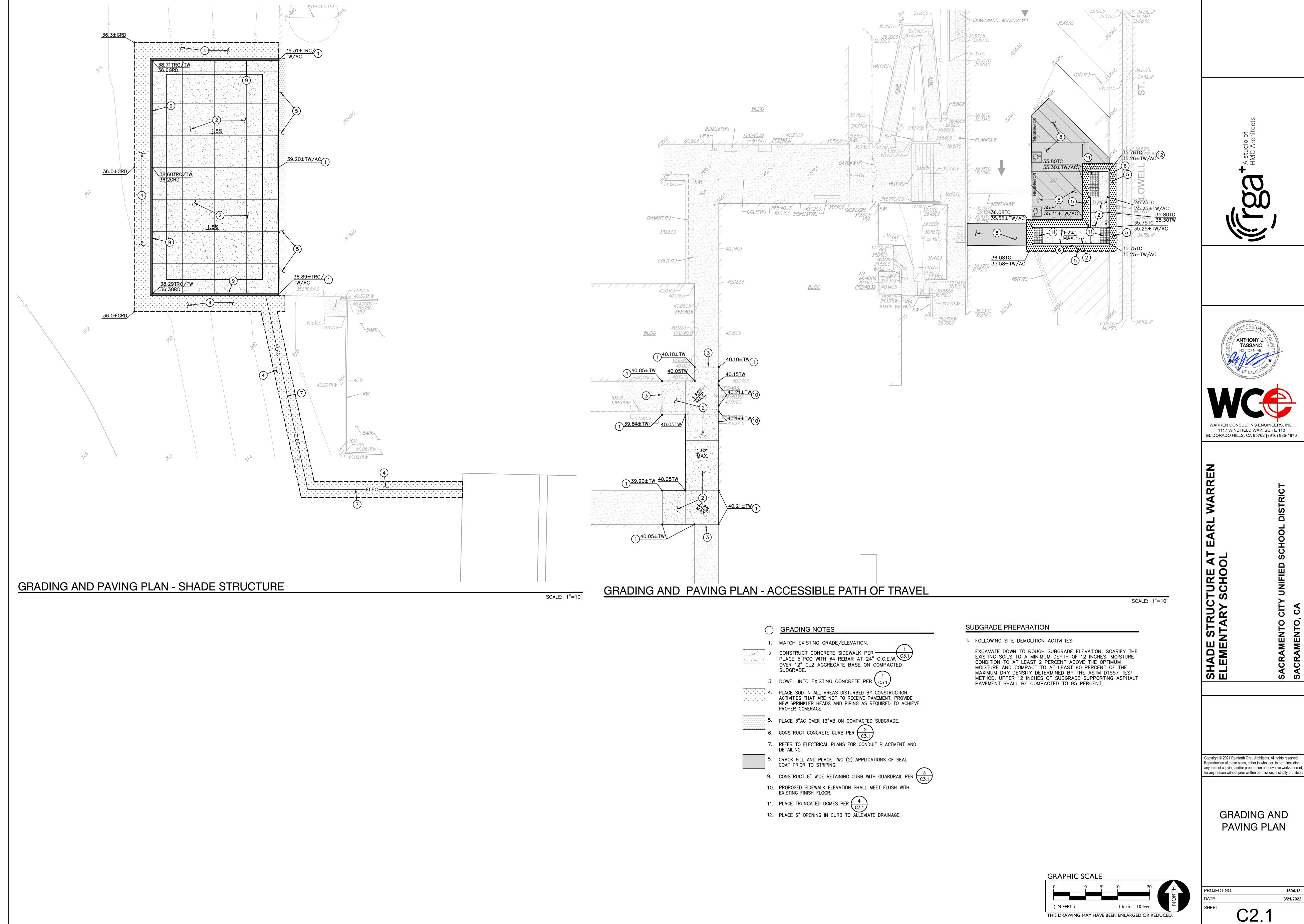
1117 WINDFIELD WAY, SUITE 110 EL DORADO HILLS, CA 95762 | (916) 985-1870

SHADE ST ELEMENT

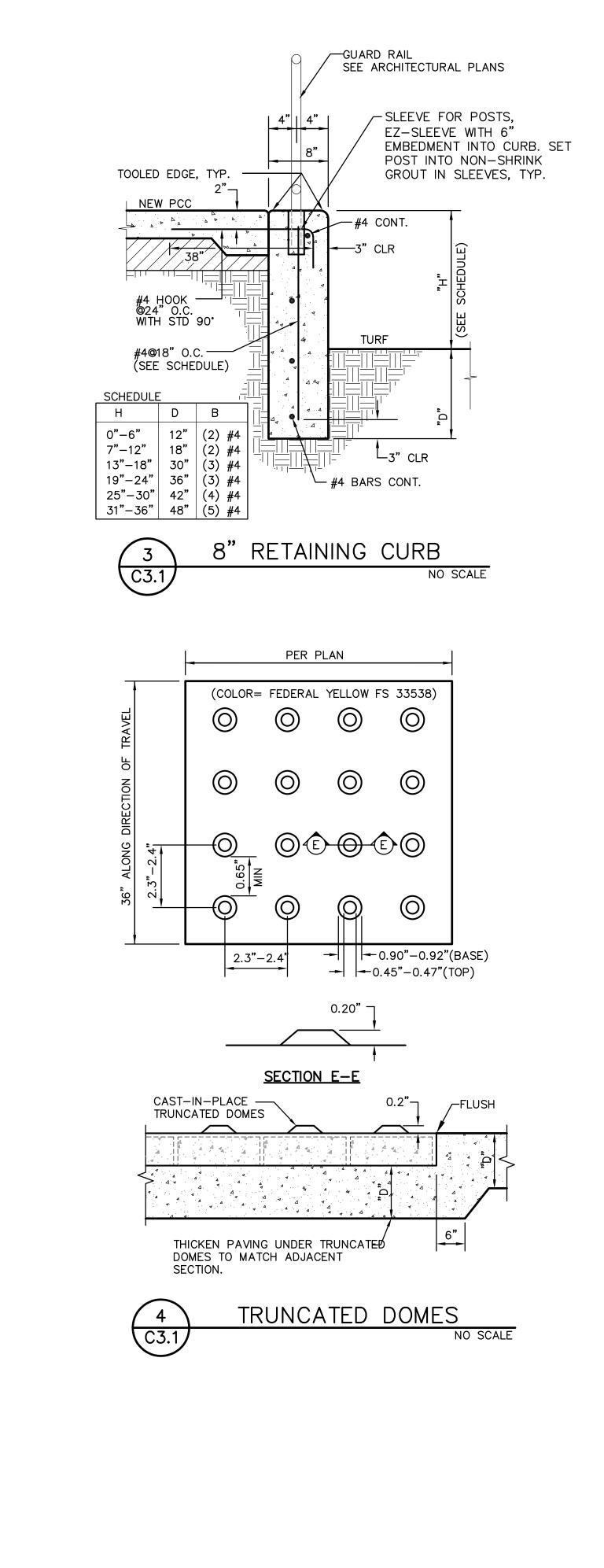
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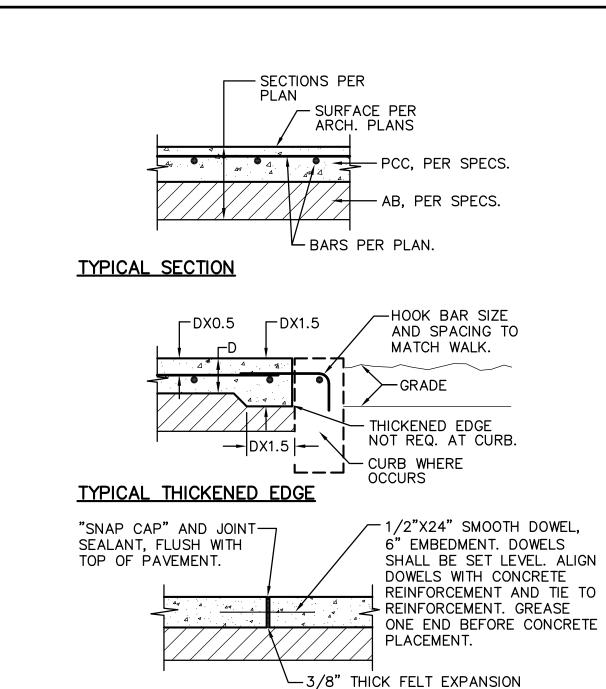
**DEMOLITION PLAN** 

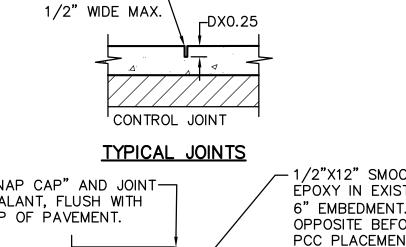
**GRAPHIC SCALE** THIS DRAWING MAY HAVE BEEN ENLARGED OR REDUCED.



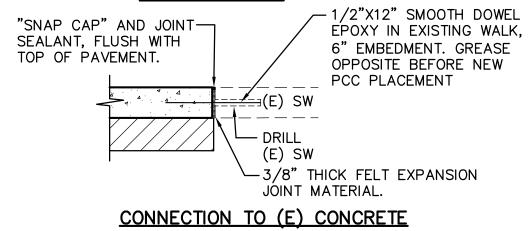
FILENAME:I:\22-044\CIVIL\EARL WARREN\DWG\22-044-C21WARREN.DWG







TOOLED JOINT



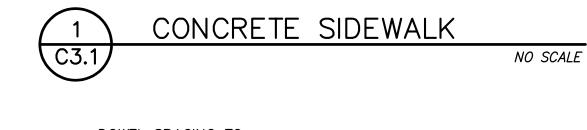
JÓINT MATERIAL.

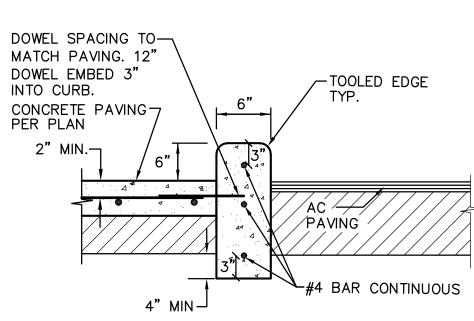
NOTES:

1. PROVIDE FELT EXPANSION JOINTS AT 20 FEET O.C. MAX.

2. PROVIDE CONTROL JOINTS AT 10 FEET O.C. MAX.

3. EXPANSION OR CONTROL JOINTS SHALL NOT EXCEED 1/2" IN SURFACE WIDTH.





NOTES:

1. PROVIDE FELT EXPANSION JOINTS (E.J.) AT 60 FEET O.C. MAXIMUM PROVIDE CONTROL JOINTS AT 10 FEET O.C. MAXIMUM, EXCEPT WHEN PLACING ADJACENT TO CONCRETE WALKS THE EXPANSION JOINTS SHALL ALIGN WITH THE EXPANSION JOINTS SHOWN FOR THE CONCRETE WALKS.

2. AT E.J. USE 1/2"X24" SMOOTH DOWELS, ALIGN WITH REBAR, GREASE 1/2 THE LENGTH BEFORE CONCRETE PLACEMENT.







SHADE STRUCTURE AT EARL WARREN ELEMENTARY SCHOOL

CITY

SACRAMENTO (

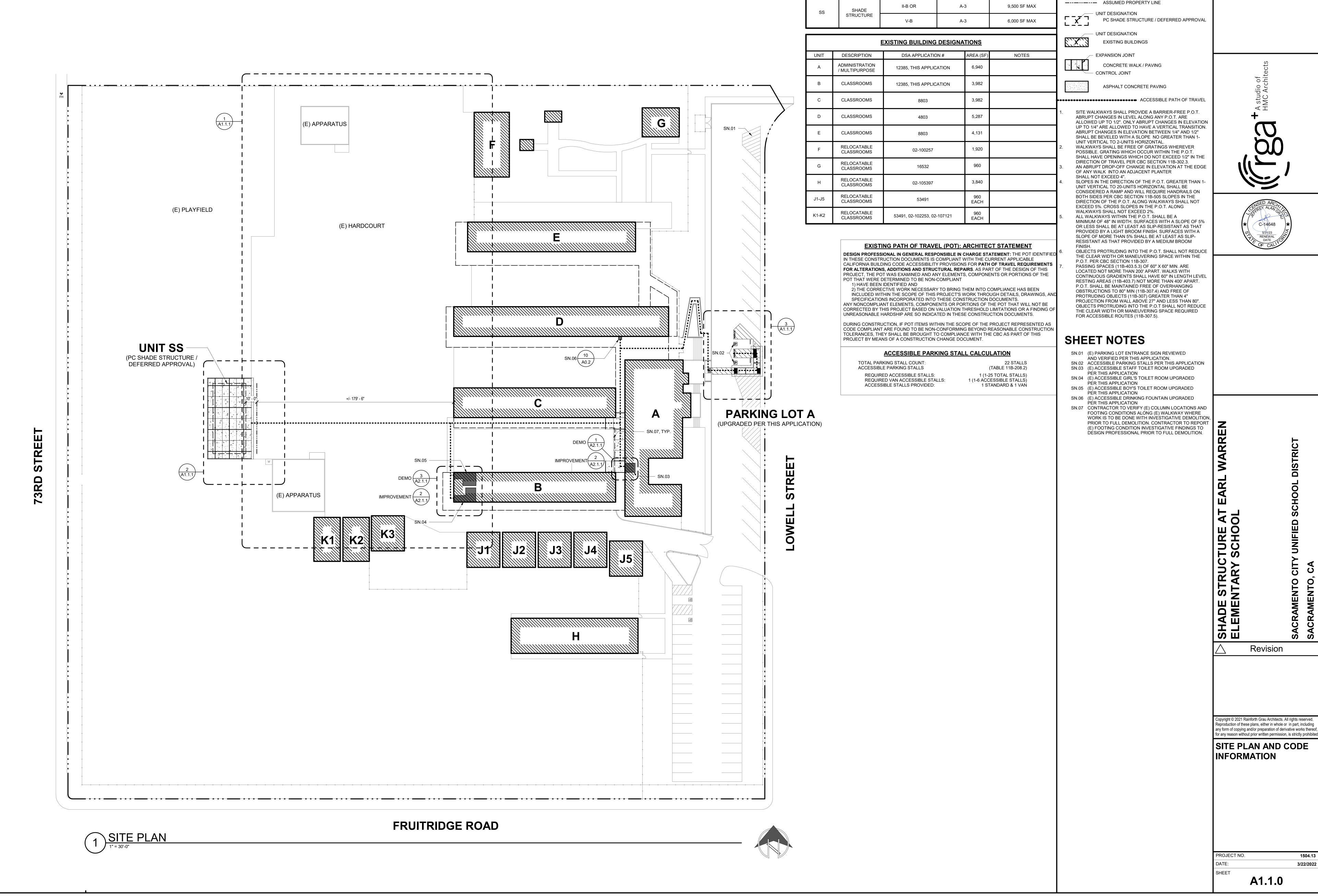
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DETAILS AND SECTIONS

PROJECT NO. 1504.13

DATE: 3/21/2022

SHEET C3.1



**LEGEND** 

ALLOWABLE

AREA

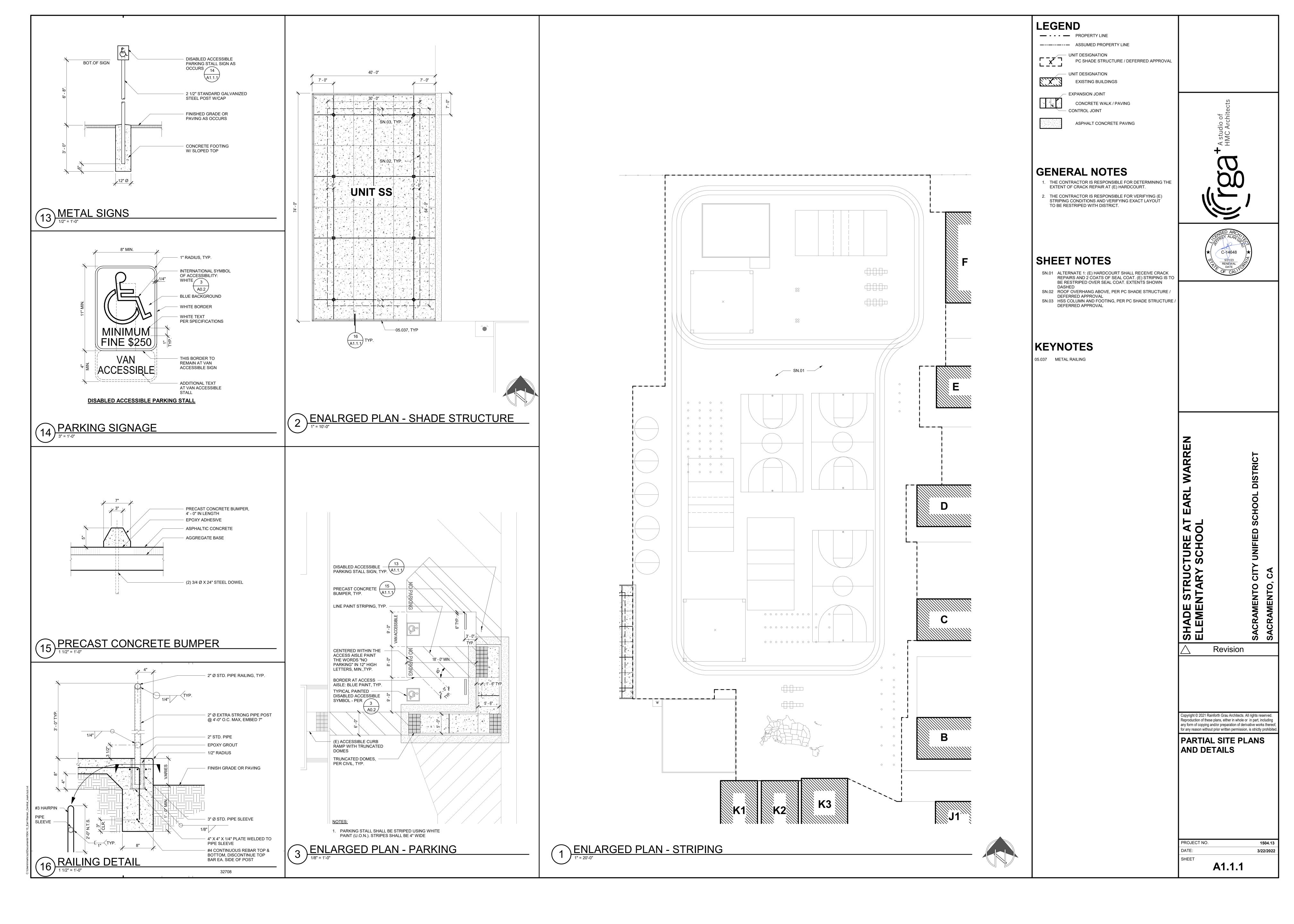
PROPOSED SHADE STRUCTURE

OCCUPANCY

**CONSTRUCTION TYPE** 

DESCRIPTION

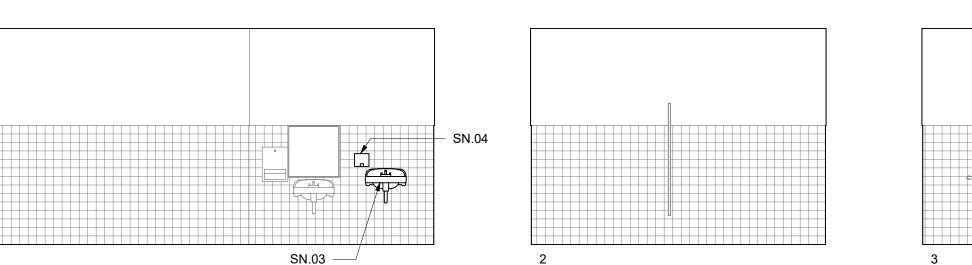
— • • • — PROPERTY LINE

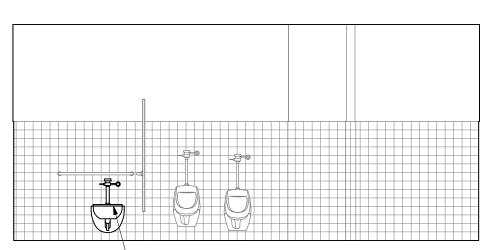


PLUMBING FIXTURE SCHEDULE - BASIS OF DESIGN						UTILITY CONNECTIONS						
SYMBOL	FIXTURE	DESCRIPTION	NOTES	VENT	WAS BRANCH	OUTLET	COLD BRANCH	WATER OUTLET		VATER OUTLET		
WC-1 (ADA)	WATER CLOSET FLUSH VALVE FLOOR MTD	"KOHLER" HIGHCLIFF ULTRA, MODEL K-96057, OR EQUAL, VITREOUS CHINA, ELONGATED, 1-1/2" TOP SPUD, 12" ROUGH-IN, 16-5/8" RIM HEIGHT, 1.28 GPF. FLUSH VALVE: "SLOAN" ROYAL OPTIMA 111-1.28	SEAT: "CHURCH" 295SSCT OR EQUAL, SELF-SUSTAINING CONCEALED CHECK HINGES, ONE PIECE SS POST HINGES, WHITE COLOR.  MOUNT FLUSH HANDLE ON WIDE SIDE OF WATER CLOSET ENCLOSURE.	2"	4"	4"	1-1/4"	1"	-			

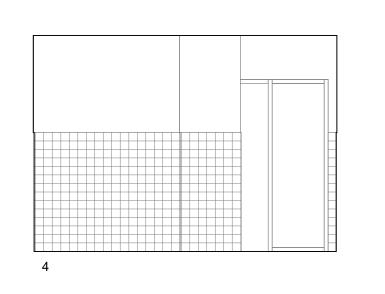
SN.14 SN.02 -SN.06 2 —22.040, SN.01

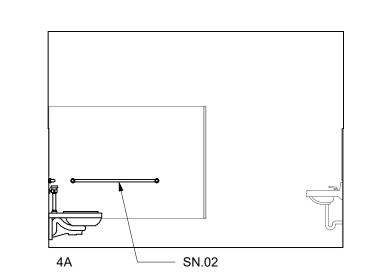
5 PLUMBING FIXTURE SCHEDULE





A101 - STAFF

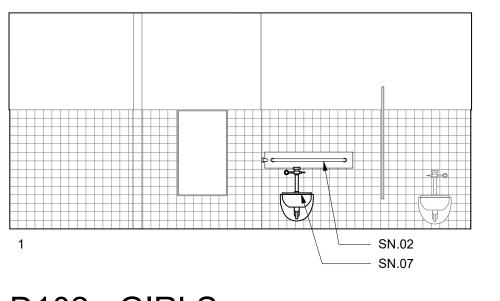


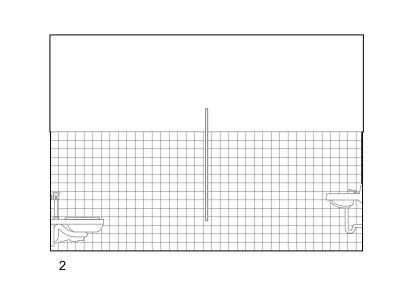


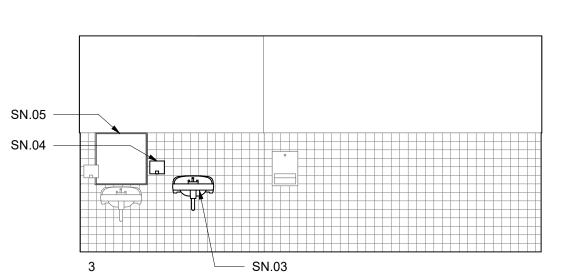
ADULT HEIGHT

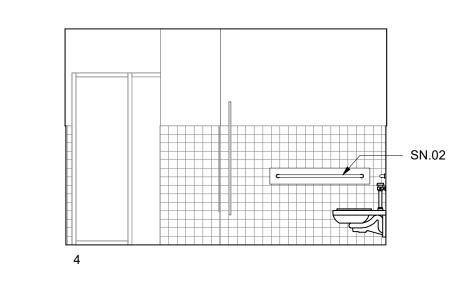
D101 - BOYS





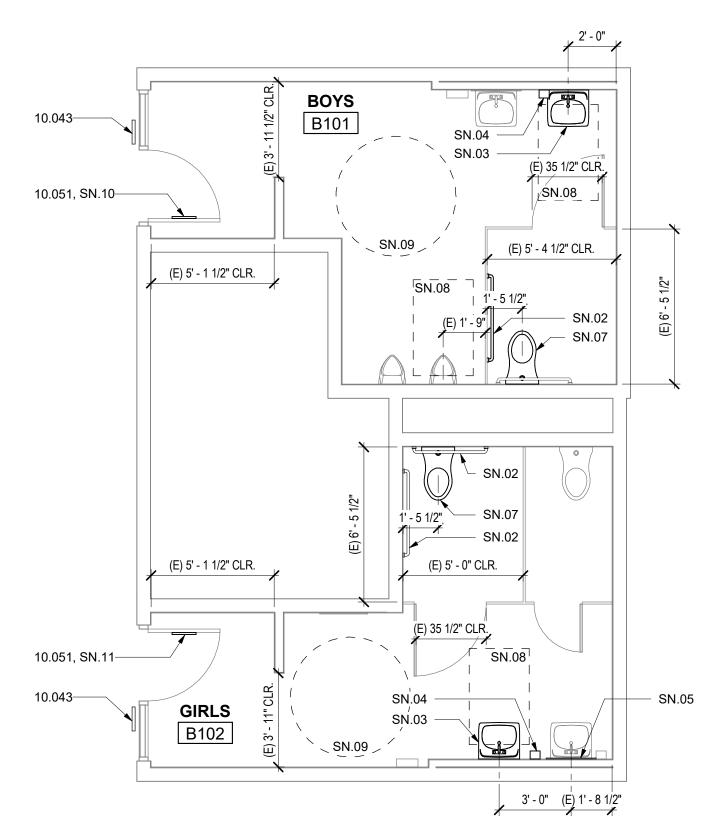


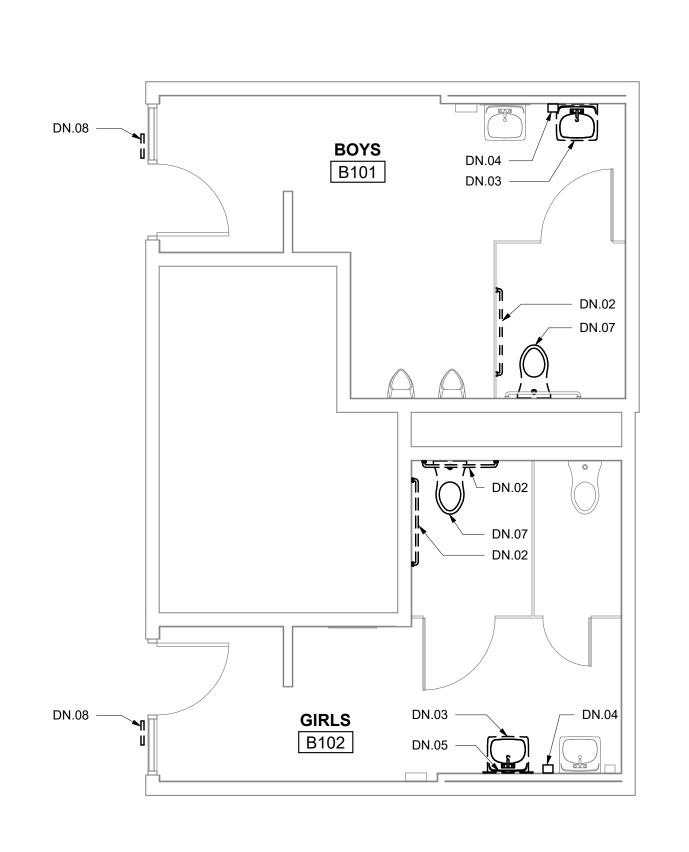


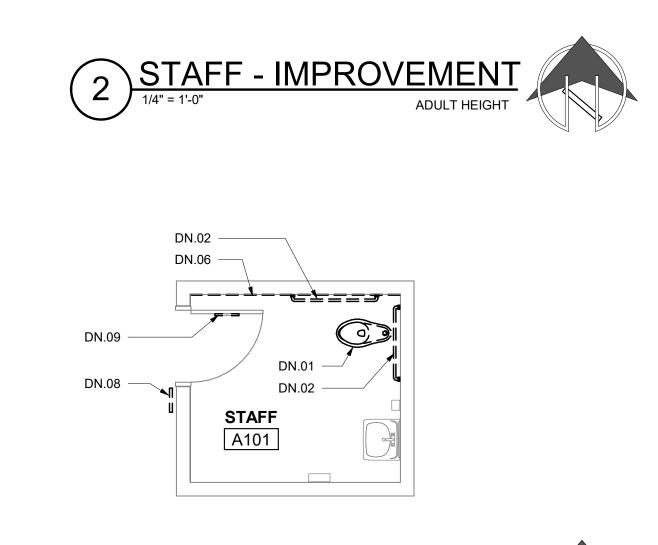


D102 - GIRLS

ADULT HEIGHT







(E) 8' - 8 1/2"

A101 SN.13

SN.09 SN.08









# **LEGEND**

CONSECUTIVE NUMBERING INTERIOR CONVENTION FOR INTERIOR **ELEVATIONS AND ROOM** ELEV. FINISHES.

# **GENERAL NOTES**

- FOR MOUNTING HEIGHTS, LOCATIONS, AND DETAILS, INCLUDING THOSE FOR DISABLED ACCESSIBITY, REFER TO SHEET A0.2
- PROTECT ALL ADJACENT SURFACES, ITEMS AND FINISHES NOT NOTED TO BE DEMOLISHED. EQUIPMENT/FIXTURES NOTED AS "SALVAGED FOR
- REINSTALLATION" WILL BE REMOVED AND STORED BY THE CONTRACTOR PRIOR TO START OF DEMOLITION. THESE EQUIPMENT/FIXTURES SHALL BE REINSTALLED BY THE CONTRACTOR UNDER THIS CONTRACT.
- REMOVE ALL ITEMS SCHEDULED TO BE REMOVED, INCLUDING MOUNTING HARDWARE.
- DEMO AND REPAIR WALL FINISH AS NECESSARY TO PERFORM FIXTURE AND EQUIPMENT WORK AS NOTED. ADJACENT FINISHES TO BE VERIFIED BY CONTRACTOR.

## **DEMOLITION NOTES**

- DN.01 REMOVE (E) FLOOR-MOUNTED WATER CLOSET DN.02 REMOVE (E) GRAB BARS AND SALVAGE FOR REINSTALLATION DN.03 REMOVE (E) LAVATORY AND SALVAGE FOR REINSTALLATION
- DN.04 REMOVE (E) SOAP DISPENSER AND SALVAGE FOR REINSTALLÁTION DN.05 REMOVE (E) MIRROR AND SALVAGE FOR REINSTALLATION
- DN.06 REMOVE (E) TILE FINISH FROM THIS WALL ONLY DN.07 REMOVE (E) WALL-MOUNTED WATER CLOSET AND SALVAGE FOR REINSTALLATION
- DN.08 REMOVE (E) TOILET ROOM I.D. SIGN DN.09 REMOVE (E) TOILET ROOM DOOR SYMBOL

#### SHEET NOTES

- SN.01 RECONNECT TO (E) WATER LINE, WASTE LINE AND VENT SN.02 REINSTALL (E) SALVAGED GRAB BARS TO COMPLY WITH A0.2 SN.03 REINSTALL (E) SALVAGED LAVATORY TO COMPLY WITH A0.2. ADJUST (E) WATER CARRIER AS REQUIRED FOR
- RECONNECTION TO LAVATORY. RECONNECT TO (E) WATER LINE, WASTE LINE AND VENT. SN.04 REINSTALL (E) SALVAGED SOAP DISPENSER TO COMPLY WITH A0.2
- SN.05 REINSTALL (E) SALVAGED MIRROR TO COMPLY WITH A0.2 SN.06 FURRED WALL PER A0.2
- SN.07 REINSTALL (E) SALVAGED WALL-MOUNTED WATER CLOSET TO COMPLY WITH A0.2. ADJUST (E) WATER CARRIER AS REQUIRED FOR RECONNECTION TO WATER CLOSET. RECONNECT TO (E) WATER LINE, WASTE LINE AND VENT.
- SN.08 30" X 48" CLEAR SPACE SN.09 60" DIA. TURNING CIRCLE SN.10 SIGN TO READ "BOYS" SN.11 SIGN TO READ "GIRLS"
- SN.12 SIGN TO READ "STAFF" SN.13 WRAP ALL EXPOSED PIPES WITH INSULATION AT LAVATORIES
- SN.14 REINSTALL (E) SALVAGED GRAB BAR TO COMPLY WITH A0.2 AND PER 12

# KEYNOTES

10.043 SIGNAGE: TOILET ROOM IDENTIFICATION 10.051 SIGNAGE: TOILET ROOM DOOR SYMBOL 22.040 WATER CLOSET

# TURE SHADE

Revision

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TOILET ROOM **DEMOLITION AND** IMPROVEMENT PLANS AND INTERIOR ELEVATIONS

UNITS A & D PROJECT NO. 3/22/2022 SHEET A2.1.1

#### ABBREVIATION LIST AMPERE ALTERNATING CURRENT AIR CONDITIONING ARC ENERGY REDUCTION AMP FRAME ABOVE FINISHED FLOOR AMPERES INTERRUPTING CAPACITY AMP TRIP SETTING AMERICAN WIRE GAUGE BARE COPPER BELOW FINISHED CEILING BREAKER BUILDING **BOOSTER POWER SUPPLY** CONDUIT CIRCUIT BREAKER CONTRACTOR FURNISHED. CONTRACTOR INSTALLED CIRCUIT CEILING CONDUIT ONLY, WITH PULL LINE CONT CONTINUOUS METALLIC COLD WATER PIPE DEMOLISH DIRECT CURRENT DISCONNECT DISTRIBUTION PANEL EXISTING EACH WITH **EVENING LIGHT** ELECTRIC EMERGENCY ELECTRICAL METALLIC TUBING END OF LINE DEVICE **EQUIPMENT** EXISTING RELOCATED ELECTRICAL WATER COOLER ELECTRIC WATER HEATER FIRE ALARM CONTROL PANEL FAEP FIRE ALARM EXTENDER PANEL FATC FIRE ALARM TERMINAL CABINET FURNISHED BY OTHERS **FLUOR** FLUORESCENT GROUND FAULT CIRCUIT INTERRUPT GENERAL LIGHTING ZONE METALLIC GAS PIPE GYPSUM HIGH INTENSITY DISCHARGE HORSE POWER HEIGHT HERTZ INTERMEDIATE METALLIC CONDUIT SHORT CIRCUIT CURRENT (RMS SYMMETRICAL) ISOLATED J-B0X JUNCTION BOX THOUSAND CIRCULAR MILLS KCMIL KILO VOLT AMP KILOWATT LIGHTING CONTROL PANEL LOW VOLTAGE THOUSAND CIRCULAR MILLS MECHANICAL MAIN DISTRIBUTION PANEL METAL HALIDE MISCELLANEOUS MAIN LUGS ONLY MAIN POINT OF ENTRY MAIN SWITCHBOARD NOT IN CONTRACT NOT IN ELECTRICAL SECTION OF THESE PLANS & SPECS. NIGHT LIGHT NUMBER NOT TO SCALE ON CENTER OFCI OWNER FURNISHED, CONTRTRACTOR INSTALLED OFOI OWNER FURNISHED, OWNER INSTALLED PULL BOX PROVISION FOR FUTURE BREAKER W/ PFB MOUNTING HARDWARE PRIMARY DAYLIT ZONE PROVISION FOR FUTURE CURRENT TRANSFORMER PHASE PLYWOOD PLYWD PANEL PNLPAIR POLYVINYL CHLORIDE CONDUIT RELOCATE / RELOCATED (R) REQ'D REQUIRED ROOM RIGID METAL CONDUIT REMOVE AND REPLACE SECONDARY DAYLIT ZONE SKYLIGHT DAYLIT ZONE SPEC SPECIFICATION SIGNAL TERMINAL CABINET SQUARE SWITCH TELEPHONE TELECOMMUNICATIONS GROUNDING TELECOMMUNICATIONS MAIN GROUNDING BUSBAR TELEPHONE TERMINAL BOARD TYPICAL UNDERGROUND UNLESS OTHERWISE NOTED UON VOLTS WEATHERPROOF WEIGHT WATT TRANSFORMER

#### **GENERAL NOTES**

- 1. PLANS ARE NOT FOR CONSTRUCTION UNTIL APPROVED BY THE AUTHORITY HAVING JURISDICTION. THE CONTRACTOR SHALL NOT ORDER ANY MATERIALS OR INSTALL ANY EQUIPMENT, PIPING, ETC. UNTIL PLANS ARE APPROVED BY THE AUTHORITY HAVING JURISDICTION.
- 2. ALL WORK SHALL BE DONE AT SUCH TIME AND IN SUCH MANNER AS PRESCRIBED BY THE SCHOOL'S REPRESENTATIVE.
- 3. PROTECT EXISTING EQUIPMENT AND FURNISHINGS FROM ANY DAMAGE DUE TO DUST, MOISTURE OR CONTACT WITH WORK CREW OR MATERIALS.
- 4. THE SCHOOL SHALL BE NOTIFIED AT LEAST FORTY-EIGHT (48) HOURS IN ADVANCE OF ANY POWER SHUTDOWN OF EXISTING PANELS OR SERVICE. SCHEDULE OF SHUTDOWNS SHALL BE AT CONVENIENCE OF THE SCHOOL. THE SCHOOL MAY, AT THEIR OPTION, HAVE A REPRESENTATIVE PRESENT DURING SHUTDOWN. ALL WORK REQUIRING SHUTDOWNS OF EXISTING PANELS OR SERVICE SHALL BE DONE BETWEEN 12:00 AM MIDNIGHT AND 6:00AM WEEKDAYS OR ON SATURDAY AND SUNDAY. REQUIRED SHUTDOWNS SHALL BE KEPT TO A MINIMUM.
- 5. ADEQUATELY STRAP AND SUPPORT ALL CONDUIT WORK PER CEC. IN GENERAL, SUPPORT ALL CONDUIT WITHIN THREE FEET (3') OF OUTLET BOX, CABINET OR PANEL AND MAXIMUM TEN FEET (10') ON CENTER THEREAFTER.
- 6. CORE BORE SHALL BE 1" DIAMETER LARGER THAN EACH CONDUIT. SPACE CONDUIT HOLES 3" APART. SEAL AROUND CONDUIT WITH NON-SHRINK, NON-METALLIC GROUT.
- 7. ALL CONDUCTORS INSTALLED IN PANELBOARDS SHALL BE TRAINED, LACED, AND INSTALLED WITH PHASE TAPE ON ALL CONDUCTORS. 8. LABEL DEVICES (I.E. RECEPTACLES, ETC.) ON EACH COVER PLATE IDENTIFYING CIRCUIT AND PANEL DEVICE IS CONNECTED TO.
- 9. CLEAN ALL EXTERIOR AND INTERIOR SURFACES OF PANELS AND ALL MATERIAL AND METAL SHAVINGS FROM PANEL AND CABINET INTERIORS. ALL OPENINGS SHALL BE SEALED AND APPLY TOUCH-UP SPRAY PAINT WHERE NEEDED.
- 10. FIELD COORDINATE DEVICE LOCATIONS PRIOR TO ROUGH-IN.
- 11. CONTRACTOR WILL PROVIDE WARNING LABELS NOTING THE POTENTIAL FOR ELECTRIC ARC FLASH HAZARDS PER CEC 110.16. PROVIDE LABELS ON EQUIPMENT SUCH AS SWITCHBOARDS, SWITCHGEAR, PANELBOARDS, INDUSTRIAL CONTROL PANELS, METER SOCKET ENCLOSURES, MOTOR CONTROL CENTERS, MOTOR STARTER / CONTACTOR PANELS, DISCONNECTS, ETC.. PROVIDE WARNING LABELS BY BRADY, MODEL NO. 101517, OR EQUAL, ON ALL
- 12. INSTALLATION SHALL COMPLY WITH CEC 210.4 EACH MULTIWIRE BRANCH CIRCUIT SHALL BE PROVIDED WITH A MEANS THAT WILL SIMULTANEOUSLY DISCONNECT ALL UNGROUNDED CONDUCTORS AT THE POINT WHERE THE BRANCH CIRCUIT ORIGINATES. THEREFORE ANY CIRCUIT SHARING A COMMON NEUTRAL SHALL BE CAPABLE OF SIMULTANEOUS DISCONNECT OR DEDICATED NEUTRALS SHALL BE INSTALLED.
- 13. SUPPORT ENCLOSURES, BOXES AND CONDUIT INSTALLATIONS PER CEC 314.23 (A) THROUGH (H).
- 14. SEAL CONDUIT OPENINGS THROUGH WALLS AND CEILINGS. INSTALL ESCUTCHEON PLATES AT BUILDING INTERIOR. WHERE EQUIPMENT IS INSTALLED ON THE EXTERIOR WALL, STUB CONDUITS THROUGH WALL AND SEAL CONDUIT OPENINGS, THEN INSTALL EXTERIOR EQUIPMENT. ALSO, SEAL AROUND THE PERIMETER EDGE OF THE EQUIPMENT ENCLOSURE BETWEEN THE ENCLOSURE AND BUILDING.
- 15. CONDUITS INSTALLED ON ROOF AND BUILDING EXTERIOR SHALL BE RIGID GALV. STEEL (HEAVY WALL) WITH THREADED FITTINGS. CONDUIT AND WALL TO BE PAINTED OUT TO MATCH EXTERIOR FINISH.
- 16. SPLICES AND TERMINALS SHALL BE COMPRESSION TYPE OF SEAMLESS PURE COPPER, TIN PLATED, LONG BARREL (TERMINALS WITH TWO-HOLE PAD AND INSPECTION WINDOW WITH NEMA DRILLING), AS MANUFACTURED BY BURNDY TYPE YS, YAZ-2N OR EQUAL. CLEAN ALL SURFACES AND INSTALL WITH OXIDE INHIBITING COMPOUND, BURNDY PENETROX-E OR EQUAL. APPLY COMPOUND BETWEEN BUS AND LUG PAD AND BETWEEN CONDUCTOR AND LUG BARREL. INSTALL COMPRESSION CONNECTORS WITH 360° CIRCUMFERENTIAL COMPRESSION DYE, BURNDY HYPRESS OR EQUAL. THE INDENTER OR OTHER TYPE TOOLS WILL NOT BE ACCEPTABLE.
- 17. INSTALL 'MECHANICALLY FASTENED PHENOLIC NAMEPLATE WITH WHITE LETTERING ON BLACK BACKGROUND ON ALL EQUIPMENT, INCLUDING PULL BOXES, WITH DESCRIPTION INDICATED ON DRAWINGS. NAMEPLATES SHALL READ EXACTLY AS DESCRIBED ON THE DRAWINGS. IN GENERAL NAMEPLATE LETTERING SIZE SHALL BE 3/16" HIGH FOR ALL NAMEPLATES SERVING FEEDER AND BRANCH CIRCUIT BREAKERS. ON MAIN SERVICE PANEL. DISTRIBUTION PANELS AND ALL OTHER NAMEPLATES LETTERING SHALL BE 1/4" HIGH.
- 17.1. ALL SWITCHBOARDS, SWITCHGEAR, PANELBOARDS, VFD'S, MOTORS, JUNCTION BOXES, PULL BOXES, DISCONNECT SWITCHES, ETC., SHALL BE MARKED TO INDICATE EACH DEVICE OR EQUIPMENT WHERE THE POWER ORIGINATES PER CEC 408.4, FIELD IDENTIFICATION REQUIRED, (B) SOURCE OF SUPPLY.
- 18. COORDINATE EQUIPMENT LOCATIONS, CONTROL AND POWER WIRING REQUIREMENTS AND CONNECT POINTS WITH ALL APPLICABLE DISCIPLINES.
- 19. PROVIDE AND INSTALL FUSES PER UNIT NAMEPLATE DATA ON THE EQUIPMENT PROVIDED.
- 20. A LAMINATED COPY OF THE FINAL RECORD ONE LINE DIAGRAM SHALL BE PLACED IN ELEC ROOM.
- 21. PROVIDE WIRING DEVICES AND COVER PLATES IN COLOR(S) SELECTED BY ARCHITECT. THE COLOR OF THE WIRING DEVICE AND COVER PLATE SHALL BE THE SAME UNLESS SPECIFICALLY NOTED OTHERWISE.
- 22. RECEPTACLE WEATHERPROOF COVERS SHALL BE LISTED "EXTRA DUTY", LOCAKBLE, METAL, IN-USE TYPE.
- 23. REINSTALL EXISTING ELECTRICAL INSTALLATIONS DISTURBED. CERTAIN EXISTING ELECTRICAL INSTALLATIONS MAY BE LOCATED IN WALLS. CEILINGS OR FLOORS THAT ARE TO BE REMOVED AND ARE ESSENTIAL FOR THE OPERATION OF OTHER REMAINING INSTALLATIONS. WHERE THIS CONDITIONS OCCURS, PROVIDE A NEW EXTENSION OF ORIGINAL CIRCUITS, RACEWAYS, EQUIPMENT AND OUTLETS TO RETAIN SERVICE CONTINUITY. INSTALLATIONS SHALL BE CONCEALED IN FINISHED AREAS.
- 24. FOR ROOF PENETRATIONS, REFER TO ARCHITECTURAL PLANS FOR INSTALLATION REQUIREMENTS.
- 25. FOR WALL PENETRATION INSTALLATIONS, REFER TO ARCHITECTURAL PLANS FOR REQUIREMENTS.
- 26. PROVIDE "LOCK-ON" DEVICE FOR ALL CIRCUIT BREAKERS ON EMERGENCY DEDICATED CIRCUITS.
- 27. DRAWINGS ARE TO BE CONSIDERED DIAGRAMMATIC. CONTRACTOR SHALL ACCEPT RESPONSIBILITY IN FAMILIARIZING THEMSELVES WITH ARCHITECTURAL AND STRUCTURAL CONDITIONS ALONG WITH INHERENT SPACE LIMITATIONS. WITH THAT UNDERSTANDING SHALL PROVIDE ALL ITEMS OF LABOR, MATERIALS AND TOOLS REQUIRED TO PROVIDE A COMPLETE INSTALLATION.
- 28. MAINTAIN A MINIMUM OF 12" SEPARATION BETWEEN ANY CONDUIT AND (E) UTILITY CONDUIT.
- 29. FOR INTERSECTING TRENCHED CONDUIT, MAINTAIN OR EXCEED THE MINIMUM CONDUIT DEPTH REQUIREMENTS.

#### MEP COMPONENT ANCHORAGE NOTE

ALL MECHANICAL, PLUMBING AND ELECTRICAL COMPONENTS SHALL BE ANCHORED AND INSTALLED PER THE DETAILS ON THE DSA APPROVED CONSTRUCTION DOCUMENTS. THE FOLLOWING COMPONENTS SHALL BE ANCHORED AND BRACED TO MEET THE FORCE AND DISPLACEMENT REQUIREMENTS PRESCRIBED IN THE 2019 CBC SECTIONS 1617A.1.18 THROUGH 1617A.1.26 AND ASCE 7-16 CHAPTERS 13, 26 AND 30:

- ALL PERMANENT EQUIPMENT AND COMPONENTS. TEMPORARY, MOVEABLE OR MOBILE EQUIPMENT THAT IS PERMANENTLY ATTACHED (E.G. HARD WIRED) TO THE BUILDING UTILITY SERVICES SUCH AS ELECTRICITY, GAS OR WATER. "PERMANENTLY ATTACHED" SHALL INCLUDE ALL ELECTRICAL CONNECTIONS EXCEPT PLUGS FOR 110/20 VOLT RECEPTACLES HAVING A FLEXIBLE CABLE.
- 3. TEMPORARY, MOVEABLE OR MOBILE EQUIPMENT WHICH IS HEAVIER THAN 400 POUNDS OR HAS A CENTER OF MASS LOCATED 4 FEET OR MORE ABOVE THE ADJACENT FLOOR OR ROOF LEVEL THAT DIRECTLY SUPPORTS THE COMPONENT IS REQUIRED TO BE RESTRAINED IN A MANNER APPROVED BY DSA.
- THE FOLLOWING MECHANICAL AND ELECTRICAL COMPONENTS SHALL BE POSITIVELY ATTACHED TO THE STRUCTURE, BUT NEED NOT DEMONSTRATE DESIGN COMPLIANCE WITH THE REFERENCES NOTED ABOVE. THESE COMPONENTS SHALL HAVE FLEXIBLE CONNECTIONS PROVIDED BETWEEN THE COMPONENT AND ASSOCIATED DUCTWORK, PIPING, AND CONDUIT. FLEXIBLE CONNECTIONS MUST ALLOW MOVEMENT IN BOTH TRANSVERSE AND LONGITUDINAL DIRECTIONS:
- A. COMPONENTS WEIGHING LESS THAN 400 POUNDS AND HAVING A CENTER OF MASS LOCATED 4 FEET OR LESS ABOVE THE ADJACENT FLOOR OR ROOF LEVEL THAT DIRECTLY SUPPORTS THE COMPONENT.
- B. COMPONENTS WEIGHING LESS THAN 20 POUNDS, OR IN THE CASE OF DISTRIBUTED SYSTEMS, LESS THAN 5 POUNDS PER FOOT, WHICH ARE SUSPENDED FROM A ROOF OR FLOOR OR HUNG FROM A WALL.
- THE ANCHORAGE OF ALL MECHANICAL, ELECTRICAL AND PLUMBING COMPONENTS SHALL BE SUBJECT TO THE APPROVAL OF THE DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE CHARGE OR STRUCTURAL ENGINEER DELEGATED RESPONSIBILITY AND ACCEPTANCE BY DSA. THE PROJECT INSPECTOR WILL VERIFY THAT ALL COMPONENTS AND

EQUIPMENT HAVE BEEN ANCHORED IN ACCORDANCE WITH THE ABOVE REQUIREMENTS.

#### PIPING, DUCTWORK, AND ELECTRICAL DISTRIBUTION SYSTEM BRACING NOTE

PIPING, DUCTWORK, AND ELECTRICAL DISTRIBUTION SYSTEMS SHALL BE BRACED TO COMPLY WITH THE FORCES AND DISPLACEMENTS PRESCRIBED IN ASCE 7-16 SECTION 13.3 AS DEFINED IN ASCE 7-16 SECTIONS 13.6.5, 13.6.6, 13.6.7, 13.6.8 AND 2019 CBC, SECTIONS 1617A.1.24, 1617A.1.25 AND 1617A.1.26.

THE METHOD OF SHOWING BRACING AND ATTACHMENTS TO THE STRUCTURE FOR THE IDENTIFIED DISTRIBUTION SYSTEM ARE AS NOTED BELOW. WHEN BRACING AND ATTACHMENTS ARE BASED ON A PREAPPROVED INSTALLATION GUIDE (E.G., OSHPD OPM FOR 2013 CBC OR LATER), COPIES OF THE BRACING SYSTEM INSTALLATION GUIDE OR MANUAL SHALL BE AVAILABLE ON THE JOBSITE PRIOR TO THE START OF AND DURING THE HANGING AND BRACING OF THE DISTRIBUTION SYSTEMS. THE STRUCTURAL ENGINEER OF RECORD SHALL VERIFY THE ADEQUACY OF THE STRUCTURE TO SUPPORT THE HANGER AND BRACE LOADS.

MECHANICAL PIPING (MP), MECHANICAL DUCTS (MD), PLUMBING PIPING (PP), ELECTRICAL DISTRIBUTION SYSTEMS (E): MP ☐ MD ☐ PP ☐ E ■ OPTION 1: DETAILED ON THE APPROVED DRAWINGS WITH PROJECT SPECIFIC NOTES AND DETAILS.

MP ☐ MD ☐ PP ☐ E ☐ OPTION 2: SHALL COMPLY WITH THE APPLICABLE OSHPD PRE-APPROVAL (OPM #)

#### SYMBOLS LIST

- F' FUSED DISCONNECT SWITCH
- ➡ DUPLEX CONVENIENCE OUTLET
- DOUBLE DUPLEX CONVENIENCE OUTLET GROUND FAULT CIRCUIT INTERRUPTER DUPLEX OUTLET
- GROUND FAULT CIRCUIT INTERRUPTER DOUBLE DUPLEX OUTLET
- SPECIAL OUTLET TO MATCH CAP PROVIDED WITH MACHINE
- FLUSH FLOOR BOX OR "POKE-THRU" UNIT EQUIPPED WITH FLUSH OR PEDESTAL DUPLEX RECEPTACLE AND VOICE/DATA OUTLETS
- AS NOTED, OR REFER TO SCHEDULE ON DRAWINGS.
- PLUGMOLD/WIREMOLD RECEPTACLE SYSTEM
- △ TRANSFORMER
- JUNCTION BOX, SIZE AS REQUIRED BY CODE
- FLEX CONNECTION TO FIXTURE
- PANELBOARD, RECESSED MOUNTED
- PANELBOARD, SURFACE MOUNTED
- MAIN SWITCHBOARD TERMINAL CABINET, RECESSED MOUNTED
- ☐ TERMINAL CABINET, SURFACE MOUNTED → HOMERUN TO PANELBOARD OR RESPECTIVE TERMINAL
- III CONDUIT RUN CONCEALED IN CEILING OR WALL, SEE SYMBOLS LIST NOTES ---- CONDUIT RUN UNDERGROUND OR UNDER FLOOR
- —EM— EMERGENCY SYSTEM CONDUIT AND WIRES
- INSULATED GREEN GROUND CONDUCTOR
- -----O CONDUIT RISER
- LIGHT. NEW OR RELOCATED EQUIPMENT, LIGHTING, DEVICES, CONDUIT, WIRING, ETC., ARE SHOWN DARK.

— - — EXISTING EQUIPMENT, LIGHTING, DEVICES, CONDUIT, WIRING, ETC., ARE SHOWN

- X X EXISTING ELECTRICAL EQUIPMENT TO BE REMOVED WIREMOLD SURFACE RACEWAY(S) WITH OUTLETS AS SHOWN OR NOTED,
- SEE SURFACE RACEWAY SCHEDULE (1) 1> SYMBOLS REFERRING TO KEYED NOTES ON SAME SHEET
- MECHANICAL EQUIPMENT BY OTHERS, CONNECTED BY ELECTRICAL CONTRACTOR
- DETAIL DESIGNATION, "A" SIGNIFIES DETAIL, "E-1" SIGNIFIES SHEET NUMBER

(1)1-1/2"C  $\leftarrow$  INDICATES SIZE OF CONDUIT = ONE AND ONE HALF INCH CONDUIT — NUMBER WITHIN PARENTHESIS INDICATES QUANTITY OF CONDUITS

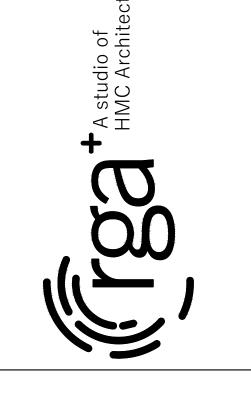
#### SYMBOLS LIST NOTES:

- 1. MOUNT SWITCH BOXES AT +48" TO TOP OF BOX UNLESS OTHERWISE NOTED.
- 2. MOUNT OUTLET BOXES AT +15" TO BOTTOM OF BOX UNLESS OTHERWISE NOTED.
- 3. "A" ADJACENT TO OUTLET INDICATES OUTLET BOX TO BE MOUNTED ABOVE COUNTER. COORDINATE WITH COUNTER HEIGHT AND DEPTH PRIOR TO ROUGH IN. MOUNT OUTLET ABOVE COUNTERS AT:
- 3.1. +46" MAX TO TOP OF BOX WHERE BOX IS INSTALLED OVER BASE CABINET. 3.2. +44" MAX TO TOP OF BOX WITH OPEN COUNTERS WITH FORWARD APPROACH. 4. OUTLET BOXES SHALL BE:
- 4.1. WALL MOUNTED -4" SQ.  $\times 2-1/8$ " DEEP MINIMUM 4.2. CEILING MOUNTED -4" SQ. OR 4" OCT.  $\times 2-1/8$ " DEEP MINIMUM
- 5. OUTLET BOXES REQUIRING 1-1/4", 1-1/2" OR 2" CONDUITS SHALL BE 4-11/16" x 3-1/4" DEEP MINIMUM.
- 6. FLUSH MOUNTED OUTLET BOXES SHALL UTILIZE TRIM RINGS. COORDINATE TRIM RING DEPTH WITH WALL FINISH PRIOR TO ROUGH-IN.
- 7. NO CROSSBARS ON CONDUIT RUN INDICATES MINIMUM 1" CONDUIT, TWO #10 CU CONDUCTORS PLUS 1#10 CU GND. CROSSBARS INDICATE NUMBER OF #10 CU CONDUCTORS IN CONDUIT. CONDUCTOR SIZES OTHER THAN #10 NOTED ON DRAWINGS. INCREASE CONDUIT SIZE AS REQUIRED TO ACCOMMODATE C.E.C. WIRE FILL REQUIREMENTS. INCLUDE ADDITIONAL BOND WIRE IN ALL PVC AND FLEXIBLE CONDUIT. LONG CROSSBAR INDICATES NEUTRAL CONDUCTOR, SHORT CROSSBARS INDICATE PHASE CONDUCTORS.
- 8. INCREASE BRANCH CIRCUIT CU CONDUCTOR SIZES AS REQUIRED BY THE 120V BRANCH CIRCUIT VOLT DROP CONDUCTOR LENGTH CHART BELOW. USE CONDUCTOR LENGTHS AS FIELD MEASURED, BASED UPON MEASURED FIELD ROUTING LENGTHS. INCREASE MINIMUM CONDUIT SIZE AS REQUIRED TO ACCOMMODATE A MAXIMUM 40% CONDUCTOR FILL OF THE BRANCH CIRCUIT CONDUCTORS. WHERE NECESSARY, PROVIDE A JUNCTION BOX AT ACCESSIBLE CEILING SPACE TO CONVERT THE LAST 15 FEET OF CONDUCTORS TO #10 AWG TO ACCOMMODATE TERMINATION OF CONDUCTORS AT WIRING DEVICES, LIGHTING FIXTURES, CIRCUIT BREAKER, ETC.
- 9. INSTALL CU GROUND CONDUCTOR IN ALL BRANCH CIRCUITS FOR LIGHT FIXTURES AND POWER DEVICES.

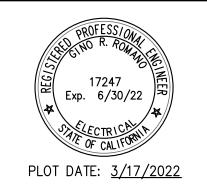
#### 120V BRANCH CIRCUIT VOLT DROP CONDUCTOR LENGTH CHART

LOAD IN		LENGT	H OF CON	DUCTOR						
VOLT	WIRE SIZE IN (GAUGE)									
AMPERES	#12	#10	#8	#6	#4					
1200VA	74	121	183	284	434					
1560VA	57	93	141	218	334					
1800VA	49	81	122	189	289					
1920VA	46	76	115	178	271					
2340VA	Х	62	94	146	223					
2880VA	Х	51	76	118	181					
3000VA	Х	48	73	114	174					
3900VA	Х	Χ	56	87	134					
4800VA	Х	Χ	46	71	108					

- 1. THIS CHART IS FOR COPPER CONDUCTORS ONLY.
- THIS CHART ASSUMES AN 80% POWER FACTOR AND STEEL RACEWAYS. 3. 2019 CALIFORNIA ENERGY CODE, 130.5(c) ALLOWS A MAXIMUM COMBINED VOLTAGE DROP OF 5%. THIS CHART ASSUMES A MAXIMUM DROP OF 3% FOR FEEDERS. THIS CHART PROVIDES THE MAXIMUM LENGTH OF CONDUCTORS FOR LESS THAN 2% VOLTAGE DROP ON A BRANCH
- CIRCUIT AT GIVEN VA LOAD. 4. USE WIRE SIZE FROM THIS CHART UNLESS LARGER CONDUCTOR SIZES ARE NOTED ON THE
- 5. FOR VA VALUES NOT SHOWN USE NEXT HIGHEST VALUE FROM THE CHART







# S **4**0 ED R H CIT 0

 $\vdash \omega$ 

S E

-RU

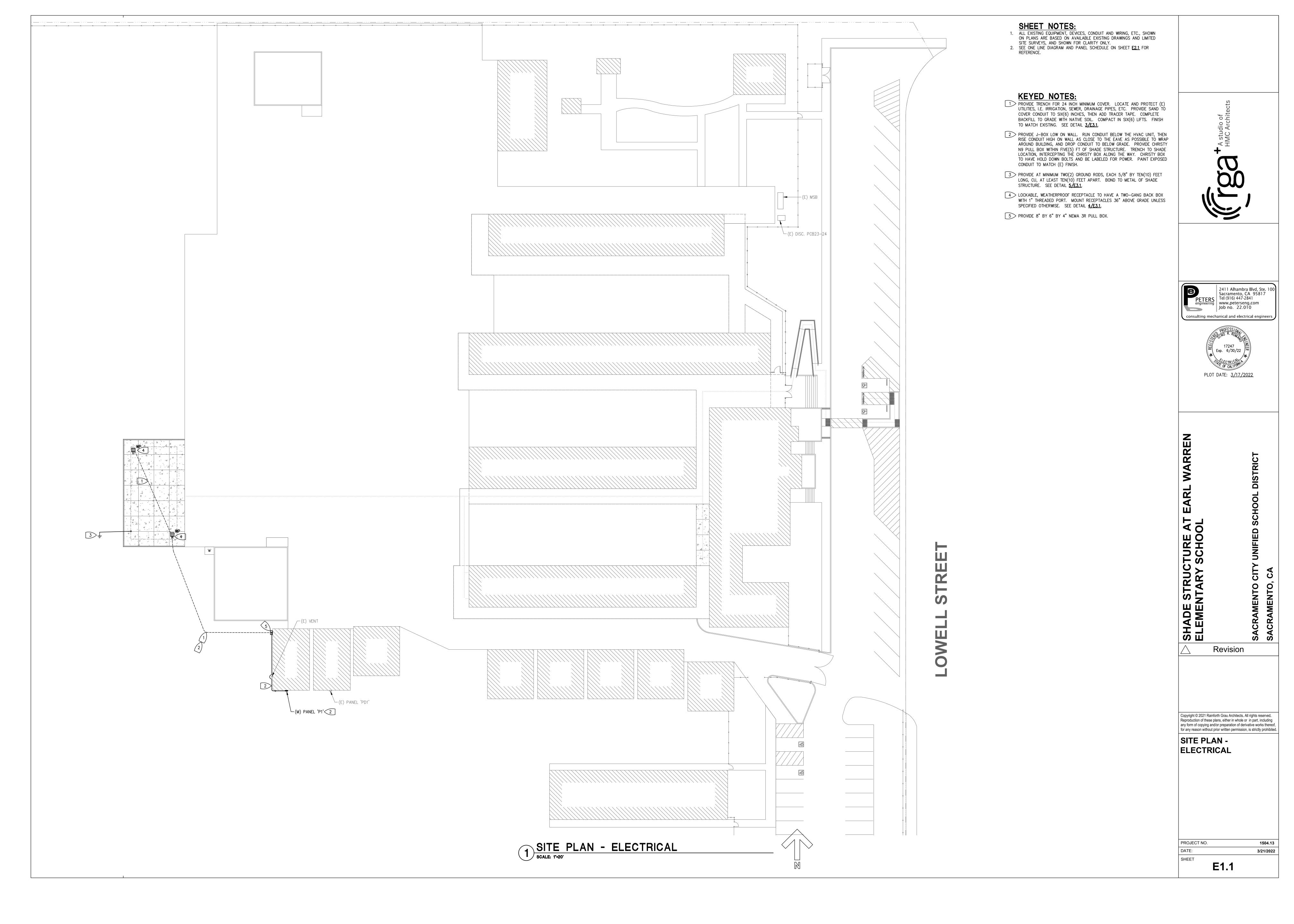
Revision

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SYMBOLS, NOTES

PROJECT NO.	1504.13
DATE:	3/21/2022
SHEET <b>E0.1</b>	



P1		SQUARE-D	MAIN:	100/2		SER	VICE:		MOUNT	IING:	ENCLOSURE:	10K	AIC
	TYPE:	HOMELINE LC	BUSS:	100	AMP	120	/208	VOLT	-	SURFACE	WIDTH:	100%	NEUT.
<u> </u>		FEEDER	RATING:	100	AMP		Ø, 3V				DEPTH:		
AØ	ВØ	DIRE	CTORY		BRKR	CKT		CKT	BRKR		DIRECTORY	AØ	BØ
4854		HVAC			70/2	5	•	6	20/1	RECEPTS		1200	
	4854	"			-	7	•	8	20/1	RECEPTS			1200
		LIGHTING			20/1	9	•	10	20/1	RECEPTS - S	SHADE STRUCT. [5]	360	
		SPACE			PFB	11	•	12	PFB	SPACE			
		SPACE			PFB	13	•	14	PFB	SPACE			
		SPACE			PFB	15	•	16	PFB	SPACE			
		NEV	V LOAD		DEMAN	ID REA	DINGS		PEAK	DEMAND @	125% + (N) LOAD	TOTAL	DEMAN
		TOTAL PANE	L VA	AMPS	AMPS	@1:	25%			IPS	VA		DAD
	AØ =	6414 \		53.5			23.9		77.3		9279 VA	19203	3 VA
	BØ =	6054 \	/A	50.5	25.8		32.3		82.7	Α	9924 VA	82.7	AMPS
NOTES:	BØ =	6054 \	/A	50.5	25.8		32.3		82.7	Α	9924 VA	82.7	AN
1.		R CONDUCTORS (				CU							
2.		H BREAKERS AR											
3. 4.		)E TYPE-WRITTEN W BREAKERS TO			-								

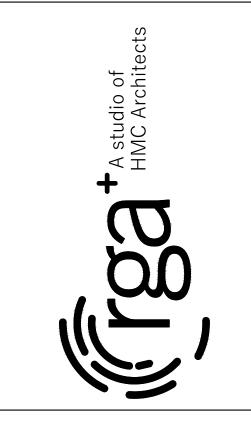
	Voltage Drop Calculations Copper												
Job Name: Earl Warren Elementary School - Shade Structure Job #: 22.020													
Date:	3/10/2022												
	VOLTAGE: 120 PHASE: 1 POWER FACTOR: 80% CONDUIT: Steel										teel		
FEEDER	AMPS AT	KVA	VOLTS	DISTANCE	DISTANCE	WIRES/	LOAD/	WIRE	WIRE	VOLTS	PERCENT		
NUMBER	LOAD	TOTAL	AT LOAD	FEET	TOTAL	PHASE	WIRE	SIZE	FACTOR	DROP	VOLT DROP		
RECEPT-1	3.0	0.4	118.48	254	254	1	3.00	10	1995	1.52	1.27%		
RECEPT-2	1.5	0.2	118.28	68	322	1	1.50	10	1995	1.72	1.44%		

SHEET NOTES:

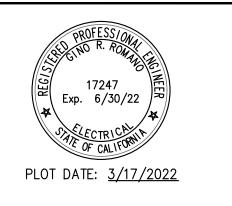
1. ALL EXISTING EQUIPMENT, DEVICES, CONDUIT AND WIRING, ETC., SHOWN ON PLANS ARE BASED ON AVAILABLE EXISTING DRAWINGS AND LIMITED SITE SURVEYS, AND SHOWN FOR CLARITY ONLY.

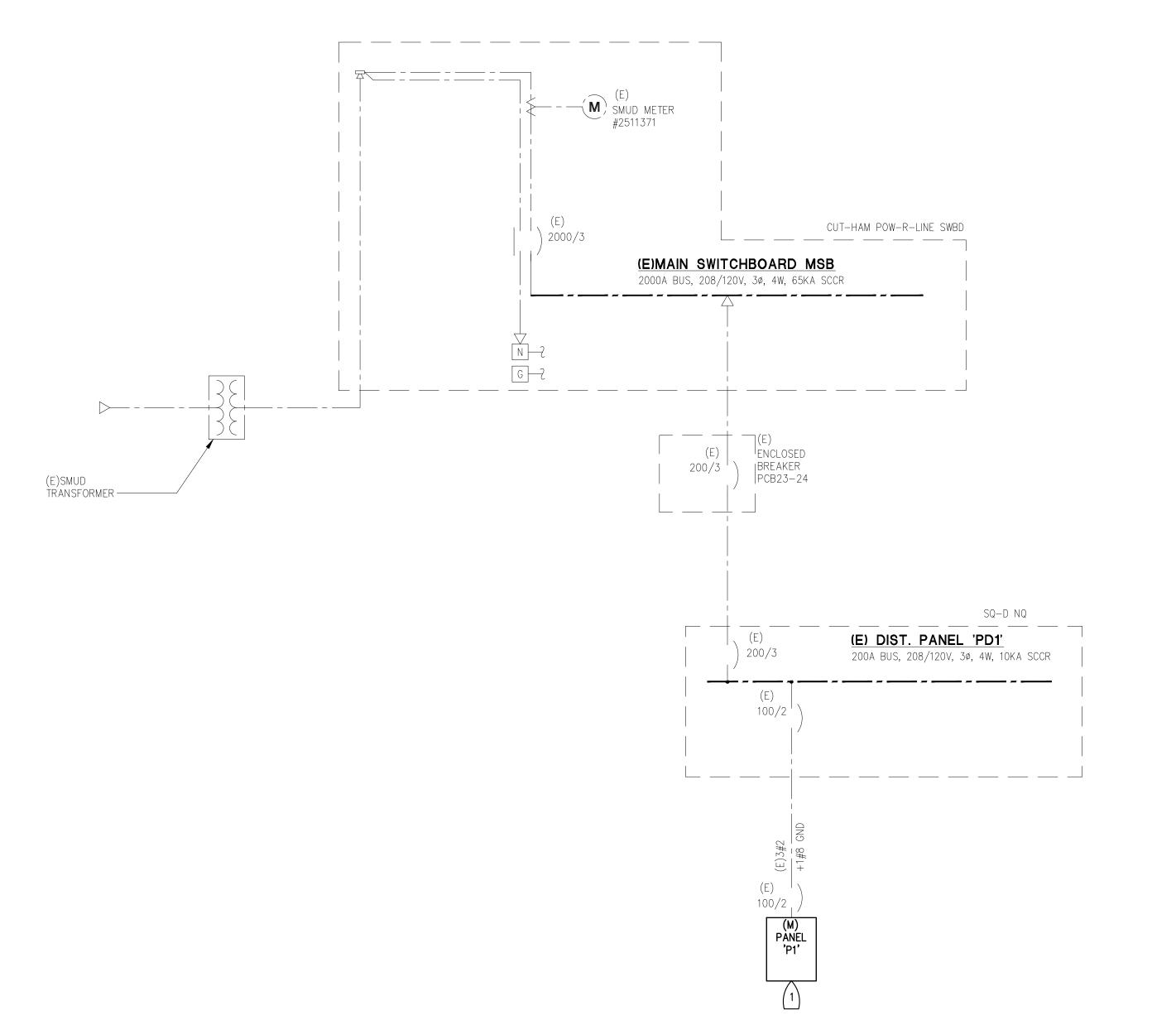
KEYED NOTES:

MODIFIED PANEL SERVES EQUIPMENT BEING ADDED IN THIS PROJECT. SEE PANEL SCHEDULE ON THIS SHEET FOR REFERENCE.









ONE LINE DIAGRAM

Revision

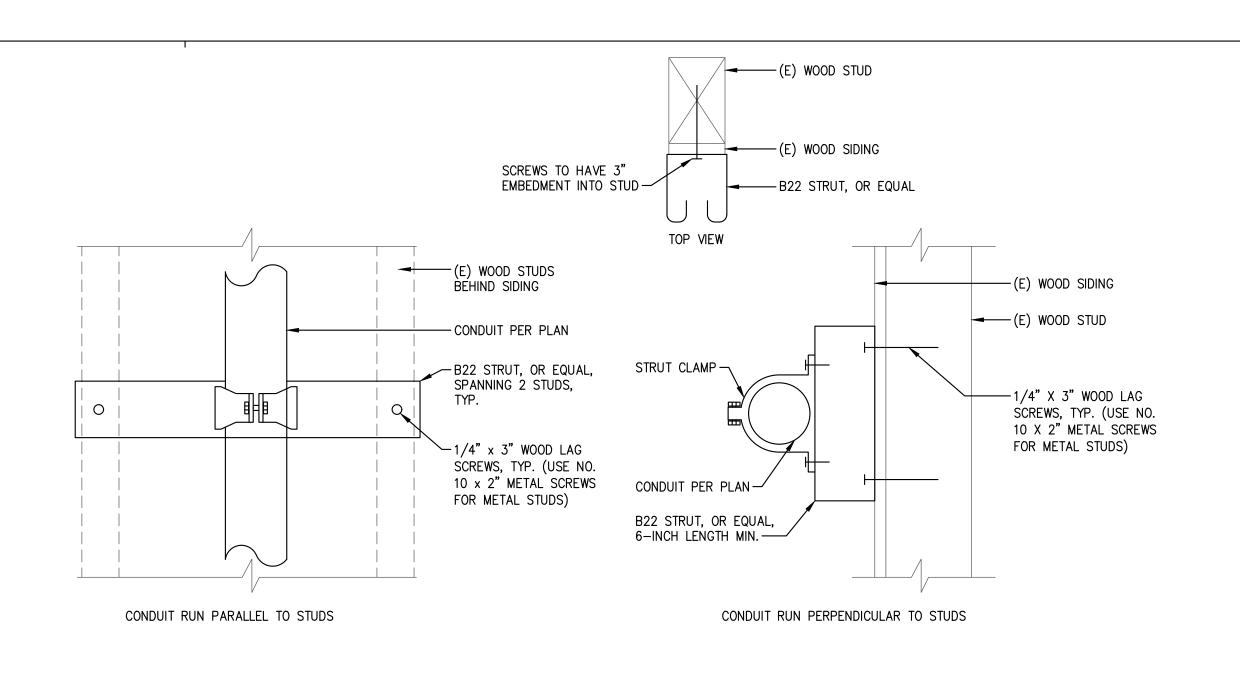
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SHADE STRUCTURE ELEMENTARY SCHO

ONE LINE DIAGRAM

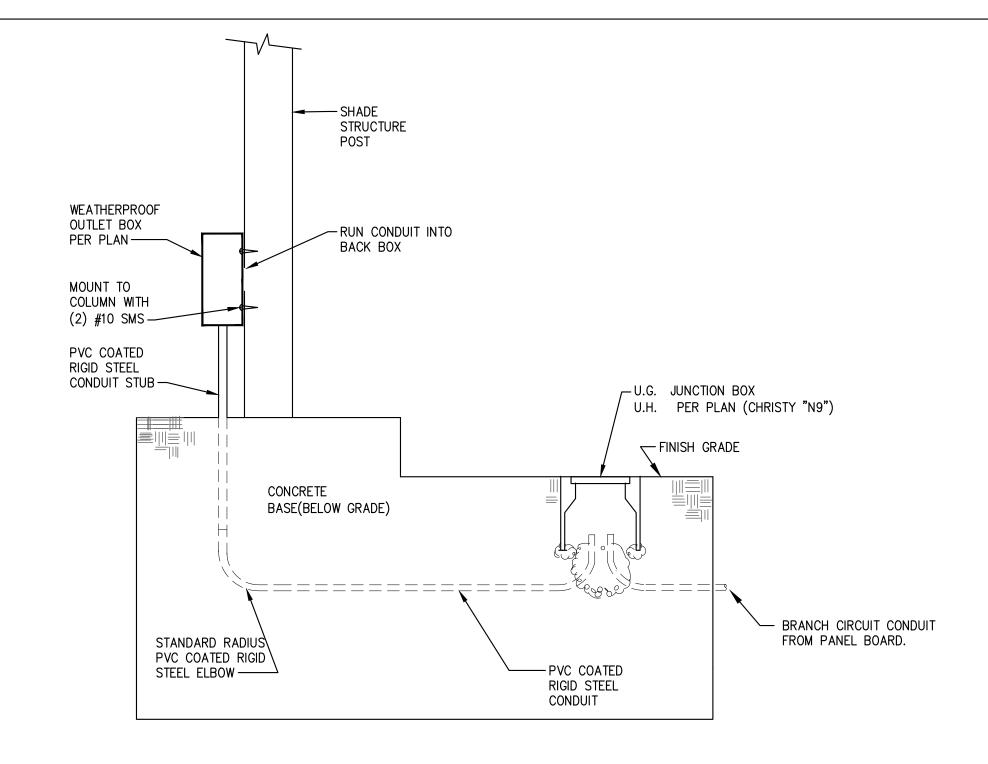
SCALE: NONE

PROJECT NO.	1504.
DATE:	3/21/20
SHEET	
<b>E2.1</b>	

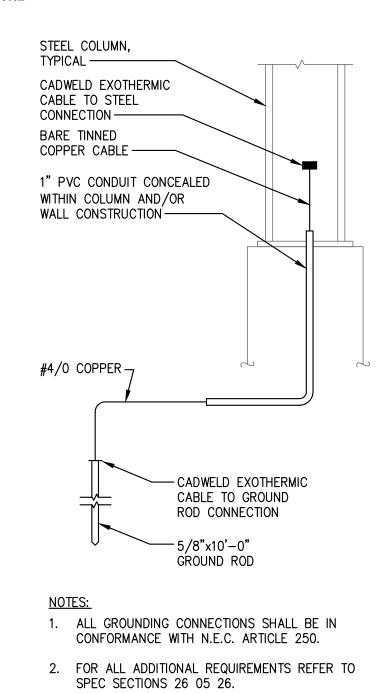


1. CONDUIT SHALL BE SUPPORTED AT INTERVALS NOT EXCEEDING TEN(10) FEET AND NOT MORE THAN THREE(3) FEET FROM THE OUTLET AND AT ANY POINT WHERE IT CHANGES DIRECTION. PERFORATED STRAP AND PLUMBER'S TAPE SHALL NOT BE PERMITTED. MAXIMUM CONDUIT AND CONDUCTOR WEIGHT IS 1.83LBS PER LINEAR FOOT.

7 CONDUIT MOUNTING DETAIL - STUD WALLS
SCALE: NONE

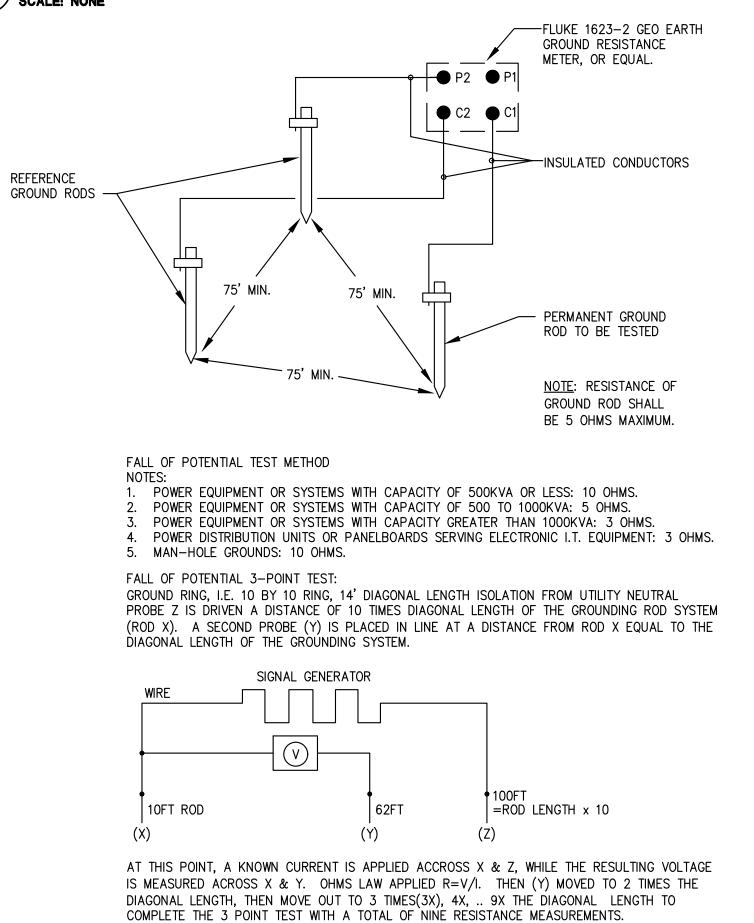


# 4 CONDUIT STUB IN POST DETAIL SCALE: NONE

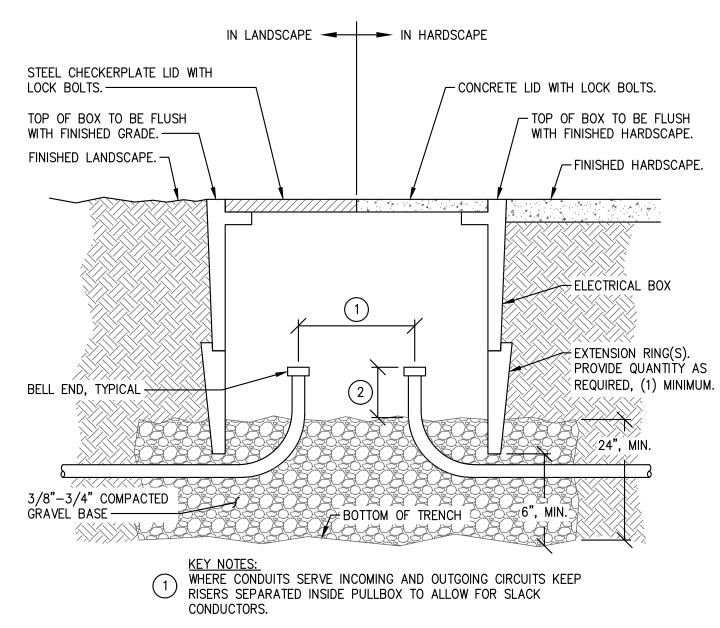


## TYPICAL STEEL COLUMN

# & REBAR GROUNDING DETAIL SCALE: NONE

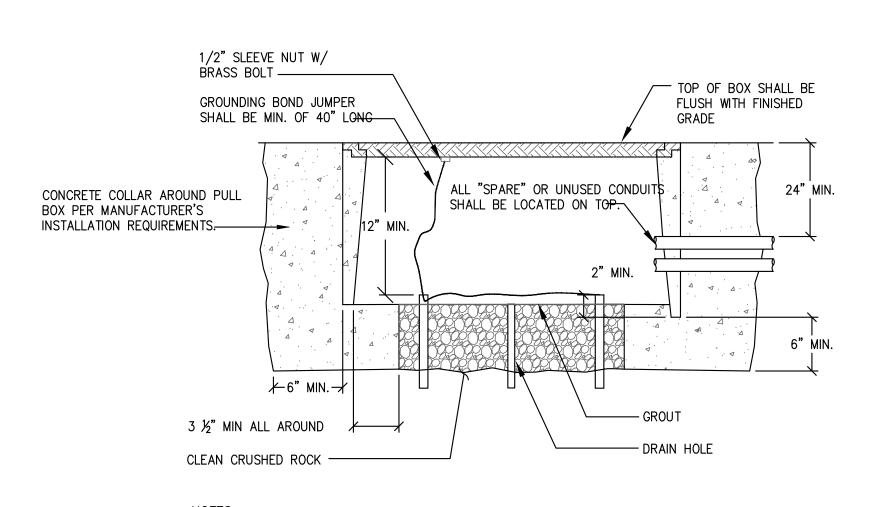


6 METHOD OF TESTING GROUND RODS DETAIL
SCALE: NONE



# TOPS OF CONDUITS MUST NOT EXTEND INTO PULLBOX MORE THAN 1/3 OF THE TOTAL AVAILABLE INSIDE BOX HEIGHT. IN ORDER TO ALLOW ADEQUATE SPACE FOR CABLE SLACK.

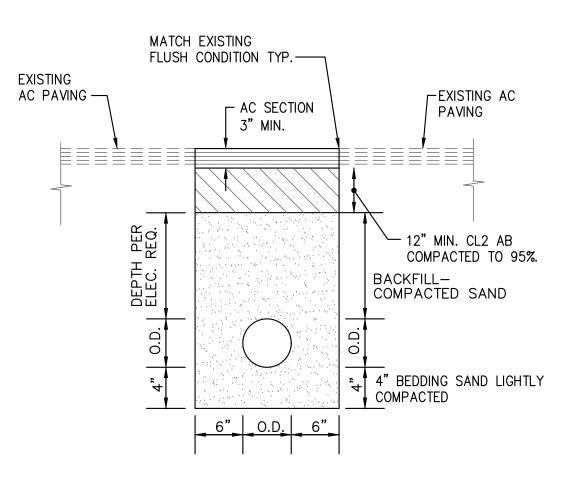
NON-TRAFFIC RATED PULL BOX



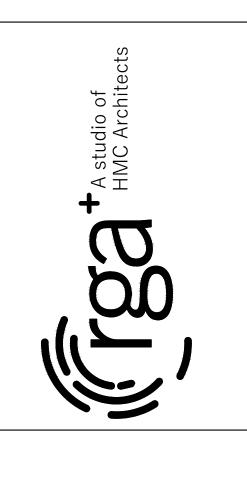
1. HANDHOLES SHALL BE PROVIDE WITH A MINIMUM OF (4) GALVANIZED PULLING PLATES IN BOTTOM OF

- 2. PULLBOXES SHALL BE PROVIDED WITH CAST IN PLACE VERTICAL CABLE RACKS. ALL CABLES SHALL BE NEATLY BUNDLED, ORGANIZED AND SUPPORTED BY CABLE RACKS.
- 3. WHERE ADDITIONAL CONDUIT ENTRIES ARE REQUIRED BEYOND QUANTITY OF TERMINATORS SHOWN. CONTRACTOR SHALL FIELD CORE DRILL AS REQUIRED. WHERE 4" TERMINATORS ARE PROVIDED CONTRACTOR SHALL PROVIDE CONDUIT REDUCERS TO MATCH SITE CONDUIT SIZE REQUIREMENTS.
- 4. FOR ALTERNATE STYLE PULLBOXES CONTRACTOR SHALL FIELD CORE DRILL ALL CONDUIT ENTRIES 2" DIA
- 5. CONTRACTOR SHALL PROVIDE THE MANUFACTURER'S INSTALLATION INSTRUCTIONS FOR TRAFFIC RATING REQUIREMENTS AS PART OF THE SUBMITTALS.

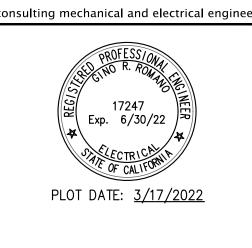
# 2 TRAFFIC RATED PULL BOX SCALE: NONE



3 TYPICAL TRENCH DETAIL
SCALE: NONE







# TURE SCHO

TRUC FARY HADE SI Revision

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**DETAILS** 

PROJECT NO. 3/21/2022 E3.1

SOCF DEAD LOAD (SUPERIMPOSED ON FRAME)   5 PSF	
ROOF LIVE LOAD         20 P           ROOF DEAD LOAD (SUPERIMPOSED ON FRAME)         5 PSF           ROOF PANEL DEAD LOAD         M=1.1 PSF, G=1.2           COLLATERAL DEAD LOAD         M=3.9 PSF, G=3.1           ROOF SNOW LOAD.         20 P           SROUND SNOW LOAD. Pg         20 P           ROOF SNOW LOAD: SLOPED, Pg         20 P           SITE APPLICATION DSA REVIEWER SHALL VERIFY THE STRUCTURE BE LOCATED AT LEAST 20 FEET FROM A SNOW LOAD LOPE FACTOR, Cg         11.6           SNOW EXPOSURE FACTOR, Cg         11.6           SNOW LOAD INPORTANCE FACTOR, Ig         1.1           HERRINA FACTOR, Cg         1.1           SASIC WIND SPEED (3 SECOND GUST), V <sub>cl</sub> 10.0           RISK CATEGORY         0           SASIC WIND SPEED (3 SECOND GUST), V <sub>cl</sub> 10.0           RISK CATEGORY         0           EXPOSURE CATEGORY         0           CACTORS, Kg, Kg, Sc         0.855, 1           Rg DOWER PASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED         CASE A (1.1 / -1.2) G           Cow, PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED         CASE A (0.6 / -0.9)           COMPONENTS & CLADDING - Cg (1 PRESSURE/SUCTION) CLEAR / OBSTRUCTED         CASE A (0.6 / -0.9)           COMPER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED         CASE A (0.6 /	VALUES
SOSP EAD LOAD (SUPERIMPOSED ON FRAME)   5 PSF	DOE
ROOF PANEL DEAD LOAD	_
COLLATERAL DEAD LOAD   ROOF SNOW LOAD	
ROOF SNOW LOAD   ROOF SNOW LOAD   ROOF SNOW LOAD   ROOF SNOW LOAD   ROOF SNOW LOAD SLOPED, P.   20 P   RISK CATEGORY   ■   ■   ROOF SNOW LOAD SLOPED, P.   20 P   STITE APPLICATION DSA REVIEWER SHALL VERIFY THE STRUCTURE BE LOCATED AT LEAST 20 FEET FROM A SNOW LOAD SLOPE FACTOR, C.   1.0   1	
I   ROBERT   ROBER	,
ROOF SNOW LOAD: SLOPED, P <sub>8</sub> STE APPLICATION DSA REVIEWER SHALL VERIFY THE STRUCTURE BE LOCATED AT LEAST 20 FEET FROM A SNOW LOAD SLOPE FACTOR, C <sub>9</sub> 1.6  SNOW LOAD SLOPE FACTOR, C <sub>9</sub> 1.7  SNOW LOAD SLOPE FACTOR, C <sub>9</sub> 1.8  SNOW LOAD IMPORTANCE FACTOR, I <sub>8</sub> 1.9  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 100 M  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 100 M  SISK CATEGORY  1.8  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 100 M  SISK CATEGORY  1.8  CATORS: K <sub>2</sub> , K <sub>2</sub> , K <sub>3</sub> 1.9  1.9  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC	PSF
SITE APPLICATION DSA REVIEWER SHALL VERIFY THE STRUCTURE BE LOCATED AT LEAST 20 FEET FROM A SNOW LOAD SLOPE FACTOR, C <sub>0</sub> 1.0.  SNOW EXPOSURE FACTOR, C <sub>0</sub> 1.0.  SNOW EXPOSURE FACTOR, C <sub>0</sub> 1.1.  SNOW EXPOSURE FACTOR, C <sub>0</sub> 1.1.  SNOW EXPOSURE FACTOR, C <sub>0</sub> 1.2.  SNOW EXPOSURE FACTOR, C <sub>0</sub> 1.3.  SNOW EXPOSURE FACTOR, C <sub>0</sub> 1.4.  SNOW EXPOSURE FACTOR, C <sub>0</sub> 1.5.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.6.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.7.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.8.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.9.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.	1
SNOW EXPOSURE FACTOR, C <sub>e</sub> 1.1. SNOW LOAD IMPORTANCE FACTOR, I <sub>e</sub> 1.2. THERMAL FACTOR, C <sub>1</sub> 3.  WIND DESIGN  BASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.  BASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.  BASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.  1.  1.  1.  1.  1.  1.  1.  1.  1	PSF
SNOW EXPOSURE FACTOR, C <sub>0</sub> 1.1.  SNOW LOAD IMPORTANCE FACTOR, I <sub>k</sub> 1.1.  THERMAL FACTOR, C <sub>1</sub> BASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 100 M  RISK CATEGORY  CACTORS: K <sub>0</sub> , K <sub>th</sub> , K <sub>d</sub> Question (3 M  Question (4 M  Question	ADJACENT STRUCTURE
SENOW LOAD IMPORTANCE FACTOR, I <sub>1</sub> THERMAL FACTOR, C <sub>1</sub> MIND DESIGN  BASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 100 M  RISK CATEGORY  RISK CATEGORY  CEXPOSURE CATEGORY  COACTION  A <sub>1</sub> = 0.00256 K, K <sub>1</sub> , K <sub>2</sub> K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>4</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>4</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>4</sub> B <sub>5</sub> = 0.00256 K, K <sub>4</sub> B <sub>4</sub> = 0.00256 K, K <sub>4</sub> B <sub>5</sub> = 0.00256 K, K <sub>4</sub> B <sub>4</sub> =	.0
THERMAL FACTOR, C, WIND DESIGN  BASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> . 100 M RISK CATEGORY   II  EXPOSURE CATEGORY   II  EXPOSURE CATEGORY   C  FACTORS, K <sub>x</sub> , K <sub>x</sub> , K <sub>x</sub>   0.85, 1  g <sub>x</sub> = 0.00256 K, K <sub>x</sub> , K <sub>x</sub>   0.85, 1  g <sub>x</sub> = 0.00256 K, K <sub>x</sub> , K <sub>x</sub>   0.85, 1  g <sub>x</sub> = 0.00256 K, K <sub>x</sub>   0.80 F ANGLE 18.43 - CLEAR / OBSTRUCTED   CASE A (1.1 /-1.2)    C <sub>ML</sub> PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED   CASE A (0.17 /-1.09)    C <sub>ML</sub> PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED   CASE A (-0.17 /-1.09)    C <sub>ML</sub> PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED   CASE A (-0.17 /-1.09)    COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED   CONE 3 - (0.29 /-  ZONE 2 - (1.77 /-  ZONE 1 - (1.15 /-  SEISMIC DESIGN   STEEL - ORDINARY CA  ANALYSIS PROCEDURE   STEEL - ORDINARY CA  SEISMIC IMPORTANCE FACTOR, Ie   EQUIVALENT LA  SEISMIC SITE CLASS   D  MCE <sub>R</sub> SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>3</sub>   0.9  SHORT PERIOD SITE COEFFICIENT, F <sub>a</sub>   1.2  LONG PERIOD COEFFICIENT, F <sub>a</sub>   1.7  FUNDAMENTAL PERIOD OF THE STRUCTURE, T   0.15  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> - USED   2.08 * 0.70  TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3)   1.0  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub>   1.0  SEISMIC DESIGN CATEGORY   1.2  RESPONSE MODIFICATION FACTOR, R   1.2  OVERSTRENGTH FACTOR, Ω   1.2  RESDUNDANCY FACTOR, Ω   1.2  RESD	.0
THERMAL FACTOR, C, WIND DESIGN  BASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> . 100 M RISK CATEGORY   II  EXPOSURE CATEGORY   II  EXPOSURE CATEGORY   C  FACTORS, K <sub>x</sub> , K <sub>x</sub> , K <sub>x</sub>   0.85, 1  g <sub>x</sub> = 0.00256 K, K <sub>x</sub> , K <sub>x</sub>   0.85, 1  g <sub>x</sub> = 0.00256 K, K <sub>x</sub> , K <sub>x</sub>   0.85, 1  g <sub>x</sub> = 0.00256 K, K <sub>x</sub>   0.80 F ANGLE 18.43 - CLEAR / OBSTRUCTED   CASE A (1.1 /-1.2)    C <sub>ML</sub> PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED   CASE A (0.17 /-1.09)    C <sub>ML</sub> PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED   CASE A (-0.17 /-1.09)    C <sub>ML</sub> PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED   CASE A (-0.17 /-1.09)    COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED   CONE 3 - (0.29 /-  ZONE 2 - (1.77 /-  ZONE 1 - (1.15 /-  SEISMIC DESIGN   STEEL - ORDINARY CA  ANALYSIS PROCEDURE   STEEL - ORDINARY CA  SEISMIC IMPORTANCE FACTOR, Ie   EQUIVALENT LA  SEISMIC SITE CLASS   D  MCE <sub>R</sub> SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>3</sub>   0.9  SHORT PERIOD SITE COEFFICIENT, F <sub>a</sub>   1.2  LONG PERIOD COEFFICIENT, F <sub>a</sub>   1.7  FUNDAMENTAL PERIOD OF THE STRUCTURE, T   0.15  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> - USED   2.08 * 0.70  TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3)   1.0  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub>   1.0  SEISMIC DESIGN CATEGORY   1.2  RESPONSE MODIFICATION FACTOR, R   1.2  OVERSTRENGTH FACTOR, Ω   1.2  RESDUNDANCY FACTOR, Ω   1.2  RESD	.0
WIND DESIGN           BASIC WIND SPEED (3 SECOND GUST), V <sub>ol</sub> R         100 M           RISK CATEGORY         0.85, 1           EXPOSURE CATEGORY         0.85, 1           10 <sub>a</sub> = 0.00256 K <sub>x</sub> K <sub>xt</sub> V <sub>x</sub> FOR ALL EAVE HEIGHTS (8', 10' & 12')         18.50           0.2 <sub>MV</sub> PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED         CASE A (1.1 / -1.2)           0.8 <sub>MV</sub> PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED         CASE A (-0.17 / -1.09)           0.2 <sub>MV</sub> PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED         CASE A (-0.6 / -0.9)           0.00MPONENTS & CLADDING - C <sub>N</sub> (PRESSURE/SUCTION) CLEAR / OBSTRUCTED         CASE A (-0.6 / -0.9)           0.00MPONENTS & CLADDING - C <sub>N</sub> (PRESSURE/SUCTION) CLEAR / OBSTRUCTED         ZONE 3 - (2.29 / -2.20	2
BASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> RISK CATEGORY  RISK CATEGORY  CATEGORY  CASEA (-0.85, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1,	
RISK CATEGORY  EXPOSURE CATEGORY  C EXPOSURE CATEGORY  C DATE OF THE EXPOSURE CATEGORY  RESPONSE MODIFICATION FACTOR, ρ  RESPONSE MODIFICATION FOR TICK. 20' WIDE, 30' WIDE, 40' WIDE)  1.10 DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  1.11 DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  1.11 DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  1.12 DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  1.14 DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  1.15 DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  1.16 DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  1.17 PENDAMENTAL OF CATEGORY  C DASS A (-0.0 f, 0.15)  C ASE A (-0.1 f, -1.2)	MPH
EXPOSURE CATEGORY FACTORS: K <sub>2</sub> , K <sub>2</sub> , K <sub>3</sub> 0.85, 1  The = 0.00256 K <sub>2</sub> K <sub>2</sub> K <sub>4</sub> V <sub>2</sub> Por All EAVE HEIGHTS (8', 10' & 12') 1.9, = 0.00256 K <sub>2</sub> K <sub>2</sub> K <sub>4</sub> V <sub>2</sub> Por All EAVE HEIGHTS (8', 10' & 12') 1.9, = 0.00256 K <sub>3</sub> K <sub>4</sub> K <sub>4</sub> V <sub>2</sub> Por All EAVE HEIGHTS (8', 10' & 12') 1.9, PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED 1.0, PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED 1.0, PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED 1.0, PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED 1.0, PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED 1.0, PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED 1.0, PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED 1.0, PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED 1.0, PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED 1.0, PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED 1.0, PER SUBJECT OBSTRUCTER, TO 1.1, PER SUBJECT OBSTRUCTED 1.0, PER SUBJECT OBSTRUCTER, TO 1.1, PER SUBJECT OBSTRUCTED 1.0, PER SUBJECT OBSTRUCTURE, TO 1.1, PER SUBJECT OBSTRUCTED 1.0, PER SUBJECT OBSTRUCTURE, TO 1.1, PER SUBJECT OBSTRUCTED	
FACTORS: K <sub>2</sub> , K <sub>2</sub> , K <sub>3</sub> Q <sub>1</sub> = 0.00256 K <sub>2</sub> K <sub>2</sub> , K <sub>3</sub> V <sup>2</sup> FOR ALL EAVE HEIGHTS (8', 10' & 12')  18.50 C <sub>MW</sub> PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED CASE A (1.1 / -1.2) C <sub>ML</sub> PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED CASE A (-0.17 / -1.09) C <sub>M</sub> PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLEAR - (OBSTRUCTED COMPONENTS & CLAD - (OBSTRUCTED COMPONENTS & CLACE COMPONENTS & COMPONENTS	
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	1, 0.85
C <sub>MM</sub> PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED         CASE A (-0.17 /-1.09)           C <sub>NL</sub> PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED         CASE A (-0.17 /-1.09)           C <sub>N</sub> PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED         CASE A (-0.6 /-0.9)           COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED         ZONE 3 - (2.29 /-2           ZONE 2 - (1.77 /-1         ZONE 1 - (1.15 /-1           SEISMIC DESIGN         ZONE 1 - (1.15 /-1           LATERAL FORCE RESISTING SYSTEM         STEEL - ORDINARY C/A           ANALYSIS PROCEDURE         EQUIVALENT LA           SEISMIC IMORTANCE FACTOR, Ie         11.0           SEISMIC SITE CLASS         D           MCE <sub>R</sub> SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>s</sub> 2.6           MCE <sub>R</sub> SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>s</sub> 2.6           MCE <sub>R</sub> SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>s</sub> 1.2           LONG PERIOD COEFFICIENT, F <sub>s</sub> 1.2           LONG PERIOD COEFFICIENT, F <sub>s</sub> 1.2           DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> - USED         2.08 * 0.70           DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> - USED         2.08 * 0.70           DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> 1.0           DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-	
$C_{NL} \ PER \ ASCE FIGURE 27.4-5 \ ROOF \ ANGLE 18.43 - CLEAR / OBSTRUCTED                                    $	
$ \begin{array}{c} C_N  \text{PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED} & CASE A  (-0.6  /  -0.9) \\ \hline COMPONENTS & CLADDING - C_N  (  \text{PRESSURE/SUCTION})  \text{CLEAR / OBSTRUCTED}} & ZONE  3 -  (2.29  /  \cdot  20  \text{Ne}  2 -  (1.77  /  -  20  /  -  20  \text{NE}  2 -  (1.28  /  -  20  ,  -  20  \text{NE}  2 $	· · · · · · · · · · · · · · · · · · ·
COMPONENTS & CLADDING - $C_N$ ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED  ZONE 3 - $(2.29)$ / $(2.29)$ / $(2.20)$	
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	· · · · · · · · · · · · · · · · · · ·
$ \begin{array}{c} \text{ZONE 1 - } 1.15 I - \\ \text{SEISMIC DESIGN} \\ \\ \text{LATERAL FORCE RESISTING SYSTEM} \\ \text{STEEL - ORDINARY CAMALYSIS PROCEDURE} \\ \text{SESIMIC IMORTANCE FACTOR, Ie} \\ \text{SEISMIC SITE CLASS} \\ \text{MCE}_R \text{ SPECTRAL RESPONSE ACCELERATION @ 0.2 \text{ s, S}_S } \\ \text{2.6} \\ \text{MCE}_R \text{ SPECTRAL RESPONSE ACCELERATION @ 0.2 \text{ s, S}_1 } \\ \text{3.1} \\ \text{3.2} \\ \text{4.1} \\ \text{5.2} \\ \text{5.2} \\ \text{5.3} \\ \text{5.4} \\ \text{5.2} \\ \text{6.4} \\ \text{6.2} \\ \text{6.2} \\ \text{6.2} \\ \text{6.2} \\ \text{6.2} \\ \text{6.2} \\ \text{6.3} \\ \text{6.2} \\ \text{6.2} \\ \text{6.3} \\ \text{6.2} \\ \text{6.2} \\ \text{6.3} \\ \text{6.2} \\ \text{6.3} \\ \text{6.2} \\ \text{6.3} \\ \text{6.3} \\ \text{6.4} \\ \text{6.2} \\ \text{6.2} \\ \text{6.2} \\ \text{6.2} \\ \text{6.3} \\ \text{6.2} \\ \text{6.3} \\ \text{6.3} \\ \text{6.4} \\ \text{6.2} \\ \text{6.2} \\ \text{6.3} \\ \text{6.2} \\ \text{6.3} \\ \text{6.3} \\ \text{6.3} \\ \text{6.3} \\ \text{6.4} \\ \text{6.4} \\ \text{6.2} \\ \text{6.2} \\ \text{6.4} \\ \text{6.2} \\ \text{6.4} \\ 6.4$	
LATERAL FORCE RESISTING SYSTEM  ANALYSIS PROCEDURE  EQUIVALENT LA SESIMIC IMORTANCE FACTOR, Ie  SEISMIC SITE CLASS  D $MCE_R$ SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>S</sub> $C.6$ $MCE_R$ SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>1</sub> $C.6$ SHORT PERIOD SITE COEFFICIENT, F <sub>a</sub> $C.6$ LONG PERIOD COEFFICIENT, F <sub>v</sub> $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $C.6$ DESIGN CATEGORY  EQUIVALENT LA  DESIGN CATEGORY  EQUIVALENT LA  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $C.6$	-1.05) / (0.5 / -1.5)
ANALYSIS PROCEDURE  SESIMIC IMORTANCE FACTOR, le  1.6  SEISMIC SITE CLASS $MCE_R$ SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>S</sub> 2.6 $MCE_R$ SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>S</sub> 3.7  SHORT PERIOD SITE COEFFICIENT, F <sub>a</sub> 1.7  FUNDAMENTAL PERIOD OF THE STRUCTURE, T  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> 2.0  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> - USED  TO DETERMINE CS (WITH CAP PER ASCE-7 12.8.1.3)  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> 1.0  SEISMIC DESIGN CATEGORY  E  RESPONSE MODIFICATION FACTOR, R  1.2  OVERSTRENGTH FACTOR, $\Omega$ 1.2  REDUNDANCY FACTOR, $\rho$ 1.4  HORIZONTAL OR VERTICAL IRREGULARITIES  SEISMIC RESPONSE COEFFICIENT, CS (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  1.2.73 PSF, 13.41	ANTILEVER COLUMN
SEISMIC SITE CLASS $MCE_R$ SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>S</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>S</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> - USED $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> - USED $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S <sub>D2</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S <sub>D2</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S <sub>D2</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S <sub>D2</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S <sub>D2</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S <sub>D2</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S <sub>D2</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S <sub>D2</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S <sub>D2</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S <sub>D2</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT S	
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	.0
MCE <sub>R</sub> SPECTRAL RESPONSE ACCELERATION @ $0.2 \text{ s}$ , $S_1$ SHORT PERIOD SITE COEFFICIENT, $F_a$ LONG PERIOD COEFFICIENT, $F_v$ 1.7  FUNDAMENTAL PERIOD OF THE STRUCTURE, T  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ 2.0  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ - USED  TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3)  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, $S_{D1}$ 1.0  SEISMIC DESIGN CATEGORY  ERSPONSE MODIFICATION FACTOR, R  OVERSTRENGTH FACTOR, $D$ 1.2  REDUNDANCY FACTOR, $D$ 1.4  HORIZONTAL OR VERTICAL IRREGULARITIES  SEISMIC RESPONSE COEFFICIENT, Cs ( $D_{CO}$ WIDE, $D_{CO}$ W	)
SHORT PERIOD SITE COEFFICIENT, $F_a$ LONG PERIOD COEFFICIENT, $F_v$ 1.7  FUNDAMENTAL PERIOD OF THE STRUCTURE, T  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ 2.0  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ - USED  TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3)  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, $S_{D1}$ 1.0  SEISMIC DESIGN CATEGORY  ERSPONSE MODIFICATION FACTOR, R  OVERSTRENGTH FACTOR, $\Omega$ 1.2  REDUNDANCY FACTOR, $\rho$ HORIZONTAL OR VERTICAL IRREGULARITIES  NON  SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  12.73 PSF, 13.41	60
LONG PERIOD COEFFICIENT, $F_V$ FUNDAMENTAL PERIOD OF THE STRUCTURE, T  0.15.  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ 2.0  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ - USED  TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3)  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, $S_{D1}$ 1.0  SEISMIC DESIGN CATEGORY  ERSPONSE MODIFICATION FACTOR, R  1.2  OVERSTRENGTH FACTOR, $\Omega$ 1.2  REDUNDANCY FACTOR, $\rho$ HORIZONTAL OR VERTICAL IRREGULARITIES  NON  SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  12.73 PSF, 13.41	90
FUNDAMENTAL PERIOD OF THE STRUCTURE, T 0.15.  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ 2.0  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ - USED 2.08 * 0.70  TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3)  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, $S_{D1}$ 1.0  SEISMIC DESIGN CATEGORY E RESPONSE MODIFICATION FACTOR, R 1.2  OVERSTRENGTH FACTOR, $\Omega$ 1.2  REDUNDANCY FACTOR, $\rho$ 1.0  HORIZONTAL OR VERTICAL IRREGULARITIES NON SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE) 1.16  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE) 12.73 PSF, 13.41	20
FUNDAMENTAL PERIOD OF THE STRUCTURE, T 0.15.  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ 2.0  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ - USED 2.08 * 0.70  TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3)  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, $S_{D1}$ 1.0  SEISMIC DESIGN CATEGORY E RESPONSE MODIFICATION FACTOR, R 1.2  OVERSTRENGTH FACTOR, $\Omega$ 1.2  REDUNDANCY FACTOR, $\rho$ 1.0  HORIZONTAL OR VERTICAL IRREGULARITIES NON SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE) 1.16  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE) 12.73 PSF, 13.41	70
DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ 2.0  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ - USED 2.08 * 0.70  TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3)  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, $S_{D1}$ 1.0  SEISMIC DESIGN CATEGORY  RESPONSE MODIFICATION FACTOR, R 1.2  OVERSTRENGTH FACTOR, $\Omega$ 1.2  REDUNDANCY FACTOR, $\rho$ 1.0  HORIZONTAL OR VERTICAL IRREGULARITIES NON  SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE) 1.16  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE) 12.73 PSF, 13.41	52 s
DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ - USED  TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3)  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, $S_{D1}$ SEISMIC DESIGN CATEGORY  RESPONSE MODIFICATION FACTOR, R  OVERSTRENGTH FACTOR, $\Omega$ REDUNDANCY FACTOR, $\rho$ HORIZONTAL OR VERTICAL IRREGULARITIES  SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  12.73 PSF, 13.41	
TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3)  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> SEISMIC DESIGN CATEGORY  RESPONSE MODIFICATION FACTOR, R  OVERSTRENGTH FACTOR, Ω  REDUNDANCY FACTOR, ρ  HORIZONTAL OR VERTICAL IRREGULARITIES  SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  12.73 PSF, 13.41	08
TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3)  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> SEISMIC DESIGN CATEGORY  RESPONSE MODIFICATION FACTOR, R  OVERSTRENGTH FACTOR, Ω  REDUNDANCY FACTOR, ρ  HORIZONTAL OR VERTICAL IRREGULARITIES  SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  12.73 PSF, 13.41	
DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> SEISMIC DESIGN CATEGORY  RESPONSE MODIFICATION FACTOR, R  OVERSTRENGTH FACTOR, Ω  REDUNDANCY FACTOR, ρ  HORIZONTAL OR VERTICAL IRREGULARITIES  SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  12.73 PSF, 13.41	<b>7</b> 0 = 1.456
SEISMIC DESIGN CATEGORY  RESPONSE MODIFICATION FACTOR, R  1.2  OVERSTRENGTH FACTOR, Ω  REDUNDANCY FACTOR, ρ  HORIZONTAL OR VERTICAL IRREGULARITIES  SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  12.73 PSF, 13.41	
SEISMIC DESIGN CATEGORY  RESPONSE MODIFICATION FACTOR, R  1.2  OVERSTRENGTH FACTOR, Ω  REDUNDANCY FACTOR, ρ  HORIZONTAL OR VERTICAL IRREGULARITIES  SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  12.73 PSF, 13.41	
RESPONSE MODIFICATION FACTOR, R  OVERSTRENGTH FACTOR, $\Omega$ REDUNDANCY FACTOR, $\rho$ HORIZONTAL OR VERTICAL IRREGULARITIES  SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  12.73 PSF, 13.41	
OVERSTRENGTH FACTOR, $\Omega$ 1.2  REDUNDANCY FACTOR, $\rho$ 1.0  HORIZONTAL OR VERTICAL IRREGULARITIES NON SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE) 1.10  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE) 12.73 PSF, 13.41	
REDUNDANCY FACTOR, ρ  HORIZONTAL OR VERTICAL IRREGULARITIES  SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  12.73 PSF, 13.41	
HORIZONTAL OR VERTICAL IRREGULARITIES  SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  12.73 PSF, 13.41	
SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  1.10  1.2.73 PSF, 13.41	
ALLOWABLE SOIL BEARING FOR FOUNDATIONS  VARIES - SEE FOUN	· ·
ALLOWABLE SOIL BEARING FOR FOUNDATIONS VARIES - SEE FOUN	
	NDATION CHARTS
FLOOD DESIGN - DESIGN IS ASSUMED TO NOT BE IN FLOOD HAZARD AREA	
IF PROJECT IS LOCATED IN A FLOOD ZONE OTHERTHAN ZONE X, A LETTER STAMPED & SIGNED FROM A SOILS ENGINEER IS REQUIRED TO VALIDATE THE	

ALL DEFLECTIONS SHOWN ALSO INCLUDE THE P-DE	LTA ROTATION PER IR PC-7	DEFLECT	IONS ARE FOR (1) STI	RUCTURE
		SOIL	CLASSES PER CBC TABLE 18	06A.2
MAXIMUM DRIFT $\delta_{max}$ SIDE COLUMNS		Soil Class 5	Soil Class 4	Soil Class 3
20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.40	2.55	2.65
30' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.25	2.35	2.45
40' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT) MINIMUM SEPARATION $(\delta_m = C_d \ \delta_{max})$ $C_d = 1.25$	(INCHES)	2.20	2.25	2.20
20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	3.00	3.19	3.31
30' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.81	2.94	3.06
40' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.75	2.81	2.75
MAXIMUM DRIFT $\delta_{max}$ CORNER COLUMNS		Soil Class 5	Soil Class 4	Soil Class 3
20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.20	2.30	2.40
30' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.30	2.45	2.50
40' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT) MINIMUM SEPARATION $(\delta_m = C_d \ \delta_{max})$ $C_d = 1.25$	(INCHES)	2.40	2.55	2.65
20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.75	2.88	3.00
30' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.88	3.06	3.13
40' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	3.00	3.19	3.31
MAXIMUM DRIFT $\delta_{max}$ END COLUMNS		Soil Class 5	Soil Class 4	Soil Class 3
20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	1.60	1.70	1.75
30' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.00	2.45	2.25
40' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT) MINIMUM SEPARATION $(\delta_m = C_d \ \delta_{max})$ $C_d = 1.25$	(INCHES)	2.50	2.30	2.80
20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.00	2.13	2.19
30' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.50	3.06	2.81
40' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	3.13	2.88	3.50

STRUCTURAL SEPARATION

ARCHITECTURAL REQUIREMENTS							
DESC RIPTION	DESIGN VAULES						
TYPE OF CONSTRUCTION	II-B						
OCCUPANCY CLASSIFICATION	A-3						
NUMBER OF STORIES	1						
FIRE SPRINKLER SYSTEM	NOT BY ICON/WEIGHT NOT INCLUDED IN DESIGN						

#### RELATED BUILDING CODES AND STANDARDS

#### TITLE 24 CODES:

2019 CALIFORNIA ADMINISTRATIVE CODE (CAC).. ..(PART 1, TITLE 24, CCR) 2019 CALIFORNIA BUILDING CODE (CBC), VOLUMES 1, AND 2.(PART 2, TITLE 24,

2019 CALIFORNIA ELECTRICAL CODE. .(PART 3, TITLE 24, CCR) 2019 CALIFORNIA MECHANICAL CODE (CMC). .(PART 4, TITLE 24, CCR) (PART 5, TITLE 24, CCR) 2019 CALIFORNIA PLUMBING CODE (CPC).. 2019 CALIFORNIA ENERGY CODE. (PART 6, TITLE 24, CCR) 2019 CALIFORNIA FIRE CODE (CFC) .(PART 9, TITLE 24, CCR` 2019 CALIFORNIA GREEN BUILDING STANDARDS CODE.....(PART 11, TITLE 24, CCR) 2019 CALIFORNIA REFERENCE STANDARDS CODE.. ..(PART 12, TITLE 24, CCR)

REFERENCE CODE SECTIONS FOR APPLICABLE STANDARDS: 2019 CBC, CHAPTER 35

#### 2019 CFC, CHAPTER 80 SCOPE OF WORK NARRATIVE

THESE DRAWINGS ILLUSTRATE THE FABRICATION AND INSTALLATION REQUIREMENTS FOR A FREE-STANDING PREFABRIC ATED STEEL SHADE STRUCTURE. THE ENTIRE STRUCTURAL SYSTEM IS COMPRISED OF HOLLOW STRUCTURAL STEEL MEMBERS SUPPORTED BY CONCRETE FOUNDATIONS. THE FLEXIBILITY INCLUDED HEREIN

ALLOWS THE STRUCTURE TO COMPLY WITH A WIDE VARIETY OF PROJECT SITES AND LOADING REQUIREMENTS.

#### <u>GENERAL:</u>

- 1. GENERAL NOTES AND TYPICAL DETAILS SHALL APPLY TO ALL PARTS OF THE JOB EXCEPT WHERE THEY MAY CONFLICT WITH DETAILS AND NOTES ON OTHER SHEETS. WHERE CONDITIONS ARE NOT SPECIFICALLY INDICATED BUT ARE OF SIMILAR CHARACTER TO DETAILS SHOWN, SIMILAR DETAILS OF CONSTRUCTION SHALL BE USED SUBJECT TO REVIEW BY THE STRUCTURAL ENGINEER FOR THIS PROJECT
- 2. WORK SHALL CONFORM TO THE REQUIREMENTS, AS AMENDED TO DATE, OF THE LATEST ADOPTED EDITION OF THE CBC, C.A.C. TITLE 24, AND ALL OTHER LOCAL, STATE AND FEDERAL REGULATIONS
- 3. OMISSIONS OR CONFLICTS BETWEEN THE VARIOUS ELEMENTS OF THE WORKING DRAWINGS AND/OR SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER FOR THIS PROJECT PRIOR TO PROCEEDING
- 4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING THE WORK OF ALL TRADES AND SHALL CHECK ALL DIMENSIONS, ALL DISCREPANCIES SHALL BE CALLED TO THE ATTENTION OF THE STRUCTURAL ENGINEER FOR THIS PROJECT AND BE RESOLVED BEFORE PROCEEDING WITH THE WORK.
- 5. THESE CONSTRUCTION DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE AND DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND SHALL BE SOLELY RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES, INCLUDING, BUT NOT LIMITED TO, BRACING, TEMPORARY SUPPORTS, AND SHORING. OBSERVATION VISIT TO THE SITE BY FIELD REPRESENTATIVES OF THE ARCHITECT/ENGINEER SHALL NOT INCLUDE INSPECTIONS OF THE PROTECTIVE MEASURES OR THE CONSTRUCTION PROCEDURES. ANY SUPPORT SERVICES PERFORMED BY THE ARCHITECT/ENGINEER DURING THE CONSTRUCTION SHALL BE DISTINGUISHED FROM CONSTRUCTION AND DETAILED INSPECTION SERVICES WHICH ARE FURNISHED BY OTHERS. THESE SUPPORT SERVICES PERFORMED BY THE ARCHITECT/ENGINEER, WHETHER OF MATERIAL OR WORK, ARE FOR THE PURPOSE OF ASSISTING IN QUALITY
- CONTROL AND IN ACHIEVING CONFORMANCE WITH CONTRACT DOCUMENTS, BUT DO NOT GUARANTEE CONSTRUCTION. 6. ASTM DESIGNATIONS AND ALL STANDARDS REFER TO THE LATEST AMENDMENTS.
- 7. CONFORM TO APPLICABLE CAL/OSHA CONSTRUCTION SAFETY REGULATIONS FOR ALL WORK PERFORMED DURING CONSTRUCTION. JOB SITE SAFETY IS STRICTLY THE RESPONSIBILITY OF THE CONTRACTOR AND NOT THE ARCHITECT/ENGINEER OR OWNER.
- 8. THE ENGINEER AND THEIR CONSULTANTS SHALL HAVE NO RESPONSIBILITY FOR THE DISCOVERY, HANDLING, REMOVAL OR DISPOSAL OF HAZARDOUS MATERIALS AT THE PROJECT SITE, INCLUDING BUT NOT LIMITED TO
- ASBESTOS, ASBESTOS PRODUCTS, POLYCHLORINATED BIPHENYL (PCB) OR OTHER TOXIC SUBSTANCES. 9. SHOULD ANY CONDITIONS DEVELOP NOT COVERED BY THE CONTRACT DOCUMENTS, OR IF A CHANGE IN THE SCOPE OF WORK IS PROPOSED, A CONSTRUCTION CHANGE DOCUMENT DETAILING AND SPECIFYING THE REQUIRED
- CHANGE(S) SHALL BE SUBMITTED TO AND APPROVED BY DSA BEFORE PROCEEDING WITH THE WORK. 10. THE SCHOOL DISTRICT INSPECTOR ON RECORD SHALL INSPECT AND APPROVE THE ERECTED FRAME PRIOR TO ROOF
- 11. SEE REQUIREMENTS FOR LOCATION IN ANY FIRE HAZARD SEVERITY ZONE FOR WILDLAND URBAN INTERFACE AREAS (WUI) AS SPECIFIED IN THE APPLICABLE VERSION OF THE CALIFORNIA BUILDING CODE. PROVIDE PROTECTION AND DETAILS OF ALL AREAS COMPLYING WITH THE WUI REQUIREMENTS.
- 12. LOCATING THIS STRUCTURE CLOSER THAN 20 FEET TO OTHER STRUCTURES MAY AFFECT THE ALLOWABLE AREA
- FOR THE EXISTING CONSTRUCTION PER THE APPLICABLE VERSION OF THE CALIFORNIA BUILDING CODE. 13. VIEWS AND DETAILS ARE NOT DRAWN TO SCALE (UNLESS NOTED OTHERWISE). DO NOT SCALE THESE DRAWINGS.

#### STRUCTURAL AND MISCELLANEOUS STEEL:

- 1. ALL STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE AMERICAN INSTITUE OF STEEL CONSTRUCTION (AISC) SPECIFICATION MANUAL REFERENCED BY THE LATEST EDITION OF THE CALIFORNIA BUILDING CODE.
- 2. PIPE SECTIONS SHALL CONFORM TO ASTM A53, Fy = 35 KSI, GRADE B OR A501 UNLESS NOTED OTHERWISE. 3. STRUCTURAL TUBING (HSS SHAPES) SHALL CONFORM TO ASTM A-500, GRADE B (OR C), Fy = 46 KSI (MIN).
- 4. IF MATERIAL AVAILABILITY IS LIMITED, MEMBER THICKNESS CAN BE INCREASED BEYOND WHAT IS SHOWN IN THESE DRAWINGS (MAXIMUM INCREASE OF 1/8").
- 5. ALL CHANNELS, ANGLES, AND MISC. STEEL SHALL CONFORM TO ASTM A-36, Fy =36 KSI.
- 6. ALL PLATE STEEL SHALL CONFORM TO ASTM A-572, Fy= 50 KSI.
- 7. ALL COLD FORM STEEL SHALL CONFORM TO ASTM A-653, CS = TYPE B, Fy = 50 KSI. 8. STRUCTURAL STEEL AND DECK SHALL BE IDENTIFIED FOR CONFORMITY PER CBC 2202A.1.
- 9. ALL ROOF DECKS SHALL HAVE KYNAR 500 METAL COATING.
- 10.ALL ROOF DECKS SHALL CONFORM TO ASTM A-792, Fy = 50 KSI.

#### INSTRUCTIONS FOR ARCHITECTS SUBMITTING THESE PRE-CHECKED DRAWING TO DSA: BEFORE SUBMITTING THESE PRE-CHECKED DRAWINGS FOR YOUR PROJECT, FOLLOW THE

STEP 1: SELECT FRAME DIMENSIONS FOR YOUR PROJECT -GABLE STRUCTURES UP TO 20' WIDE USE THE "RG 20" BASE FRAME -GABLE STRUCTURES UP TO 30' WIDE USE THE "RG 30" BASE FRAME

STEPS BELOW TO PROPERLY DEFINE THE APPROVED OPTIONS:

-GABLE STRUCTURES UP TO 40' WIDE USE THE "RG 40" BASE FRAME -MAXIMUM WIDTH IS 40' (SEE "ARCHITECTURAL VIEWS" SHEET FOR REFERENCE) -THE 24', 44', 64', 84' AND 104' LENGTHS ARE SUGGESTED BECAUSE THEY ARE THE MOST COMMON

(20' BAYS ARE THE MOST ECONOMICAL) -FRAME LENGTHS ASSUME 2' OVERHANGS (UNO BY ARCHITECT - 2' MAX DIMENSION)

STEP 2: SELECT ROOF DECK FOR YOUR PROJECT -"M" REPRESENTS McELROY METAL "MULTI-RIB" ROOF PANEL

-"G" REPRESENTS McELROY METAL "MEGA-RIB" ROOF PANEL -"S" REPRESENTS MCELROY METAL "MEDALLION-LOK" 16" STANDING SEAM ROOF PANEL

STEP 3: IDENTIFY THE Ss ACCELERATION (q) FOR YOUR PROJECT

-Ss VALUE DETERMINES THE REQUIRED SEISMIC DESIGN FORCES -Ss VALUE DEPENDS ON THE PROJECTS GEOGRAPHICAL LOCATION (VALUES RANGE FROM 0.00 TO 3.73)

STEP 4: IDENTIFY THE Ss REGION FOR YOUR PROJECT

CONSTRUCTION.

-THE REGIONS ARE DEPENDANT ON THE Ss VALUE DETERMINED IN STEP 3 -THE SS REGION DICTATES THE MAXIMUM DEAD LOAD PERMITTED ON THE FRAME (SEE TABLE TO RIGHT)

STEP 5: IDENTIFY THE ROOF DEAD LOAD FOR YOUR PROJECT -THE ROOF DECK DEAD LOAD WILL ALWAYS BE INCLUDED -THE COLLATERAL LOAD REPRESENTS ADDITIONAL LOAD THAT CAN BE SUPPORTED BY THE FRAME -BE SURE THE TOTAL ROOF DEAD LOAD FOR YOUR PROJECT IS LESS THAN OR EQUAL TO THE MAX

-MARK UP PC DRAWINGS WITH SIZE AND LOCATION OF CUTOUTS BEFORE SUBMITTING TO DSA

DEAD LOAD SHOWN IN STEP 4 FOR YOUR SE VALUE -Sds value used in calculation is the capped Sds (see design criteria) STEP 6: IDENTIFY THE FOUNDATION REQUIREMENTS FOR YOUR PROJECT

-IDENTIFY SOIL CLASS FOR PROJECT SITE PER SITE SPECIFIC SOIL CONDITIONS -USE THIS TO SELECT CORRECT FOUNDATION SIZE ON FOUNDATION SHEET STEP 7: SELECT MISCELLANEOUS OPTIONS FOR YOUR PROJECT -MAXIMUM CLEAR HEIGHT IS 12'-0"; (SEE "ARCHITECTURAL VIEWS" SHEET FOR REFERENCE)

STEP 8: SELECT APPLICABLE SHEET INDEX FOR YOUR PROJECT -REFERENCE THE BASE FRAME (STEP 1) AND THE ROOF PANEL TYPE (STEP 2) -IDENTIFY THE APPLICABLE SHEÈT INDEX

STEP 9: INCLUDE APPLICABLE SHEETS WITH YOUR DSA SUBMITTAL -INCLUDE 'MISC DESIGN OPTIONS' SHEET FOR PROJECTS WITHOUT ELECTRICAL CUTOUTS OR GUTTERS

#### NOTICE OF DISCLAIMER FOR STRUCTURAL ENGINEERING RESPONSIBILITY

- 1. PER TITLE 24, PART 1, SECTION 4-316(e) OF THE CALIFORNIA CODE OF REGULATIONS, THIS NOTICE SHALL
- BE GIVEN TO DSA PRIOR TO THE APPROVAL OF PLANS AND SPECIFICATIONS. 2. FOR THE SITE SPECIFIC PROJECT, J. R. MILLER & ASSOCIATES IS NOT THE DESIGN PROFESSIONAL IN
- GENERAL RESPONSIBLE CHARGE. 3. FOR THE SITE SPECIFIC PROJECT, J.R. MILLER & ASSOCIATES' RESPONSIBILITY IS LIMITED TO THE PREPARATION OF THE PLANS AND SPECIFICATIONS FOR THE SHELTERS OF THIS PC ONLY.
- 4. STRUCTURAL OBSERVATION OF CONSTRUCTION IS SPECIFICALLY EXCLUDED FROM J.R. MILLER & ASSOCIATES' RESPONSIBILITY FOR THE SITE SPECIFIC PROJECT. 5. ALL CONSTRUCTION ACTIVITIES RELATED TO STRUCTURAL ENGINEERING SHALL BE DELEGATED TO A QUALIFIED

ENGINEER BY THE DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE CHARGE. THESE ACTIVITIES INCLUDE.

BUT ARE NOT LIMITED TO. APPROVAL OF INSPECTOR QUALIFICATIONS. STRUCTURAL OBSERVATION OF CONSTRUCTION, REVIEW OF INSPECTION REPORTS, AND SIGNING OFF OF THE VERIFIED REPORT FOR COMPLETED WORK. 6. J.R. MILLER & ASSOCIATES WILL BE RESPONSIBLE FOR RESPONDING TO QUESTIONS PERTAINING TO THE PLANS

AND SPECIFICATIONS FOR THE SHELTERS OF THIS PC WHICH ARISE DURING PLAN REVIEW AND

- 1. ALL WELDING SHALL COMPLY WITH AWS D1.1 SPECIFICATIONS AND SHALL BE DONE BY AWS QUALIFIED WELDERS CERTIFIED FOR THE TYPE OF WELDING TO BE PERFORMED AS REQUIRED BY DSA.
- 2. ALL WELDING SHALL BE DONE BY GAS METAL ARC PROCESS WITH E70XX ELECTRODES. FLUX CORE ARC WELD
- SHALL CONFORM TO CHARPY NOTCH TOUGHNESS RATING OF 20 ft-16 @ ( 0° F). 3. ALL WELDING SHALL BE DONE IN THE SHOP WITH REQUIRED INSPECTION, PRE-APPROVED BY DSA, TO ENSURE
- PROPER MATERIAL ID AND WELDING.
- 4. WELD FILLER METAL MANUFACTURER SHALL PROVIDE WRITTEN CERTIFICATION OF COMPLIANCE WITH CODE AND SPECIFIC ATIONS.

- 1. ALL BOLTS SHOWN ON THESE DRAWINGS ARE ASTM F3125 GRADE A325 HIGH STRENGTH BOLTS (UNO), WITH THE NUTS CONFORMING TO ASTM A-563.
- 2. HIGH STRENGTH BOLTS SHALL BE VERIFIED AND INSPECTED PER CBC 1705A2.1
- 3. BEFORE ERECTING THE FRAME, VERIFY ALL BOLTS AND NUTS ARE CLEAN OF DEBRIS AND BURRS INCLUDING THE HARDWARE ALREADY FASTENED INSIDE THE MEMBERS. CHASING SOME OF THE BOLTS AND NUTS MAY BE
- 4. HARDENED STEEL WASHERS SHALL CONFORM TO ASTM F-436.
- 5. THE BOLTING INSTALLATION REQUIREMENTS OUTLINED BELOW ARE CRITICAL TO THE STRUCTURE'S DESIGN AND PERFORMANCE. THE INSTALLER IS REQUIRED TO COORDINATE THIS PHASE OF CONSTRUCTION WITH THE SPECIAL BOLTING INSPECTOR AND THE INSPECTOR OF RECORD PRIOR TO THE ERECTION OF THE FRAME. ALL BOLTS SHALL BE INSTALLED AND INSPECTED PER THE APPLICABLE VERSION OF AISC'S "SPECIFICATION FOR STRUCTURAL JOINTS
- USING HIGH-STRENGTH BOLTS", CBC 1705A.2.1; AISC 341-16 J7; AISC 360-16 N5.6. A)PRETENSIONED JOINTS MUST BE INSTALLED AND INSPECTED TO MEET ONE OF THE FOLLOWING REQUIREMENTS:
  - 1. TURN-OF-NUT PRETENSIONING
  - 2. CALIBRATED WRENCH PRETENSIONING 3. DIRECT-TENSION-INDICATOR PRETENSIONING (CONTRACTOR RESPONSIBLE FOR PURCHASE OF

- 1. ALLOWABLE SOIL PRESSURES ASSUME CLASS 5 SOIL CLASSIFICATION PER CBC TABLE 1806A, UNLESS NOTED
- 2. PER CBC SECTION 1803A.2, GEOTECHNICAL REPORTS ARE NOT REQUIRED FOR ONE-STORY LIGHT-STEEL FRAME BUILDINGS OF TYPE II CONSTRUCTION AND 4,000 SQUARE FOOT OR LESS IN FLOOR AREA AND NOT LOCATED WITHIN EARTHQUAKE FAULT ZONESOR SIESMIC HAZARD ZONES AS SHOWN ON THE MOST RECENT MAPS PUBLISHED BY THE CGS. ALLOWABLE FOUNDATION AND LATERAL SOIL PRESSURE VALUES MAY BE DETERMINED FROM TABLE 1806A.2.
- 3. FILL AND BACKFILL SHALL BE COMPACTED TO 95% OF MAX. DENSITY IN ACCORDANCE WITH ASTM TEST METHOD D-1557 OR AS RECOMMENDED BY THE GEO-TECH ENGINEER. FLOODING NOT PERMITTED.
- 4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SHORING, ETC. NECESSARY TO SUPPORT CUT AND/OR FILL BANKS DURING EXCAVATION, AND FORMING AND PLACEMENT OF CONCRETE.
- 5. MINIMUM SETBACK FROM TOE OF SLOPE ON AN ASCENDING SLOPE SHALL BE 15 FEET AND MINIMUM SETBACK
- FROM TOE OF SLOPE ON A DESCENDING SLOPE SHALL BE 40 FEET 6. PER CBC SECTION 1803A.6, GEOHAZARD REPORTS ARE NOT REQUIRED FOR ONE-STORY LIGHT-STEEL FRAME BUILDINGS OF TYPE II CONSTRUCTION AND 4,000 SQUARE FOOT OR LESS IN FLOOR AREA AND NOT LOCATED WITHIN EARTHQUAKE
- FAULT ZONESOR SIESMIC HAZARD ZONES AS SHOWN ON THE MOST RECENT MAPS PUBLISHED BY THE CGS. 7. GEOHAZRD REPORTS ARE TO COMPLY WITH DSA IR A-4 PER IR-7 SECTION 1.8
- 8. SITE SPECIFIC GEOTECHNICAL REPORT IS REQUIRED AT THE TIME OF SITE APPLICATION IS USING OTHER THAN
- 9. LATERAL BEARING HAS BEEN INCREASED PER CBC 1806A.3.4 & HAS BEEN DESIGNED FOR P-DELTA EFFECTS

#### 1. MIX DESIGN REQUIREMENTS: (NORMAL WEIGHT CONCRETE)

STRENGTH Pc	W/C RATIO	W/C RATIO	SLUMP (±1")	UNIT WEIGHT					
(28 DAYS)	(NON—AIR ENTRAINED)	(AIR ENTRAINED)		(NORMAL WEIGHT)					
4500 PSI 0.44 0.35 3" 150 PCF									

ENTRAINMENT FOR THESE CATEGORIES SHALL BE AS FOLLOWS: F0-0, F1-4.5, F2-6 3. AGGREGATES SHALL CONFORM TO THE ASTM C-33 WITH PROVEN SHRINKAGE CHARACTERISTICS OF LESS THAN 0.005.

SCHOOL DISTRICT:

- MAX AGGREGATE SIZE = 1". 4. CEMENT SHALL CONFORM TO ASTM C-150 (TYPE V) UNLESS NOTED OTHERWISE ON THE DRAWINGS.
- 5. CONCRETE SHALL BE MAINTAINED IN A MOIST CONDITION FOR A MINIMUM OF FIVE DAYS AFTER PLACEMENT.
- ALTERNATE METHODS WILL BE APPROVED IF SATISFACTORY PERFORMANCE CAN BE ASSURED. 6. CONCRETE SHALL NOT FREE FALL MORE THAN FIVE FEET.
- 7. CONCRETE DURABILITY SHALL BE PER CBC 1904A.1 & ACI 318-14 CHAPTER 19.
- 8. CONCRETE SHALL BE TESTED PER CBC 1903A, TABLE 1705A.3. AND ACI 318-14 SECTION 26.12.

#### STEP 10: IDENTIFY PROJECT NAME AND SCHOOL DISTRICT

CONSTRUCTION NOTES

TESTS AND INSPECTIONS FOR THE PROJECT.

SHALL COMPLY WITH ALL LOCAL ORDINANCES

PROJECT NAME:

		FRAME DIMENSIONS											
-			SUG	GESTED		OTHER							
STE	FRAME WIDTH	[] 20'	<b>X</b> 30'	[] 40'		[ ] (40' MAX)							
	FRAME LENGTH	Г 1 44'	<b>M</b> 64'	[]84'	[ ] 104'	[] (NO MAX)							

7		ROOF PANEL	
STEP	ROOF PANEL TYPE	<b>⋈</b> м [] G [] S	
3 EP	Pf	ROJECT SITE — Ss ACCELERATION (g)	
S S		<u>0.642</u>	

	Ss REGION								
	Ss REGIONS	MAX DEAD LOAD							
	Х	0 < Ss <= 2.14	5 PSF						
		2.14 < Ss <= 2.50	5 PSF						
RIPTION		2.50 < Ss <= 2.75	5 PSF						
		2.75 < Ss <= 3.00	4 PSF						
		Ss > 3.73 MAX	3 PSF						
	RIPTION	RIPTION	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$						

		TOTAL ROOF DEAD LOA	AD				
		DEAD LOAD	EXAMPLES				
F 5	ROOF DECK	<u>1.1</u> PSF	M=1.1PSF; G=1.2PSF ;S=1.3PSF (SEE STEP 2)				
STE	COLLATERAL	<u>0</u> PSF	LIGHTING, ETC				
	TOTAL	<u>1.1</u> PSF	ADD ROOF DECK AND COLLATERAL LOADS (MAX 5 PSF)				

1. A DSA-CERTIFIED CLASS 3 PROJECT INSPECTOR IS REQUIRED FOR THIS PROJECT.

BEFORE PROCEEDING WITH THE WORK, (SECTION 4-317(c), PART 1, TITLE 24, CCR)

2. CHANGES TO THE APPROVED DRAWINGS AND SPECIFICATIONS SHALL BE MADE BY ADDENDA OR CONSTRUCTION CHANGE

CONTINUOUS INSPECTION OF WORK, THE DUTIES OF THE INSPECTOR ARE DEFINED IN SECTION 4-342, PART 1, TITLE 24, CCR.

RECONSTRUCTION IS TO BE IN ACCORDANCE WITH TITLE 24, CCR. SHOULD ANY EXISTING CONDITIONS SUCH AS DETERIORATION

FINISHED WORK WILL NOT COMPLY WITH TITLE 24, CCR, A CONSTRUCTION CHANGE DOCUMENT (CCD), OR A SEPARATE SET OF PLANS AND SPECIFICATIONS, DETAILING AND SPECIFYING THE REQUIRED WORK SHALL BE SUBMITTED TO AND APPROVED BY DSA

OR NON-COMPLYING CONSTRUCTION BE DISCOVERED WHICH IS NOT COVERED BY THE CONTRACT DOCUMENTS WHEREIN THE

6. GRADING PLANS, DRAINAGE IMPROVEMENTS, ROAD AND ACCESS REQUIREMENTS AND ENVIRONMENTAL HEALTH CONSIDERATIONS

4. A DSA ACCEPTED TESTING LABORATORY DIRECTLY EMPLOYED BY THE DISTRICT (OWNER) SHALL CONDUCT ALL THE REQUIRED

5. THE INTENT OF THESE DRAWINGS AND SPECIFICATIONS ARE THAT ALL THE WORK OF THE ALTERATION, REHABILITATION OR

 $\,$  3. A "DSA CERTIFIED" PROJECT INSPECTOR EMPLOYED BY THE DISTRICT (OWNER) AND APPROVED BY DSA SHALL PROVIDE

DOCUMENT (CCD) APPROVED BY DSA, AS REQUIRED BY SECTION 4-338, PART 1, TITLE 24, CCR.

1. REINFORCING STEEL SHALL BE DEFORMED STEEL CONFORMING TO THE REQUIREMENTS OF ASTM A-615,

DETAILING, FABRICATION, AND ERECTION OF REINFORCING BARS SHALL CONFORM TO THE ACL

- GR 60: (#4 BARS AND LARGER)
- "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCING CONCRETE STRUCTURES." 3. MIN. COVER FOR CAST-IN-PLACE CONCRETE SHALL BE AS FOLLOWS:
  - A. CAST AGAINST EARTH ..... B. CAST AGAINST FORM BELOW GRADE .....
  - C. FORMED SLABS (#11 BAR & SMALLER)......3/4"
- D. SLABS ON GRADE (FROM TOP OF SLAB)......1" 4. BARS SHALL BE CLEAN OF RUST, GREASE OR OTHER MATERIAL LIKELY TO IMPAIR BOND.
- BENDS SHALL BE MADE COLD.
- REINFORCING SHALL BE LAP SPLICED PER ACI 318-14 SECTION 25.5. 6. PRIOR TO PLACING OF CONCRETE, REINFORCING STEEL AND EMBEDDED ITEMS SHALL BE WELL SECURED IN POSITION.
- 7. WELDING OF REINFORCING IS NOT ALLOWED. 8. REINFORCING STEEL SHALL BE INSPECTED PER CBC 1705A.3.

#### POWDER-COAT FINISH SYSTEM:

GR 40: (#3 BARS)

- ALL BUILDINGS THAT HAVE A POWDER-COATED FINISH SHALL MEET THE FOLLOWING SPECIFICATIONS:
- 1. THE STEEL FRAME SHALL BE SHOT-BLASTED TO A NEAR WHITE CONDITION PER SSPC-10 SPECIFICATIONS. 2. THE STEEL SHALL BE WASHED IN A ZINC PHOSPHATE IN AN MINIMUM EIGHT STAGE ELECTRO DEPOSITION
- 3. IMMEDIATELY FOLLOWING PRE-TREATMENT THE STEEL SHALL BE TOTALLY IMMERSED IN A LIQUID EPOXY
- PRIMER(E-COAT) AND COATED TO A UNIFORM THICKNESS OF A MINIMUM OF 0.7 TO 0.9 MILS. THE E-COATING SHALL
- PROVIDE A MINIMUM OF 1000 HOURS OF SALT SPRAY CORROSION PROTECTION TO THE STEEL. 4. THE STEEL SHALL THEN HAVE A TGIC POLYESTER COLOR COAT APPLIED OVER THE E-COATED SURFACE.
- 5. THE COLOR COAT SHALL THEN HAVE A CLEAR TGIC COATING APPLIED TO SEAL IN THE COLOR COAT AND RESIST
- ULTRAVIOLET LIGHT, TO HELP PREVENT FADING. 6. THE FINISH THICKNESS OF THESE THREE APPLICATIONS SHALL BE A MINIMUM OF 8 TO 12 MILS

MINIMUM MISC ELLANEOUS

7. ALL CARBON STEEL MEMBERS (COLUMNS, BEAMS, PLATES, ETC.) NOT POWDER-COATED SHALL BE PAINTED WITH PRIME COAT PER THE "AISC CODE OF STANDARD PRACTICE" AND THE "AISC SPECIFICATION SECTION M3" (UNLESS NOTED

OTHERWISE).	
ABBREVIATIONS:	

REINFORCING STEEL:

ATIONS:		
AMERICAN CONCRETE INSTITUTE	MPH	MILES PER HOUR
AMERICAN INSTITUTE OF STEEL CONSTRUCTION	М	MULTI-RIB ROOF PANEL (MCELROY)
ASSEMBLY (INTERNAL REFERENCE)	NTS	NOT TO SCALE
AMERICAN SOCIETY FOR TESTING AND MAT'LS	NO	NUMBER
AMERICAN WELDING SOCIETY	ОС	ON CENTER
CALIFORNIA BUILDING CODE	OSHA	OCCUPATIONAL HEALTH AND SAFETY ADMIN
COMPLETE JOINT PENETRATION	PCF	POUNDS PER CUBIC FOOT
CLEAR	PJ	PRETENSIONED JOINT
DEGREE	PLCS	PLACES
DIAMETER	PLT	PLATE
DIMENSION	PSF	POUNDS PER SQUARE FOOT
DIVISION OF THE STATE ARCHITECT	PSI	POUNDS PER SQUARE INCH
EQUAL	QTY	QUANTITY
FEET	REF	REFERENCE
GAGE	SQ	SQUARE
INCHES	SS	STANDING SEAM ROOF PANEL (MCELROY)
KIPS PER SQUARE INCH	TYP	TYPIC AL
MAXIMUM	UNO	UNLESS NOTED OTHERWISE
	AMERICAN INSTITUTE OF STEEL CONSTRUCTION  ASSEMBLY (INTERNAL REFERENCE)  AMERICAN SOCIETY FOR TESTING AND MAT'LS  AMERICAN WELDING SOCIETY  CALIFORNIA BUILDING CODE  COMPLETE JOINT PENETRATION  CLEAR  DEGREE  DIAMETER  DIMENSION  DIVISION OF THE STATE ARCHITECT  EQUAL  FEET  GAGE  INCHES  KIPS PER SQUARE INCH	AMERICAN CONCRETE INSTITUTE  AMERICAN INSTITUTE OF STEEL CONSTRUCTION  ASSEMBLY (INTERNAL REFERENCE)  AMERICAN SOCIETY FOR TESTING AND MAT'LS  AMERICAN WELDING SOCIETY  OC  CALIFORNIA BUILDING CODE  COMPLETE JOINT PENETRATION  PCF  CLEAR  PJ  DEGREE  PLCS  DIAMETER  PLT  DIMENSION  PSF  DIVISION OF THE STATE ARCHITECT  EQUAL  FEET  GAGE  GAGE  INCHES  KIPS PER SQUARE INCH  TYP

U.S. GEOLOGIC AL SURVEY

DIV. OF THE STATE ARCHITECT APP: 04-120013 PC REVIEWED FOR SS I FLS I ACS I CG DATE: 08/06/2021

DRAWN BY

DATE

REV

REV DATE

ARCHITECTS ENGINEERS

2700 SATURN STIBREA, CA 92821

. 714.524.1870 | F. 714.524.1875

**ANGEL** 

4/2/202

FOUNDATION REQUIREMENTS SOIL CLASS 5 (BEARING)-1500 PSF 📈 | SOIL CLASS 4 (BEARING)-2000 PSF [] SOIL CLASS 3 (BEARING)-3000 PSF [ SOIL CLASS 5 (LATERAL BEARING)-100 PSF SOIL CLASS 4 (LATERAL BEARING)-150 PSF SOIL CLASS 3 (LATERAL BEARING)-200 PSF

MISC ELLANEOUS DESIGN OPTIONS CLEAR HEIGHT []8' 🔀 10' []12' [] ' (12' MAX) ELECTRICAL CUTOUTS **⋈** YES [ ] NO GUTTERS 🔀 YES [ ] NO

SHEET INDEX

	1 2 2 1												
	BASE FRAME		RG 20				RG 30				RG 40		
	ROOF PANEL TYPE		М	G	S		М	G	S		М	G	S
	SELECT ONE		[]	[]	[]		[]	[]	[X]		[]	[]	[]
	GENERAL NOTES		LS1.0	LS1.0	LS1.0		LS1.0	LS1.0	LS1.0		LS1.0	LS1.0	LS1.0
	DSA 103 EXAMPLE		LS1.1	LS1.1	LS1.1		LS1.1	LS1.1	LS1.1		LS1.1	LS1.1	LS1.1
\ \tilde{\omega}	FOUNDATION PLAN		LS2.0	LS2.0	LS2.0		LS3.0	LS3.0	LS3.0		LS4.0	LS4.0	LS4.0
STEP	FRAMING PLAN		LS2.1	LS2.1	LS2.1		LS3.1	LS3.1	LS3.1		LS4.1	LS4.1	LS4.1
	FRAME CONNECTION DETAILS		LS2.1	LS2.1	LS2.1		LS3.1	LS3.1	LS3.1		LS4.2	LS4.2	LS4.2
	ROOFING LAYOUT & DETAILS		LS2.2	LS2.3	LS2.4		LS3.2	LS3.3	LS3.4		LS4.3	LS4.4	LS4.5
	MISC DESIGN OPTIONS		LS5.0	LS5.0	LS5.0		LS5.0	LS5.0	LS5.0		LS5.0	LS5.0	LS5.0

ISTINCTIVE STEEL SHELTERS COPYRIGHT 2004, ICON SHELTER

616.396.0919 800.748.0985 616.396.0944 FX

1455 LINCOLN AVE

HOLLAND MI, 49423

PRE-CHECK (PC) DOCUMENT Code: 2019 CBC

A separate project application for construction is required.

PRINTED ON:

<b>SA 103-19: LISTING OF</b>	STRUCTURAL TESTS & SPECIAL IN	SPECTIONS, 2019 CBC			S15 & SPECIAL IN	ISPECTIO	NS (SOILS), 2019 CBC	DSA 103-19: LIST Application Number:	School Name:	J & JI LOIAL INJ	School District:	DSA 1 Applicat
pplication Number: 4-000000	School Name: ICON Shelter Systems	School District: PC Submittal	Application Number: 04-000000 DSA File Number:	School Name: ICON Shelter Systems Increment Number:			School District: PC Submittal Date Created:	04-000000 DSA File Number:	ICON Shelter Systems Increment Number:		PC Submittal  Date Created:	Applica 04-0000 DSA File
A File Number:	Increment Number:	Date Created: 2021-07-14 05:50:33					2021-07-14 05:50:33				2021-07-14 05:50:33	
												5 T
	2010	ono.	Geotechnical Reports:	: Project has a geotechnic	cal report, or CDs in	ndicate soil	s special inspection is required by GE	C. Compaction tes	ting.	Test L(	* Under the supervision of the geotechnical engineer. (Refer to specific items identified in the Appendix for exemptions when	re 🗆 a
IMPORTANT: This fo	<b>2019</b> orm is only a summary list of structural test:	CBC and some of the special inspections required for the project.	1. GENERAL:	· ·	Table 1705A.6						soils testing may be conducted under the supervision of a geotechnical engineer or LOR's engineering manager. In such cases, the LOR's form 291 shall satisfy the soil test reporting requirements for the exempt ite	DSA L
Generally, the structural te	ests and special inspections noted on this f	orm are those that will be performed by the Geotechnical Engineer uplete test and inspection program must be performed as detailed	Test or Special Inspe	ection	Type Pe By	erformed C y	ode References and Notes				251 shall satisfy the soil test reporting requirements for the exempt te	□ <b>b</b>
on the DSA approved doci	cuments. The appendix at the bottom of thi	of form identifies work NOT subject to DSA requirements for special for providing inspection of all facets of construction, including but		ared properly prior to placement of	Periodic of		By geotechnical engineer or his or her qualified representative. See Appendix for exemptions.)	4. CAST-IN-PLACE	E DEEP FOUNDATIONS (PIERS):	Table 1705A.8		□ C.
not limited to, special inspe	ections not listed on this form such as struc	tural wood framing, high-load wood diaphragms, cold-formed steel etc., per Title 24, Part 2, Chapter 17A (2019 CBC).	Foundation excavation	excavations for foundations. tions are extended to proper deptl	th			Test or Special In	spection	Type Perfo	rmed   Code References and Notes	d
_		is document are from the CBC, or California Building Code.	and have reached pro  • Materials below foot design bearing capaci	tings are adequate to achieve the				✓ a. Inspect drilling of and accurate record	operations and maintain complete	Continuous G	E*  * By geotechnical engineer or his or her qualified representative.  (See Appendix for exemptions.)	e
TO COLUMNS	ica section and table references round in th	is document are from the CDC, of Camornia Building Code.		·				□ <b>b.</b> Verify pier locati	ions, diameters, plumbness, bell cable), lengths and embedment into	Continuous G	E* * By geotechnical engineer or his or her qualified representative.  (See Appendix for exemptions.)	6
1. TYPE		2. PERFORMED BY	2, SOIL COMPACTION Test or Special Inspe		Table 1705A.6  Type Pe	erformed C	ode References and Notes		ible); record concrete or grout		(See Appendix for exemptions.)	
ntinuous – Indicates that a cont	ntinuous special inspection is	GE – Indicates that the special inspection shall be performed by a registered geotechnical engineer or his or her authorized representative.	□ a. Perform classification	tion and testing of fill materials.	By	LOR* *	Under the supervision of the geotechnical engineer.	C. Confirm adequa	te end strata bearing capacity.	Continuous G	E* * By geotechnical engineer or his or her qualified representative. (See Appendix for exemptions.)	
uired		COR – Indicates that the test or special inspection shall be performed by a testing	b. Verify use of proper	er materials, densities and	Continuous	GE* *	By geotechnical engineer or his or her qualified representative. (Refer to be cific items identified in the Appendix for exemptions where soils SI and	d. Concrete piers.		Provide tests and insp	pections per CONCRETE section below.	b
iodic – Indicates that a periodic	ic special inspection is required	laboratory accepted in the DSA Laboratory Evaluation and Acceptance (LEA) Program. See CAC Section 4-335.	during placement of f	es, placement and compaction fill.		te	sting may be conducted under the supervision of a geotechnical ngineer or LOR's engineering manager. In such cases, the LOR's form DSA					□ c.
		PI – Indicates that the special inspection may be performed by a project inspector when specifically approved by DSA.				29 ite	21 shall satisfy the soil SI and test reporting requirements for the exempt ems.)					
t – Indicates that a test is require	ired	SI – Indicates that the special inspection shall be performed by an appropriately						DGS DSA 103-19 (Revised 07.	/16/2020)			DGS DSA
		qualified/approved special inspector.						DIVISION OF THE STATE ARC	HITECT	DEPARTMENT OF GEN Page 3 o		DRNIA DIVISION
OSA 103-19 (Revised 07/16/2020)			DGS DSA 103-19 (Revised 07/16/	,	DEPARTMENT OF C	GENERAL SERVI	CES STATE OF CALIFORNIA					
ON OF THE STATE ARCHITECT	DEPARTMENT OF GI Page 1					2 of 11						
					ESTS & SPECIAL I	INSPECTI	ONS (Concrete), 2019 CBC		TING OF STRUCTURAL TEST -14 Sections 26.12 & 26.13	TS & SPECIAL INS	PECTIONS (Concrete), 2019 CBC	<b>DSA 10</b> Table 170
cation Number:	STRUCTURAL TESTS & SPECIAL IN School Name: ICON Shelter Systems	SPECTIONS (SOILS), 2019 CBC  School District: PC Submittal	Table 1705A.3; ACI 318-7 Application Number: 04-000000	-14 Sections 26.12 & 26.13 School Name: ICON Shelter System:	ns.		School District: PC Submittal	Application Number: 04-000000 DSA File Number:	School Name: ICON Shelter Systems Increment Number:		School District: PC Submittal Date Created:	Applicati 04-00000 DSA File I
ile Number:	Increment Number:	Date Created: 2021-07-14 05:50:33	DSA File Number:	Increment Number:			Date Created: 2021-07-14 05:50:33	DSA FIJE NUMBER:	mcrement wumber:		2021-07-14 05:50:33	
7. CAST-IN-PLACE CONCRE			17. STRUCTURAL  Material Verification and	L STEEL, COLD-FORMED STEEL AN	ND ALUMINUM USED FO	OR STRUCTU	RAL PURPO	☑ b. Test high-stren	ngth bolts, nuts and washers.	Test I	OR Table 1705A.2.1 Item 1c, 2213A.1; RCSC 2014 Section 7.2; DSA IR 17-	
Test or Special Inspection	Ву	rformed Code References and Notes	Test or Special Ins		Туре	Performed	Code References and Notes	Inspection of High-Stre				Te:
rial Verification and Testing: a. Verify use of required desig		SI Table 1705 A.3 Item 5, 1910 A.1.		ation of all materials and:	Periodic		Table 1705A.2.1 Item 3a–3c. 2202A.1; AISI S100-16 Section A3.1 & A3.2, AISI S240-15 Section A3 & A5, AISI S220-15 Sections A4 & A6. * By special		'snug tight") connections.	Periodic	SI Table 1705A.2.1 Item 2a, 1705A.2.6, 2204A.2; AISC 360-16 J3.1, J3.2 M2.5 & N5.6; RCSC 2014 Section 9.1; DSA IR 17-9.	fille
b. Identifiy, sample, and test r	reinforcing steel. Test	LOR 1910A.2; ACI 318-14 Section 26.6.1.2; DSA IR 17-10. (See Appendix for exemptions.)	with requirements  • Material sizes, typ		, mpi		inspector or qualified technician when performed off-site.	d. Pretensioned a	and slip-critical connections.	*	SI Table 1705A.2.1 Items 2b & 2c, 1705A.2.6, 2204A.2; AISC 360-16 J3 J3.2, M2.5 & N5.6; RCSC 2014 Sections 9.2 & 9.3; DSA IR 17-9. * "Continuous" or "Periodic" depends on the tightening method used.	.1, de
<b>c.</b> During concrete placement for strength tests, perform slu	nt, fabricate specimens Test  Jump and air content	LOR Table 1705A.3 Item 6; ACI 318-14 Sections 26.5 & 26.12.	requirements.  D. Test unidentified	ed materials	Test	LOR	2202A.1.					
tests, and determine the temp concrete.			c. Examine seam w	velds of HSS shapes	Periodic	SI	DSA IR 17-3.	19. WELDI		D1.2 for Aluminum;	705A.2.1 Items 4 & 5; AWS D1.1 and AWS D1.8 for structural steel; AWS AWS D1.3 for cold-formed steel; AWS D1.4 for reinforcing steel; DSA IR 17-	□ d. v
<b>d</b> . Test concrete (f'c).	Test	LOR 1905A.1.15; ACI 318-14 Section 26.12.	<u> </u>	ıment steel fabrication per DSA-app uments.	proved Periodic		Not applicable to cold-formed steel light-frame construction, except for russes (1705A.2.4).	   Verification of Materia	ls, Equipment, Welders, etc.:	3 (See Appendix for	exemptions.)	e.i
e. Batch plant inspection:	See Notes	SI Default of 'Continuous' per 1705A.3.3. If approved by DSA, batch plant inspection may be reduced to 'Periodic' subject to requirements in		GTH BOLTS: RCSC 2		l'	NOSES (TOOKILITY).	Test or Special II	nspection	Type Perf By	ormed Code References and Notes	23.
		Section 1705A.3.3.1, or eliminated per 1705A.3.3.2. (See Appendix for exemptions.)		nd Testing of High-Strength Bolts	s, Nuts and Washers:				er material identification markings per listed on the DSA-approved document:		SI DSA IR 17-3.	Te
f. Welding of reinforcing steel	el. Provide special i	rspection per STEEL, Category 19.1(d) & (e) and/or 19.2(g) & (h) below.	Test or Special Ins	·	, i	Ву	Code References and Notes		er material manufacturer's certificate of	Periodic	SI DSA IR 17-3.	✓ a./
			certificates of com	ation markings and manufacturer's npliance conform to ASTM standard ISA-approved documents.			Table 1705A.2.1 Items 1a & 1b, 2202A.1; AISC 360-16 Section A3.3, J3.1, and N3.2; RCSC 2014 Section 1.5 & 2.1; DSA IR 17-8 & DSA IR 17-9.	c. Verify WPS, we	lder qualifications and equipment.	Periodic	SI DSA IR 17-3.	□ b.
												DGS DSA 10
SA 103-19 (Revised 07/16/2020)			DGS DSA 103-19 (Revised 07,	•	DEPARTMENT O	DE GENERAL SEE	VICES STATE OF CALIFORNIA	DGS DSA 103-19 (Revised 0 DIVISION OF THE STATE ARG	·	DEPARTMENT OF GE	NERAL SERVICES STATE OF CALIF	DIVISION O
N OF THE STATE ARCHITECT	DEPARTMENT OF C Page		NIA DIVISION OF THE STATE ANCI	nitect		ge 6 of 11	VICES STATE OF CALIFORNIA			Page 7 o	of 11	
		SPECTIONS (Steel and Aluminum), 2019 CBC	DSA 103-19: LIST Application Number:	TING OF STRUCTURAL T School Name:	TESTS & SPECIAL	INSPECT	ONS(SIGNATURE), 2019 CBC School District:	DSA 103-19: LIST Application Number: 04-000000	OF REQUIRED VERIFIED RE School Name: ICON Shelter Systems	PORTS, CBC 2019	School District: PC Submittal	
tion Number:	103-16, AISC 341-16, AISC 358-16, AISC 360-16; School Name: ICON Shelter Systems	AISI S100-16 School District: PC Submittal	04-000000 DSA File Number:	ICON Shelter System Increment Number			PC Submittal  Date Created: 2021-07-14 05:50:33	DSA File Number:	Increment Number:		Date Created: 2021-07-14 05:50:33	
	Increment Number:	Date Created: 2021-07-14 05:50:33										
								1				
			Name of Architect or Enginee	eer in general responsible charge:				1. Soils Testing and	d Inspection: Geotechnical Verified R	eport Form DSA 293		_
3. ANCHOR BOLTS AND ANG	ICHOR RODS:		Name of Structural Engineer	r (When structural design has been deleg	gated):				ng and Inspection: Laboratory Verified	·		_
est or Special Inspection		rformed Code References and Notes						<sup>3.</sup> DSA 292			independently contracting SI, Special Inspection Verified Report Form	
. Anchor Bolts and Anchor Ro	ods Test	LOR Sample and test anchor bolts and anchor rods not readily identifiable p procedures noted in DSA IR 17-11.	er Signature of Architect or Stru	uctural Engineer:	Date:			High-Strength B 4. Report Form DS		ry Verified Report Form	DSA 291, or, for independently contracting SI, Special Inspection Verified	i 
o. Threaded rod not used for fo	foundation anchorage. Test	LOR Sample and test threaded rods not readily identifiable per procedures noted in DSA IR 17-11.	Note: To facilitate DS	SA electronic mark-ups and idoas	ication stamp application	n. DSA recom	mends against using secured electronic or digital signatures.					
			NOTE. 10 IACIIITATE DS	and Identifi	жилон эсангр application	יי וecom	DSA STAMP	]				
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A 103-19 (Revised 07/16/2020) N OF THE STATE ARCHITECT	DEPARTMENT OF C	ENERAL SERVICES STATE OF CALIFO	DIVISION OF THE STATE ARC		DEPARTMENT C	OF GENERAL SE age 10 of 11	RVICES STATE OF CALIFORNIA		HITECT	DEPARTMENT OF GENE Page 11 o		NIA

Application Number: 04-000000 ICON Shelter Systems Increment Number: DSA File Number: Date Created: 5. RETAINING WALLS: Type Performed Code References and Notes Test or Special Inspection Continuous GE\* 1705A.6.1. \* By geotechnical engineer or his or her qualified representative. (See Section 2 above). a. Placement, compaction and inspection of backfill. □ b. Placement of soil reinforcement and/or drainage Continuous | GE\* | \* By geotechnical engineer or his or her qualified representative Continuous

# By geotechnical engineer or his or her qualified representative See DSA IR 16-3. c. Segmental retaining walls; inspect placement of units, dowels, connectors, etc. d. Concrete retaining walls. Provide tests and inspections per CONCRETE section below. e. Masonry retaining walls. Provide tests and inspections per MASONRY section below. 6. OTHER SOIL Test or Special Inspection Type Performed Code References and Notes a. Soil Improvements Test GE\* Submit a comprehensive report documenting final soil improvements constructed, construction observation and the results of the confirmation testing and analysis to CGS for final acceptance. \* By geotechnical engineer or his or her qualified representative b. Inspection of Soil Improvements Continuous GE\* \* By geotechnical engineer or his or her qualified representative DGS DSA 103-19 (Revised 07/16/2020) INIA DIVISION OF THE STATE ARCHITECT DEPARTMENT OF GENERAL SERVICES STATE OF CALIFORNIA Page 4 of 11 DSA 103-19: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (Concrete), 2019 CBC Table 1705A.3; ACI 318-14 Sections 26.12 & 26.13
Application Number: School Name: PC Submittal ICON Shelter Systems Date Created: 2021-07-14 05:50:33 DSA File Number: Increment Number: 19.1 SHOP WELDING: Type Performed Code References and Notes Test or Special Inspection a. Inspect groove welds, multi-pass fillet welds, single pass | Continuous fillet welds > 5/16", plug and slot welds. applicable); DSA IR 17-3. ✓ b. Inspect single-pass fillet welds ≤ 5/16", floor and roof
 Periodic
 SI
 1705A.2.2, Table 1705A.2.1 Items 5a.5 & 5a.6; AISC 360-16 (and AISC) 341-16 as applicable); DSA IR 17-3. deck welds. c. Inspect welding of stairs and railing systems. Periodic SI 1705A.2.1; AISC 360-16 (and AISC 341-16 as applicable); AWS D1.1 & D1.3; d. Verification of reinforcing steel weldability Periodic SI 1705A.3.1; AWS D1.4; DSA IR 17-3. Verify carbon equivalent reported on other than ASTM A706. Continuous SI Table 1705A.2.1 Item 5b, 1705A.3.1, Table 1705A.3 Item 2, 1903A.8; AWS D1.4; DSA IR 17-3. e. Inspect welding of reinforcing steel. 23. ANCHOR BOLTS AND ANCHOR RODS: Type Performed Code References and Notes Test or Special Inspection ☑ a. Anchor Bolts and Anchor Rods LOR Sample and test anchor bolts and anchor rods not readily identifiable per procedures noted in DSA IR 17-11. LOR Sample and test threaded rods not readily identifiable per procedures noted in DSA IR 17-11. **b.** Threaded rod not used for foundation anchorage.

DEPARTMENT OF GENERAL SERVICES

STATE OF CALIFORNIA

DSA 103-19: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (SOILS), 2019 CBC

DIV. OF THE STATE ARCHITECT APP: 04-120013 PC REVIEWED FOR SS 🗹 FLS 🗹 ACS 🗹 CG 🗌 DATE: 08/06/2021

RH/DSA-PC

ANGEL

4/2/2021

ARCHITECTS ENGINEERS

2700 SATURN ST I BREA, CA 92821

T. 714.524.1870 | F. 714.524.1875 WWW.JRMA.COM

ICON STD

DRAWN BY

DATE

REV

REV DATE

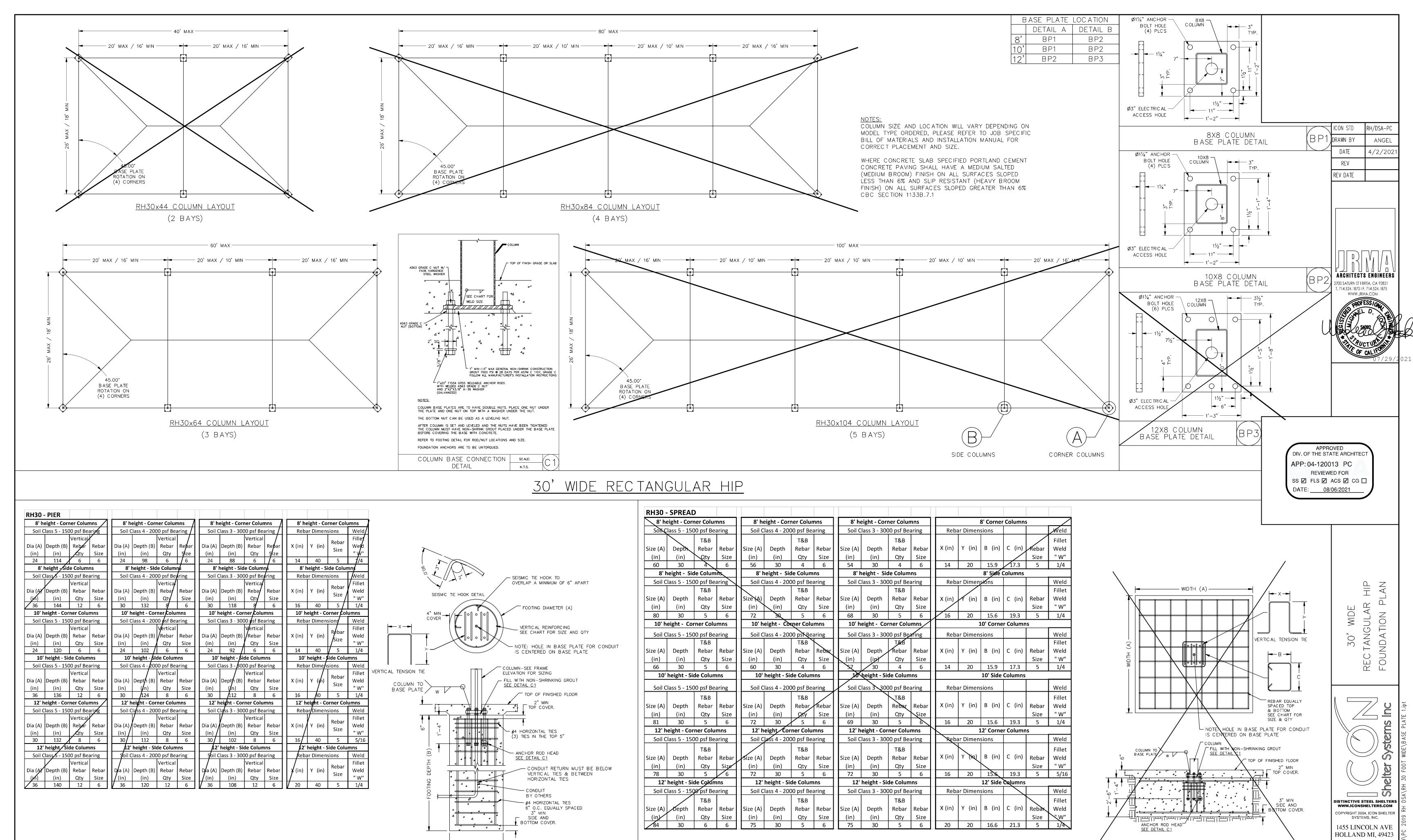
103 DSA

DISTINCTIVE STEEL SHELTERS COPYRIGHT 2004, ICON SHELTER 1455 LINCOLN AVE

HOLLAND MI, 49423

616.396.0919 800.748.0985 616.396.0944 FX

PRE-CHECK (PC) DOCUMENT Code: 2019 CBC A separate project application for construction is required.



FOOTING DIAMETER (A)

SEE DETAILS BP1. BP2 OR BP3 FOR ANCHOR BOLT PATTERNS

BP1 & BP2 ARE (4) BOLT PATTERN WHILE B3 IS A (6) BOLT

PRE-CHECK (PC) DOCUMENT
Code: 2019 CBC
A separate project application for construction is required.

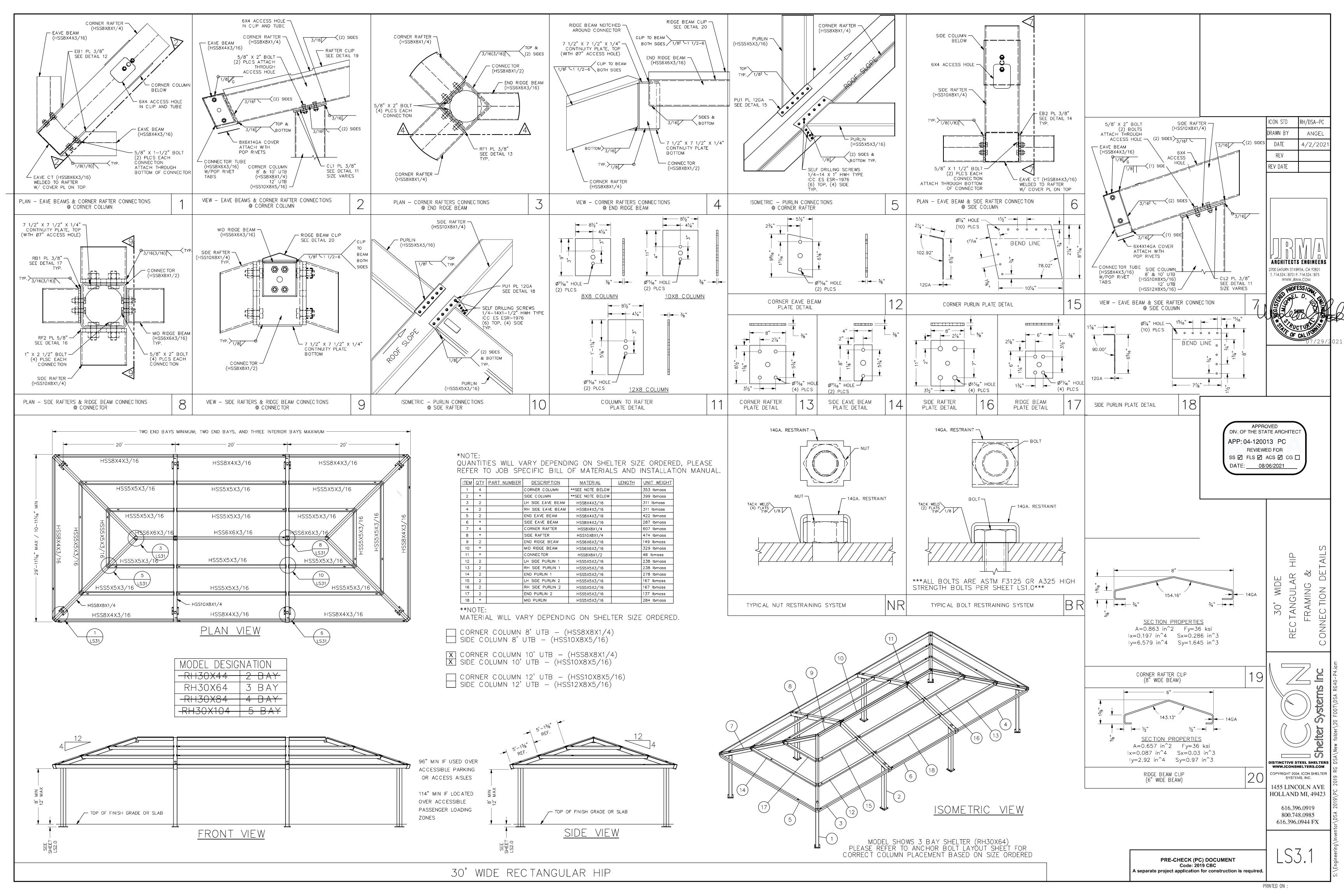
DETAILS BP1. BP2 OR BP3 FOR ANCHOR BOLT PAT**N**ERNS

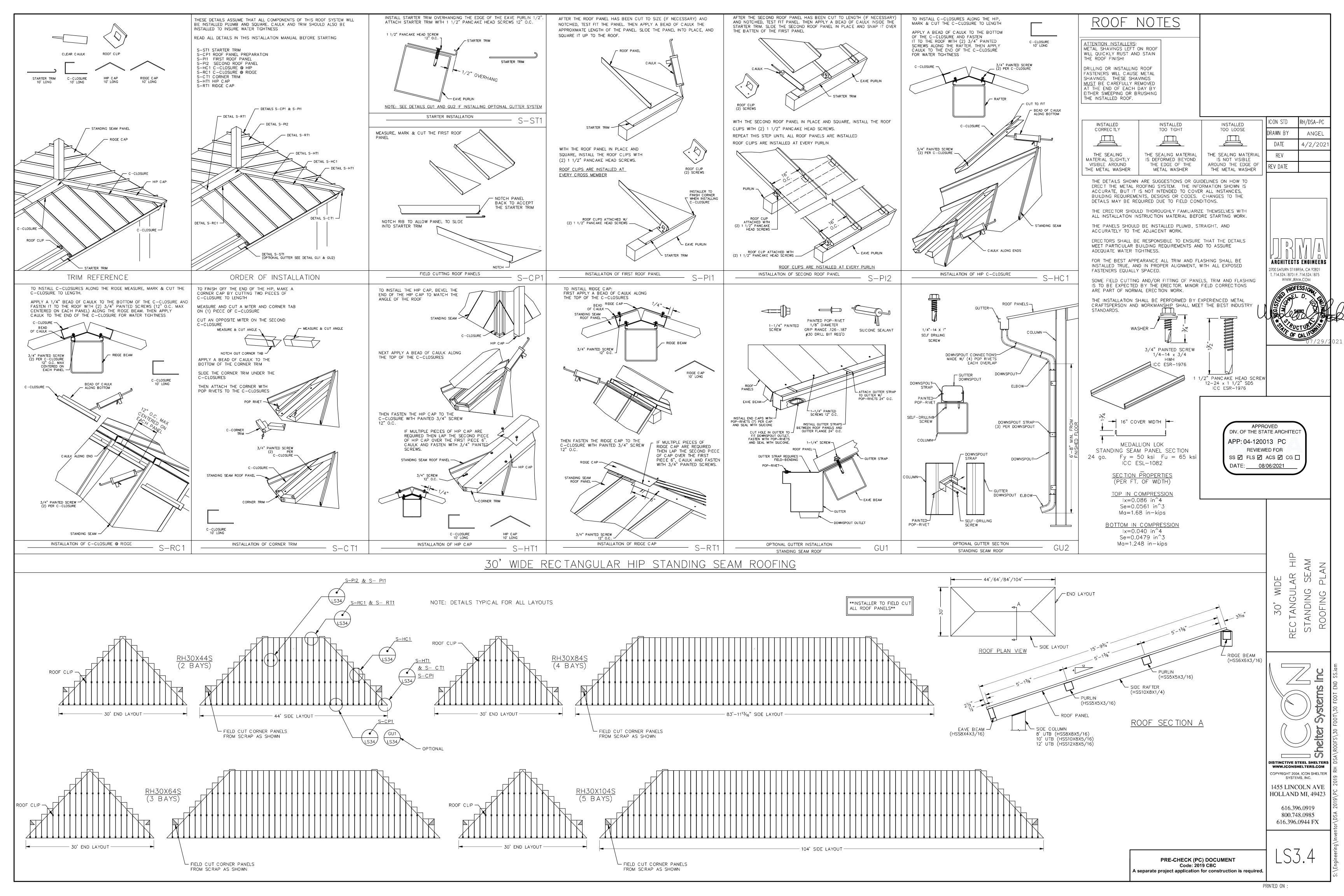
 $t ar{eta}$ P1 & BP2 are (4) bolt pattern while b3 is a (6) b $\delta$ L^

PRINTED ON :

616.396.0919 800.748.0985

616.396.0944 FX



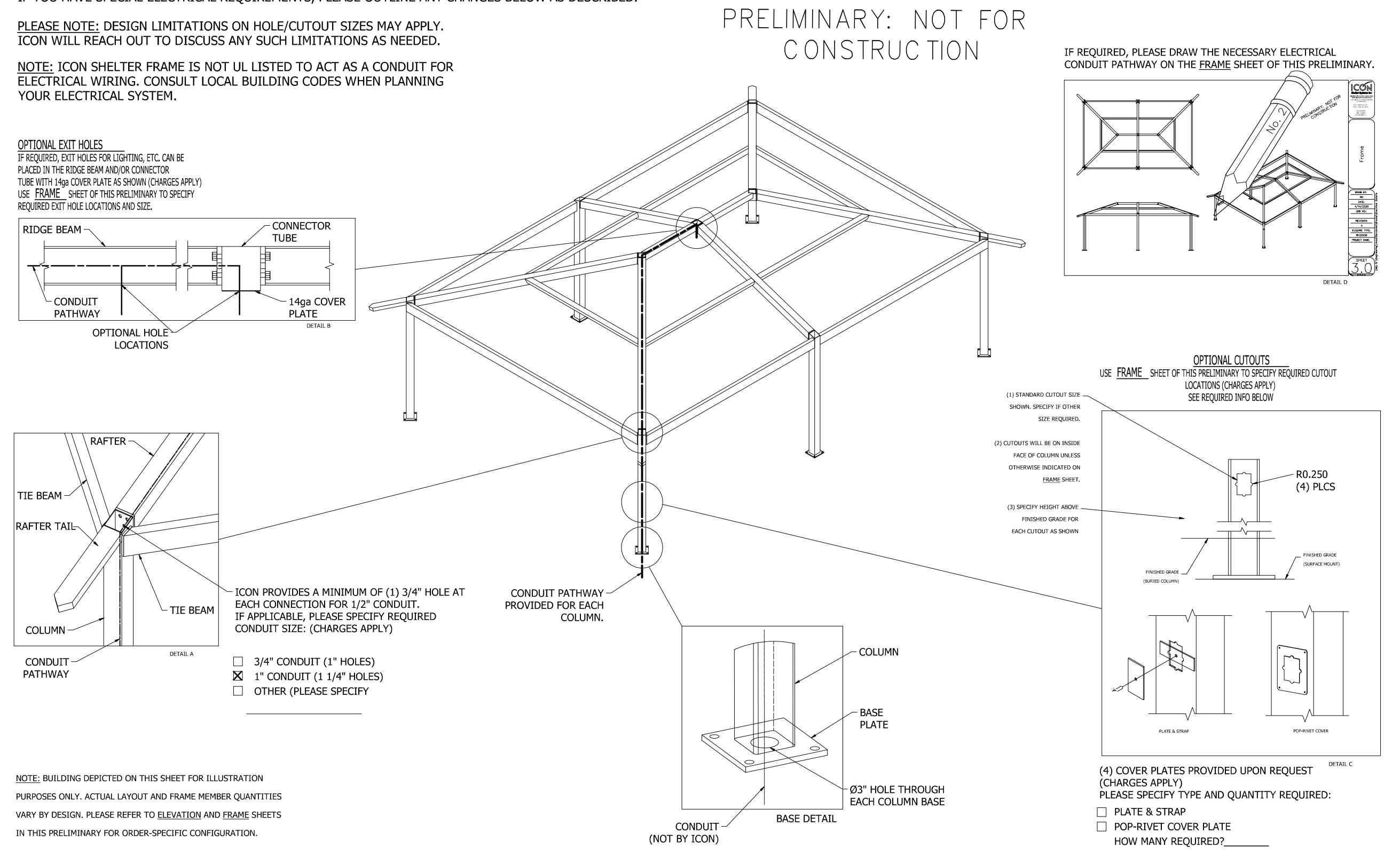


# ELECTRICAL INFORMATION - RECTANGULAR HIP

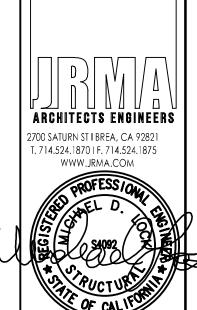
ICON'S STANDARD ELECTRICAL IS DESIGNED TO ACCOMMODATE Ø1/2" CONDUIT WITH A Ø3" INLET HOLE ON THE BOTTOM OF EACH COLUMN. THE CONDUIT PATHWAY RUNS THROUGH THE COLUMN, RAFTER, AND RIDGE BEAM THROUGH ALL BOLTED CONNECTIONS AS SHOWN. IF YOU HAVE SPECIAL ELECTRICAL REQUIREMENTS, PLEASE OUTLINE ANY CHANGES BELOW AS DESCRIBED.

2. ELECTRICAL EXIT HOLES (DETAIL B) 3. ELECTRICAL ACCESS & COVER PLATES (DETAIL C) 4. ELECTRICAL CONDUIT PATHWAY (DETAIL D)

1. CONDUIT HOLE SIZE (DETAIL A)

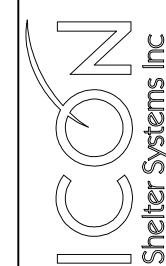


REV DATE



APPROVED DIV. OF THE STATE ARCHITEC APP: 04-120013 PC SS 🗹 FLS 🗹 ACS 🗹 CG 🗌

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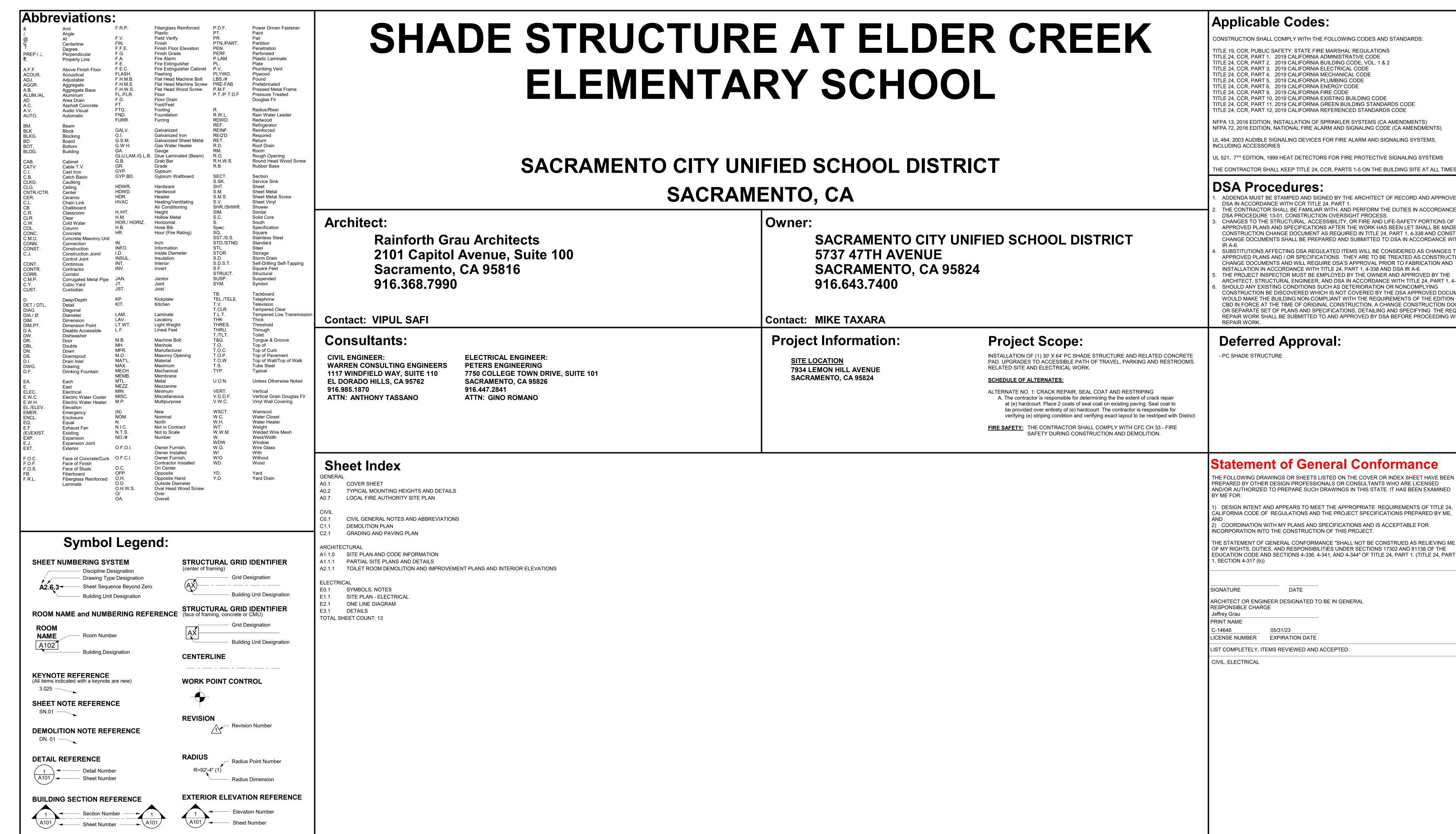


COPYRIGHT 2004, ICON SHELTER 1455 LINCOLN AVE HOLLAND MI, 49423 616.396.0919

> 800.748.0985 616.396.0944 FX

PRE-CHECK (PC) DOCUMENT Code: 2019 CBC

A separate project application for construction is required.



**SPECIAL ELEVATION REFERENCE** 

**WALL SECTION REFERENCE** 

1 Section Number

TITLE 24, CCR, PART 11, 2019 CALIFORNIA GREEN BUILDING STANDARDS CODE

JL 521,  $\,$   $^{ extsf{TH}}$  EDITION, 1999 HEAT DETECTORS FOR FIRE PROTECTIVE SIGNALING SYSTEM

THE CONTRACTOR SHALL BE FAMILIAR WITH, AND PERFORM THE DUTIES IN ACCORDANCE WITH CHANGES TO THE STRUCTURAL, ACCESSIBILITY, OR FIRE AND LIFE-SAFETY PORTIONS OF THE APPROVED PLANS AND SPECIFICATIONS AFTER THE WORK HAS BEEN LET SHALL BE MADE BY A CONSTRUCTION CHANGE DOCUMENT AS REQUIRED IN TITLE 24, PART 1, 4-338 AND CONSTRUCTION

INSTALLATION IN ACCORDANCE WITH TITLE 24, PART 1, 4-338 AND DSA IR A-6.



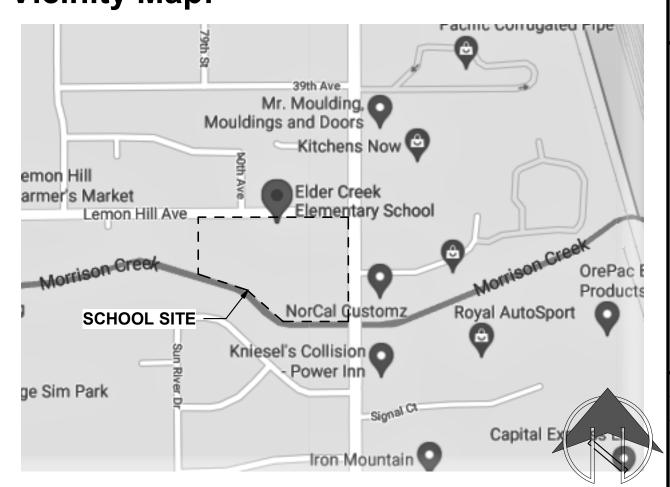
THE FOLLOWING DRAWINGS OR SHEETS LISTED ON THE COVER OR INDEX SHEET HAVE BEEN PREPARED BY OTHER DESIGN PROFESSIONALS OR CONSULTANTS WHO ARE LICENSED AND/OR AUTHORIZED TO PREPARE SUCH DRAWINGS IN THIS STATE. IT HAS BEEN EXAMINED

) DESIGN INTENT AND APPEARS TO MEET THE APPROPRIATE REQUIREMENTS OF TITLE 24. CALIFORNIA CODE OF REGULATIONS AND THE PROJECT SPECIFICATIONS PREPARED BY ME, 2) COORDINATION WITH MY PLANS AND SPECIFICATIONS AND IS ACCEPTABLE FOR

> TURE HADI LEMI S Ш

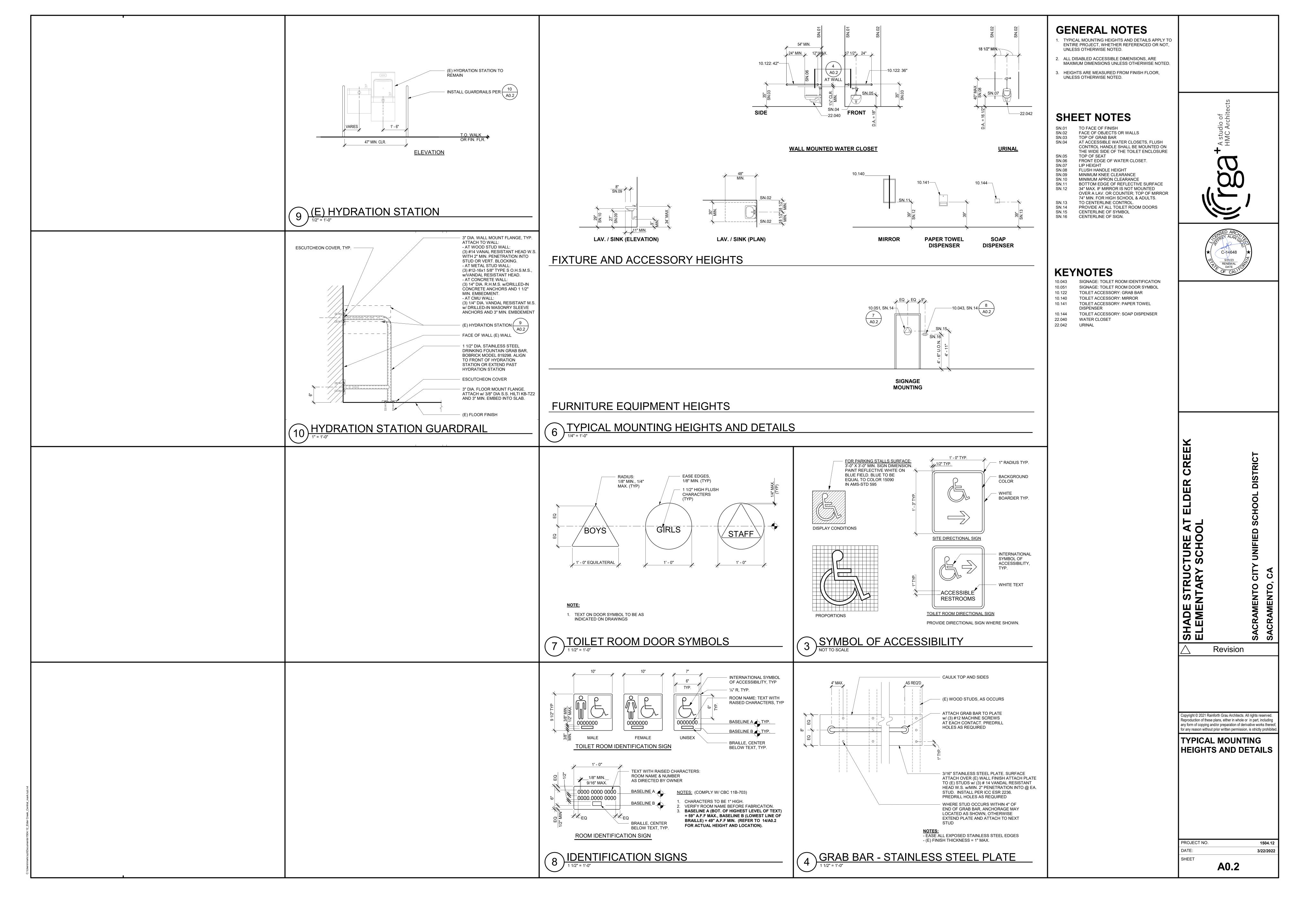
Revision

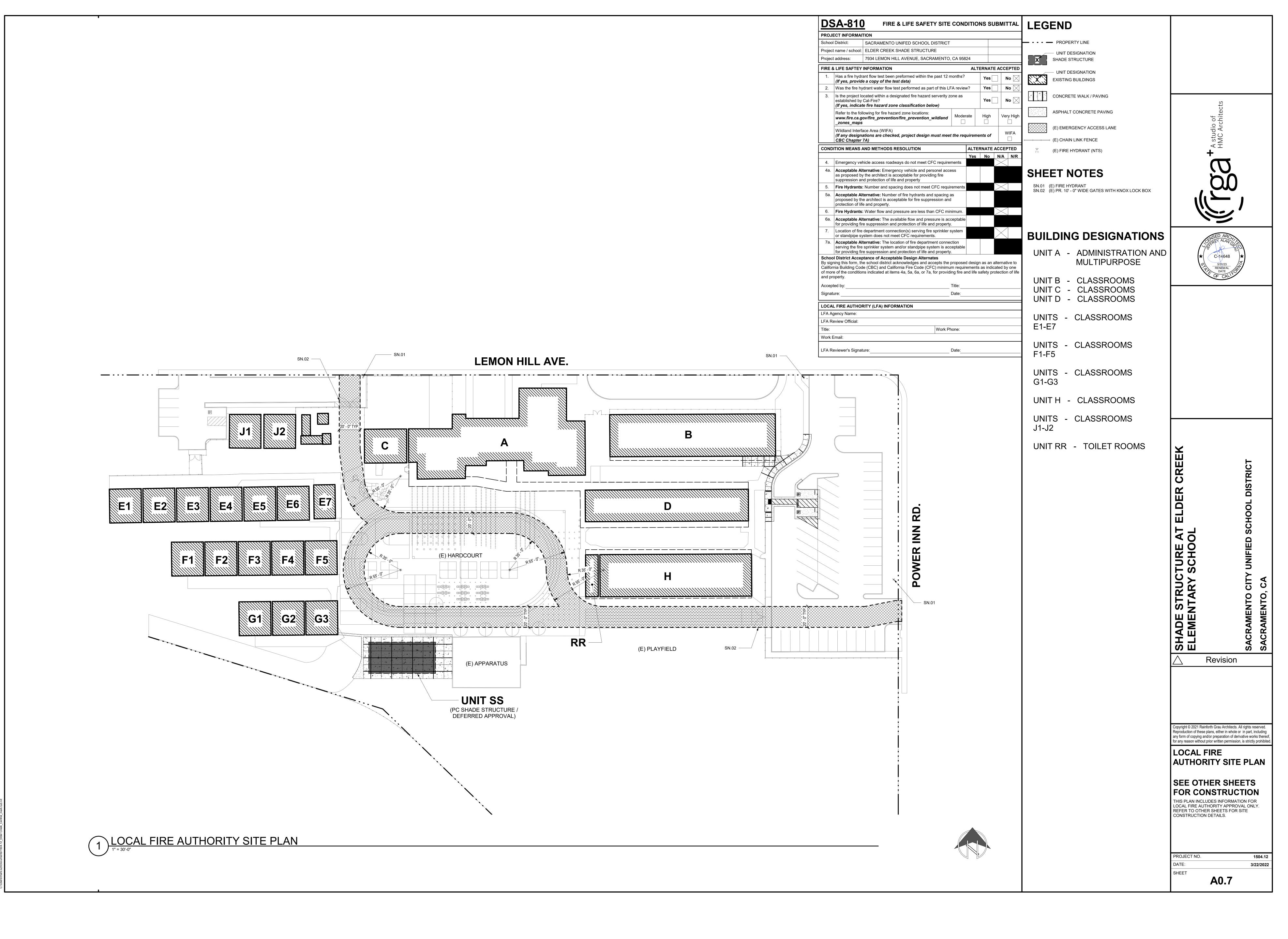
**Vicinity Map:** 



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COVER SHEET





= PROPERTY CORNER NOTHING FOUND OR SET = TEMPORARY BENCHMARK (SEE TBM LIST FOR INFO) = SWALE OR DRAINAGE FLOW

= DRAINAGE FLOW = FENCE (TYPE NOTED) = TREE (SIZE/TYPE INDICATED)

= SLOPE \_\_\_\_\_ 100 \_\_\_\_\_ = CONTOUR = CONCRETE SURFACE = EDGE OF ASPHALT = EDGE OF BUILDING 7/ // // // = SIGN = POST OR BOLLARD = GROUND ELEVATION

**EXISTING UTILITIES** 

= HARD SURFACE ELEVATION

= STORM DRAIN LINE (SIZE & DIRECTION OF FLOW)  $\underline{\phantom{a}}$  = STORM DRAIN LINE (RECORD INFORMATION)  $\underline{\phantom{a}}$  12"SD  $\underline{\phantom{a}}$  = STORM DRAIN LINE (UNDERGROUND LOCATING)

= STORM DRAIN MANHOLE = STORM DRAIN CLEANOUT = DROP INLET = AREA DRAIN

= RAIN WATER LEADER = DOWNSPOUT SANITARY SEWER LINE (SIZE & DIRECTION OF FLOW) \_\_\_\_\_12~SS \_ = SANITARY SEWER LINE (RECORD INFORMATION)

\_\_\_\_\_\_\_\_\_\_\_\_\_\_\_\_ = SANITARY SEWER LINE (UNDERGROUND LOCATING) = SANITARY SEWER MANHOLE = SANITARY SEWER CLEANOUT = WATER LINE (SIZE INDICATED)

- -W - -W = WATER LINE (RECORD INFORMATION)-W- - W = WATER LINE (UNDERGROUND LOCATING)

= WATER MANHOLE = WATER VALVE = WATER METER

= WATER BOX = IRRIGATION CONTROL VALVE = FIRE HYDRANT

= BACKFLOW PREVENTER

= SPRINKLER = HOSE BIBB

-OH-E- = OVERHEAD ELECTRIC LINE ——E—— = UNDERGROUND ELECTRIC LINE

---E---= UNDERGROUND ELECTRIC LINE — — E — — = UNDERGROUND ELECTRIC LINE (UNDERGROUND LOCATING)

= ELECTRIC MANHOLE

= UTILITY POLE (WITH GUY WIRE) = ELECTRIC METER

= ELECTRIC BOX = STREET LIGHTING BOX  $\square$   $\square$   $\square$   $\square$   $\square$  = LIGHT STANDARD

□ □ □ □ = SIGNAL LIGHT = FLOOD LIGHT = ELECTRICAL OUTLET

---G---= GAS LINE (RECORD INFORMATION)

--G--= GAS LINE (UNDERGROUND LOCATING) = GAS MANHOLE

= GAS VALVE = GAS METER --- T --- = TELEPHONE LINE

20 CPS CHISELED "+"

---T---= TELEPHONE LINE (RECORD INFORMATION) -- T -- = TELEPHONE LINE (UNDERGROUND LOCATING)

= STORM DRAIN BOX = TRAFFIC SIGNAL BOX

TBM LIST NUMBER DESCRIPTION NORTHING EASTING ELEV CPS PICKER 9826.08 9887.52 35.33 CPF BM318-D6B EL=39.128 9626.23 10408.31 39.13 CPS CHISELED "+" 9926.31 10335.06 36.81 4 CPS CHISELED "+"@LIFT STA 10027.66 10326.68 39.06 CPS CHISELED "+" 9947.25 10206.39 37.16 CPS CHISELED "+" 9999.42 10051.23 37.01 CPS CHISELED "+" 10068.98 10044.86 38.28 8 CPS CHISELED "+" 10132.52 10043.86 39.36 9 CPS CHISELED "+" 10118.88 9942.60 39.41 10 CPS CHISELED "+" 10109.46 9863.00 39.44 11 CPS CHISELED "+" 10213.76 9790.60 36.52 12 CPS CHISELED "+" 10071.81 9790.91 37.94 13 CPS CHISELED "+" 9956.32 9754.16 36.90 14 CPS PK&WASHER 10013.73 9549.89 36.13 15 CPS CHISELED "+" 9940.09 9971.33 36.90 16 CPF CHISELED "+" 10214.15 9443.22 36.79 17 CPS CHISELED "+" 10166.91 9539.19 37.64 18 CPS CHISELED "+" 10151.92 9640.82 37.70 19 CPS CHISELED "+" 10114.53 10239.27 38.56

10215.06 10234.42 37.34

#### CIVIL ABBREVIATIONS AND LEGEND

**ABBREVIATIONS** 

NOTE: NOT ALL ABBREVIATIONS

MAY BE USED ON THESE PLANS.

AGGREGATE BASE

AREA DRAIN

ASPHALTIC CONCRETE

AIR RELEASE VALVE

BLOW-OFF VALVE

**BUTTERFLY VALVE** 

CABLE TELEVISION

COMMUNICATION

CURB RETURN

CONCRETE SURFACE

DOUBLE CHECK VALVE

DECOMPOSED GRANITE

DUCTILE IRON PIPE

EDGE OF PAVEMENT

FIRE SERVICE LINE

DOUBLE DETECTOR CHECK VALVE

FIRE DEPARTMENT CONNECTION

SANITARY SEWER FORCE MAIN

HIGH DENSITY POLYETHYLENE PIPE

FINISHED FLOOR ELEVATION

BACK OF WALK

**CENTERLINE** 

**CLEANOUT** 

CONCRETE

CONSTRUCT

DROP INLET

DOWNSPOUT

ELECTRIC

EASEMENT

**EXISTING** 

FLOWLINE

FIRE HYDRANT

GATE VALVE

HEADER BOARD

PIPE INVERT ELEVATION

PORTLAND CEMENT CONCRETE

JOINT UTILITY POLE

HOSE BIBB

HIGH POINT

LINEAL FEET

**MOWSTRIP** 

OVERHEAD

LEFT

LIP OF GUTTER

NOT TO SCALE

PLANTER DRAIN

PROPERTY LINE

RIGHT OF WAY

STORM DRAIN

SANITARY SEWER

SCHEDULE

**STANDARD** 

SIDEWALK

UTILITY

WATER

WITHOUT

WATER VALVE

WITH

TELEPHONE

TOP OF CURB

TRENCH DRAIN

TELEPHONE POLE

TOP OF SEAT WALL

VITRIFIED CLAY PIPE

UNDERGROUND

POWER POLE

POST INDICATOR VALVE

PUBLIC UTILITY EASEMENT

STORM DRAIN MANHOLE

SANITARY SEWER MANHOLE

TRENCH DRAIN CATCH BASIN

TOP OF RAMP ELEVATION

TOP OF WALK ELEVATION

UNLESS OTHERWISE NOTED

TOP OF RETAINING WALL

SUBGRADE ELEVATION

REINFORCED CONCRETE PIPE

MANHOLE RIM ELEVATION (SOLID COVER)

REDUCED PRESSURE BACKFLOW PREVENTER

POLYVINYL CHLORIDE

GRATE ELEVATION

GRADE ELEVATION

DIAMETER

DRAWING

CLASS

CATV

COMM

CONC.

CONST.

CO

CR

DWG

**ESMT** 

**HDPE** 

SCH

STD

W/

W/O

CATCH BASIN

AGGREGATE SUB-BASE

CORRUGATED METAL PIPE

ASSESSOR'S PARCEL NUMBER

<u>LEGEND</u> NOTE: NOT ALL SYMBOLS MAY BE USED ON THESE PLANS. PROPOSED GRADING & DRAINAGE SYMBOLS: 8" SD STORM DRAIN LINE (SIZE AND FLOW SHOWN) STORM DRAIN MANHOLE (SDMH) ——— CATCH BASIN (CB) ——— DROP INLET (DI) —— AREA DRAIN (AD) PLANTER DRAIN (PD) OR FLOOR DRAIN (FD) STORM DRAIN CLEANOUT ELEVATION FINISHED FLOOR ELEVATION BUILDING PAD ELEVATION PAD = 99.33CONCRETE SIDEWALK GRADED DIRECTION FOR DRAINAGE FLOW  $\longrightarrow$ ---- SWALE

TREE TO BE REMOVED RETAINING WALL

PROPOSED SANITARY SEWER SYMBOLS: 6" SS SANITARY SEWER LINE (SIZE AND FLOW SHOWN) SANITARY SEWER MANHOLE (SSMH)

SEWER CLEANOUT FLUSHER BRANCH

PROPOSED WATER SYMBOLS:

8" RW RECLAIMED WATER LINE & SIZE 8" IRR IRRIGATION SERVICE LINE & SIZE 8" NP NON POTABLE WATER LINE & SIZE 8" SP FIRE SPRINKLER SERVICE LINE & SIZE <del>────</del> GATE VALVE

———M——— WATER METER → → → FH FIRE HYDRANT ASSEMBLY FIRE DEPARTMENT CONNECTION DETECTOR CHECK VALVE DOUBLE DETECTOR CHECK VALVE REDUCED PRESSURE BACKFLOW PREVENTER BUTTERFLY VALVE AIR RELEASE VALVE + SIZE BLOW-OFF VALVE + SIZE POST INDICATOR VALVE

#### **DEMOLITION GENERAL NOTES**

- IN THE EVENT THAT ANY UNUSUAL CONDITIONS NOT COVERED BY THE GEOTECHNICAL INVESTIGATION REPORT OR ARE ENCOUNTERED DURING GRADING OPERATIONS THE GEOTECHNICAL ENGINEER AND THE ARCHITECT SHALL BE IMMEDIATELY NOTIFIED FOR DIRECTIONS.
- 2. NO BURNING OR BLASTING SHALL BE PERMITTED.
- ADDITIONAL DEMOLITION INFORMATION MAY BE SHOWN ON THE GRADING, DRAINAGE, AND UTILITY PLANS, AND THOSE PLANS PREPARED BY OTHER DISCIPLINES FOR THIS PROJECT.
- 4. ALL DEMOLISHED ITEMS SHALL BE DISPOSED OF OFFSITE AT A SUITABLE, LEGAL, DUMP SITE OR OTHER FACILITY.
- 5. ALL DISPOSED OF MATERIALS SHALL BE RECYCLED IF POSSIBLE
- 6. THE TYPES, LOCATIONS, SIZES AND/OR DEPTHS OF EXISTING UNDERGROUND UTILITIES AS SHOWN IN THESE PLANS WERE OBTAINED FROM SOURCES OF VARYING RELIABILITY. THE CONTRACTOR IS CAUTIONED THAT ONLY ACTUAL EXCAVATION WILL REVEAL THE TYPES, EXTENT, SIZES, LOCATIONS, AND DEPTHS OF SUCH UNDERGROUND UTILITIES. A REASONABLE EFFORT HAS BEEN MADE TO LOCATE AND DELINEATE ALL KNOWN UNDERGROUND UTILITIES. HOWEVER, WARREN CONSULTING ENGINEERS CAN ASSUME NO RESPONSIBILITY FOR THE COMPLETENESS OR ACCURACY OF ITS DELINEATION OF SUCH UNDERGROUND UTILITIES, NOR FOR THE EXISTENCE OF OTHER BURIED OBJECTS OR UTILITIES WHICH MAY BE ENCOUNTERED BUT WHICH ARE NOT SHOWN ON THESE DRAWINGS. THE CONTRACTOR OR ANY SUBCONTRACTOR FOR THIS CONTRACT SHALL NOTIFY THE DISTRICT TWO (2) WORKING DAYS IN ADVANCE OF PERFORMING ANY EXCAVATION WORK IN ORDER TO VERIFY TO THE GREATEST EXTENT POSSIBLE THE EXISTING UTILITY LINES, CONFLICTS AND PROPOSED UTILITY CONNECTION POINTS.
- 7. THE SCHOOL DISTRICT SHALL HAVE SALVAGE RIGHTS TO ANY DEMOLISHED ITEMS SHOWN HEREON. THE CONTRACTOR SHALL GIVE THE DISTRICT NOTICE 7 DAYS PRIOR TO THE START OF DEMOLITION. THE DISTRICT SHALL MOVE ANY RETAINED ITEMS OUT OF THE CONTRACTORS WORK AREA, UNLESS ANOTHER ARRANGEMENT IS MADE WITH THE CONTRACTOR. ANY REMAINING ITEMS BECOME THE PROPERTY OF THE CONTRACTOR AND SHALL BE REMOVED FROM THE SITE. ANY ITEMS NOT SHOWN FOR REMOVAL SHALL REMAIN AND SHALL BE PROTECTED FROM DAMAGE DURING CONSTRUCTION TO A REASONABLE
- 8. EXISTING UTILITY STRUCTURES IN AREAS OF NEW PAVING SHALL BE REMOVED AND REPLACED WITH NEW BOX/COVER AT NEW GRADE UNLESS SPECIFICALLY NOTED OTHERWISE.
- 9. ITEMS OUTSIDE THE LIMITS OF DEMOLITION SHALL REMAIN AND BE PROTECTED FROM DAMAGE DURING CONSTRUCTION.
- 10. EXISTING UTILITY STRUCTURES AND PIPING NOT SHOWN ON DEMOLITION PLAN TO BE REMOVED SHALL REMAIN AND BE PROTECTED.

#### UTILITY VERIFICATION NOTE

PRIOR TO THE START OF CONSTRUCTION, VERIFY AND POTHOLE ALL UTILITY POINTS OF CONNECTION FOR LOCATION DEPTH. AND SIZE, IF CONFLICT IS FOUND. CONTACT THE ENGINEER IMMEDIATELY FOR

#### IRRIGATION DEMOLITION NOTE

WITHIN LANDSCAPE AREAS TO BE DEMOLISHED THERE MAY BE EXISTING IRRIGATION LINES NOT SHOWN ON THIS PLAN. CONTRACTOR SHALL REMOVE LATERAL LINES AND HEADS ENCOUNTERED. MAIN LINES AND CONTROL WIRES MAY ONLY BE REMOVED PROVIDED THAT ROUTING IS KNOWN AND REMOVAL WILL NOT DEACTIVATE AN IRRIGATION SYSTEMS INTENDED TO REMAIN. IF CONFLICT IS FOUND, CONTACT THE ENGINEER FOR DIRECTION.

#### **GENERAL NOTES:**

 THE TYPES, LOCATIONS, SIZES, AND/OR DEPTHS OF EXISTING UNDERGROUND UTILITIES AS SHOWN ON THESE PLANS WERE OBTAINED FROM SOURCES OF VARYING RELIABILITY THE CONTRACTOR IS CAUTIONED THAT ONLY ACTUAL EXCAVATION WILL REVEAL THE TYPES, EXTENT, SIZES, LOCATIONS AND DEPTHS OF SUCH UNDERGROUND UTILITIES. A REASONABLE EFFORT HAS BEEN MADE TO LOCATE AND DELINEATE ALL KNOWN UNDERGROUND UTILITIES. HOWEVER. WARREN CONSULTING ENGINEERS CAN ASSUME NO RESPONSIBILITY FOR THE COMPLETENESS OR ACCURACY OF ITS DELINEATION OF SUCH UNDERGROUND UTILITIES, NOR FOR THE EXISTENCE OF OTHER BURIED OBJECTS OR UTILITIES WHICH MAY BE ENCOUNTERED BUT WHICH ARE NOT SHOWN ON THESE PLANS. THE CONTRACTOR OR ANY SUBCONTRACTOR FOR THIS CONTRACT SHALL NOTIFY MEMBERS OF UNDERGROUND SERVICE ALERT (USA) TWO (2) WORKING DAYS IN ADVANCE OF

PERFORMING ANY EXCAVATION WORK BY CALLING TOLL FREE 1—800—227—2600, OR 811.



- . WARREN CONSULTING ENGINEERS, INC. (WCE) ASSUMES NO RESPONSIBILITY FOR ERRORS IN PHYSICAL LOCATION OF IMPROVEMENTS, HORIZONTAL OR VERTICAL, IF STAKED BY OTHERS. IN ADDITION, ANY SUCH ERRORS IN PHYSICAL LOCATION MAY AFFECT THE INTENDED DESIGN OF SUCH IMPROVEMENTS AND WCE CANNOT BE HELD RESPONSIBLE FOR SUCH CONDITIONS WHICH ARE A RESULT OF ERRORS IN SURVEYING, OR IMPROPER CONSTRUCTION.
- 3. IF SUBSURFACE CULTURAL RESOURCES, REMAINS, AND/OR ARTIFACTS ARE UNCOVERED DURING PROJECT CONSTRUCTION, ALL WORK IN THE VICINITY SHALL BE STOPPED UNTIL SUCH ITEMS CAN BE ASSESSED BY AN APPROPRIATE MEMBER OF THE COUNTY ENVIRONMENTAL IMPACT SECTION STAFF.
- 4. CONTRACTOR AGREES THAT HE/SHE SHALL ASSUME SOLE AND COMPLETE RESPONSIBILITY FOR JOB SITE CONDITIONS DURING THE COURSE OF CONSTRUCTION OF THIS PROJECT, INCLUDING SAFETY OF ALL PERSONS AND PROPERTY: THAT THIS REQUIREMENT SHALL APPLY CONTINUOUSLY AND SHALL NOT BE LIMITED TO NORMAL WORKING HOURS: AND THAT THE CONTRACTOR SHALL DEFEND, INDEMNIFY AND HOLD THE OWNER AND ENGINEER HARMLESS FROM ANY AND ALL LIABILITY, REAL OR ALLEGED, IN CONNECTION WITH THE PERFORMANCE OF WORK ON THIS PROJECT, EXCEPTING FOR LIABILITY ARISING FROM THE SOLE NEGLIGENCE OF THE OWNER OR ENGINEER.
- 5. THE CONTRACTOR SHALL OBTAIN AN EXCAVATION PERMIT FROM THE STATE OF CALIFORNIA DEPARTMENT OF INDUSTRIAL SAFETY FOR ALL EXCAVATIONS OF 5 FEET OR MORE IN DEPTH.
- 6. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO MAKE ALL NECESSARY PRE-BID AND PRE-CONSTRUCTION SITE INSPECTION. AND/OR OBSERVATIONS ON THE SITE TO PRE-DETERMINE ALL HIS/HER MEANS AND METHODS NECESSARY TO COMPLETE THE IMPROVEMENTS SHOWN ON THESE PLANS AND PER THE PROJECT SPECIFICATIONS. IT IS THE CONTRACTORS RESPONSIBILITY TO DETERMINE, AND INCLUDE IN HIS/HER CONTRACT, ALL MEANS AND METHODS NECESSARY TO PERFORM A COMPLETE AND ACCEPTABLE JOB.
- . WHERE IMPROVEMENTS LIE WITHIN AN EXISTING DEVELOPED AREA, CONTRACTOR SHALL USE CAUTION WHEN ACCESSING THE SITE THROUGH THESE EXISTING IMPROVEMENTS. IT IS THE CONTRACTORS RESPONSIBILITY TO PROTECT ANY SUCH EXISTING IMPROVEMENTS OUTSIDE THE PROJECT BOUNDARY, OR EXISTING IMPROVEMENTS WITHIN THE BOUNDARY WHICH ARE TO REMAIN. PROPER PRECAUTIONS SHALL BE PROVIDED AND MAINTAINED THROUGHOUT CONSTRUCTION. ANY DAMAGE SHALL BE REPAIRED OR REPLACED TO THE SATISFACTION OF THE
- 8. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO KEEP DETAILED RECORDS OF MINOR CHANGES OR ADJUSTMENTS MADE DURING CONSTRUCTION (WHICH WERE NOT FORMALLY ISSUED). UPON PROJECT COMPLETION, THESE RECORDS AND/OR INFORMATION SHALL BE PROVIDED TO THE OWNER AND WARREN CONSULTING ENGINEERS, INC. UNLESS AN OFFICIAL "AS-BUILT" SET OF PLANS IS A REQUIREMENT OF THE CONTRACT. IF AS-BUILT PLANS ARE A REQUIREMENT OF THE CONTRACT, REFER TO SPECIFICATIONS FOR AS-BUILT DELIVERABLE REQUIREMENTS.
- 9. IN VEHICULAR PATHWAYS, EXISTING ASPHALTIC AND/OR CONCRETE SURFACES SHALL BE CUT TO A NEAT AND STRAIGHT LINE, PARALLEL OR PERPENDICULAR TO THE VEHICULAR TRAVELED PATH. THIS IS TYPICALLY THE ROADWAY CENTERLINE, BUT MAY VARY. THAT SAWCUT EDGE SHALL BE PROTECTED FROM DAMAGE DURING CONSTRUCTION SO A CLEAN EDGE REMAINS FOR PATCH BACK.. IF EDGE IS DAMAGED, A NEW SAW CUT WILL BE REQUIRED. THE EXPOSED EDGE SHALL BE "TACKED" WITH EMULSION PRIOR TO PAVING.
- 10. NO BURNING OR BLASTING SHALL BE ALLOWED ONSITE UNLESS SPECIFICALLY ADDRESSED ON PLANS, OR SPECIFICALLY APPROVED AND COORDINATED WITH THE ARCHITECT, ENGINEER, AND LOCAL AGENCY OR OTHER ADMINISTRATIVE AUTHORITY.
- 11. SUBGRADE AND RESULTING FINISHED GRADE SHALL BE CONSTRUCTED SMOOTH AND UNIFORM BETWEEN SPOT ELEVATIONS, CONTOURS OR OTHER STRUCTURE ELEVATIONS SHOWN ON GRADING OR OTHER PLANS. NO MOUNDS, RUTS, DEPRESSIONS OR OTHER GRADING DEFICIENCIES WILL BE ALLOWED UNLESS SPECIFICALLY SHOWN ON PLANS.
- 12. ON NEW WATER SYSTEMS, SERVICE LATERALS SHALL BE MADE USING APPROPRIATE "TEE" AND "WYE" FITTINGS. SADDLE TAPS WILL ONLY BE ALLOWED WHEN MAKING CONNECTIONS TO EXISTING WATER MAINS.
- 13. CURING COMPOUND SHALL BE APPLIED IN A CONTINUOUS SOLID WET FLOWING COAT. ANY "SPOTTY" APPLICATIONS SHALL BE RECOATED IMMEDIATELY. APPLICATION SHALL BE INSPECTED BY PROJECT INSPECTOR DURING APPLICATION.
- 14. EMBEDMENT OF FEATURES IN CONCRETE PAVING, CURBS, OR WALLS, SUCH AS SQUARE OR ROUND TUBING, POSTS, OR COLUMNS, STEEL BOLTED PLATES, OR OTHER STRUCTURES, SHALL REQUIRE ADDITIONAL SCORE OR EXPANSION JOINTS TO PREVENT UNCONTROLLED CRACKING. THOSE ADDITIONAL JOINTS MAY OR MAY NOT BE SPECIFICALLY SHOWN ON PLANS BUT SHALL BE PROVIDED BY THE CONTRACTOR.
- 15. EMBEDMENT OF FEATURES IN CONCRETE PAVING, CURBS, OR WALLS, SUCH AS SQUARE OR ROUND TUBING, POSTS, OR COLUMNS, STEEL BOLTED PLATES, OR OTHER STRUCTURES, SHALL REQUIRE A MINOR ADJUSTMENT OF REBAR WITHIN CONCRETE TO ALLOW FOR SUCH STRUCTURE. THAT REBAR ADJUSTMENT MAY NOT BE SPECIFICALLY SHOWN 16. NO MORE THAN 1 GALLON OF WATER PER YARD OF CONCRETE CAN BE ADDED TO THE TRUCK AFTER ARRIVAL TO
- PROJECT SITE. THE ADDITION OF WATER CAN ONLY BE ADDED UNDER THE SUPERVISION OF THE CONCRETE INSPECTOR OR LABORATORY TECHNICIAN.

17. WHEN PUMPING CONCRETE FOR PLACEMENT, ABSOLUTELY NO WATER IS TO BE ADDED TO PUMP HOPPER. ANY

- WATER ADDED TO HOPPER WILL BE REASON FOR CONCRETE REJECTION AT THE CONTRACTORS EXPENSE. 18. ALL CONTRACTION/CONSTRUCTION JOINTS "CJ" SHALL BE 1/4 THE SLAB THICKNESS DEEP. BUT NO LESS THAN 1" FOR CONTROLLING OF CRACKING. CONTRACTOR SHALL EXERCISE CAUTION WHEN FINAL TROWELING OF CONCRETE SO AS NOT TO FILL IN THESE JOINTS WITH CONCRETE CREAM. ANY CRACKS OUTSIDE OF JOINTS WHICH WERE CONSTRUCTED LESS THAN 1" DEEP, SHALL BE CAUSE FOR CONCRETE SLAB(S) TO BE REMOVED AND REPLACE AT
- 19. ANY SCREED BOARDS SET WITHIN CONCRETE SLABS SHALL BE AN "OVERHEAD SCREED" SO THERE IS NO
- 20. 3-1/2" FELT JOINTS WILL NOT BE ACCEPTED. PROVIDE A FULL 4" FELT JOINT FOR 4" SLAB CONSTRUCTION, AND A 6" FELT JOINT FOR A 6" SLAB SLAB CONSTRUCTION.
- 21. SHOULD ANY SHRINKAGE CRACKS OCCUR OUTSIDE OF EITHER THE EXPANSION JOINTS OR CRACK CONTROL JOINTS, THEN THE CONCRETE SLAB SHALL BE SAWCUT AT THE NEAREST JOINTS ON EACH SIDE OF THE CRACK AND THE CONCRETE SECTION SHALL BE, REMOVED AND REPLACED. NEW CONCRETE SHALL BE DOWELED INTO EXISTING CONCRETE PER DRAWING DETAIL.
- 22. ALL AREAS DISTURBED BY GRADING OPERATIONS WHETHER SHOWN ON THE DRAWINGS OR NOT SHALL BE HYDRO SEEDED UNLESS OTHERWISE NOTED. HYDRO SEEDING SHALL CONFORM TO LOCAL CITY/COUNTY STANDARDS.
- 23. REPAIR OR PATCHING OF GALVANIZED METALS, SUCH AS AFTER WELDING GALVANIZED COMPONENTS, SHALL BE MADE USING A ZINC COMPOSITION "HOT STICK" APPLICATION PER ASTM A 780-01. GALVANIZING PAINTS WILL NOT BE ALLOWED.

#### **GENERAL PAVING SURFACE NOTES:**

INTERFERENCE WITH THE PLACEMENT AND ALIGNMENT OF SLAB REINFORCING.

- 1. PROVIDE EQUIVALENT OF MEDIUM BROOM FINISH AT SLOPES UP TO 5.99%, TYPICAL. PROVIDE EQUIVALENT OF HEAVY BROOM FINISH AT SLOPES 6% AND GREATER. REFER TO SPECIFICATIONS.
- 2. ALL NEW PEDESTRIAN WALKWAYS (NON-RAMP) SHALL BE SLOPED NO GREATER THAN 2.0%, AND NO LESS THAN 0.75% IN ANY DIRECTION, UNLESS SPECIFICALLY LABELED OTHERWISE. ALL CONCRETE SHALL MEET THE FOLLOWING SLOPE REQUIREMENTS: - NO GREATER THAN 5% SLOPE IN THE DIRECTION OF TRAVEL.
- NO GREATER THAN 2% SLOPE CROSSING THE DIRECTION OF TRAVEL. NO GREATER THAN 2% SLOPE IN ANY DIRECTION IN COURTYARD OR PLAZA AREAS.

#### CIVIL SHEET INDEX

- CO.1 CIVIL GENERAL NOTES AND ABBREVIATIONS
- C1.1 DEMOLITION PLAN
- C2.1 GRADING AND PAVING PLAN





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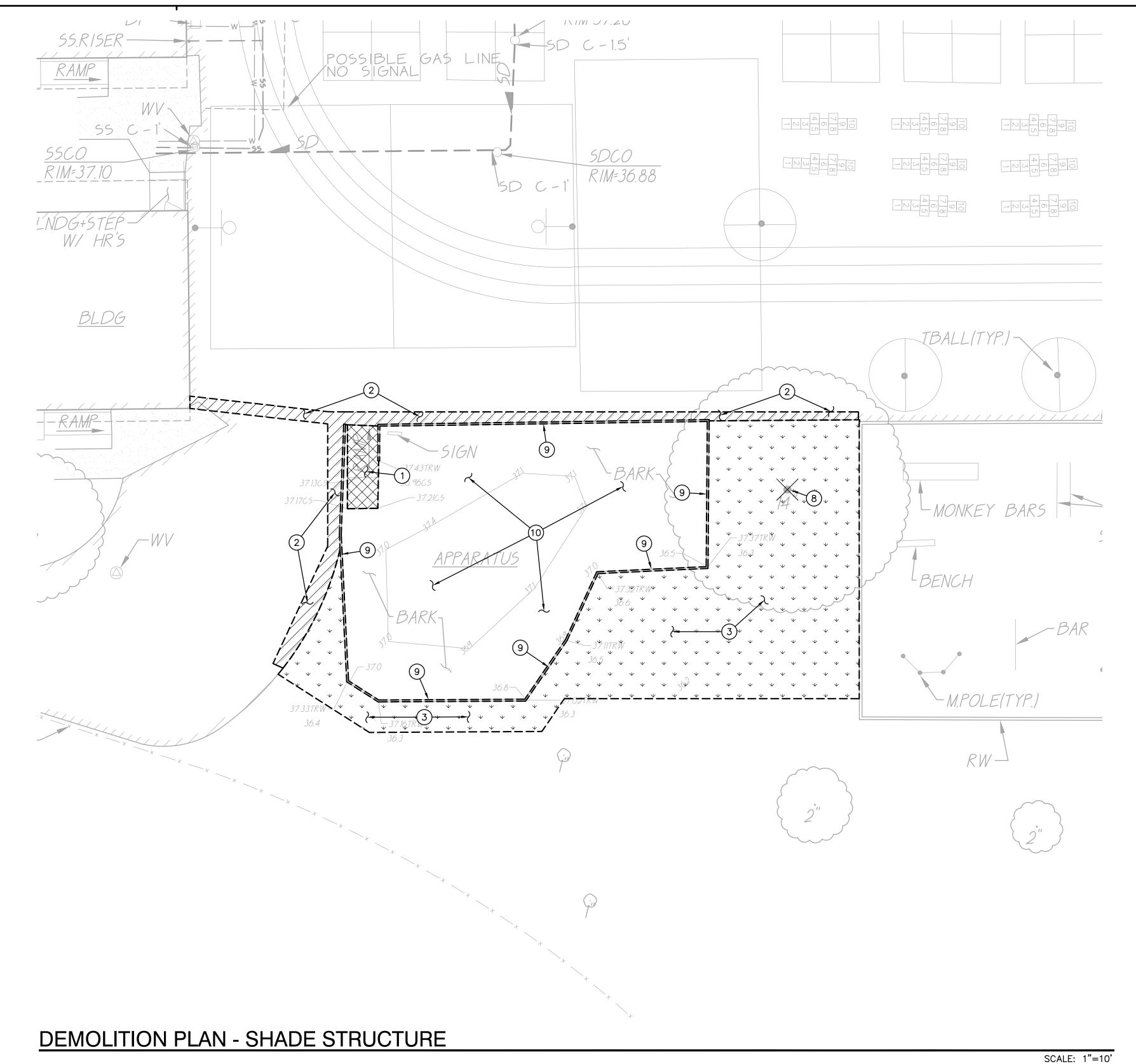
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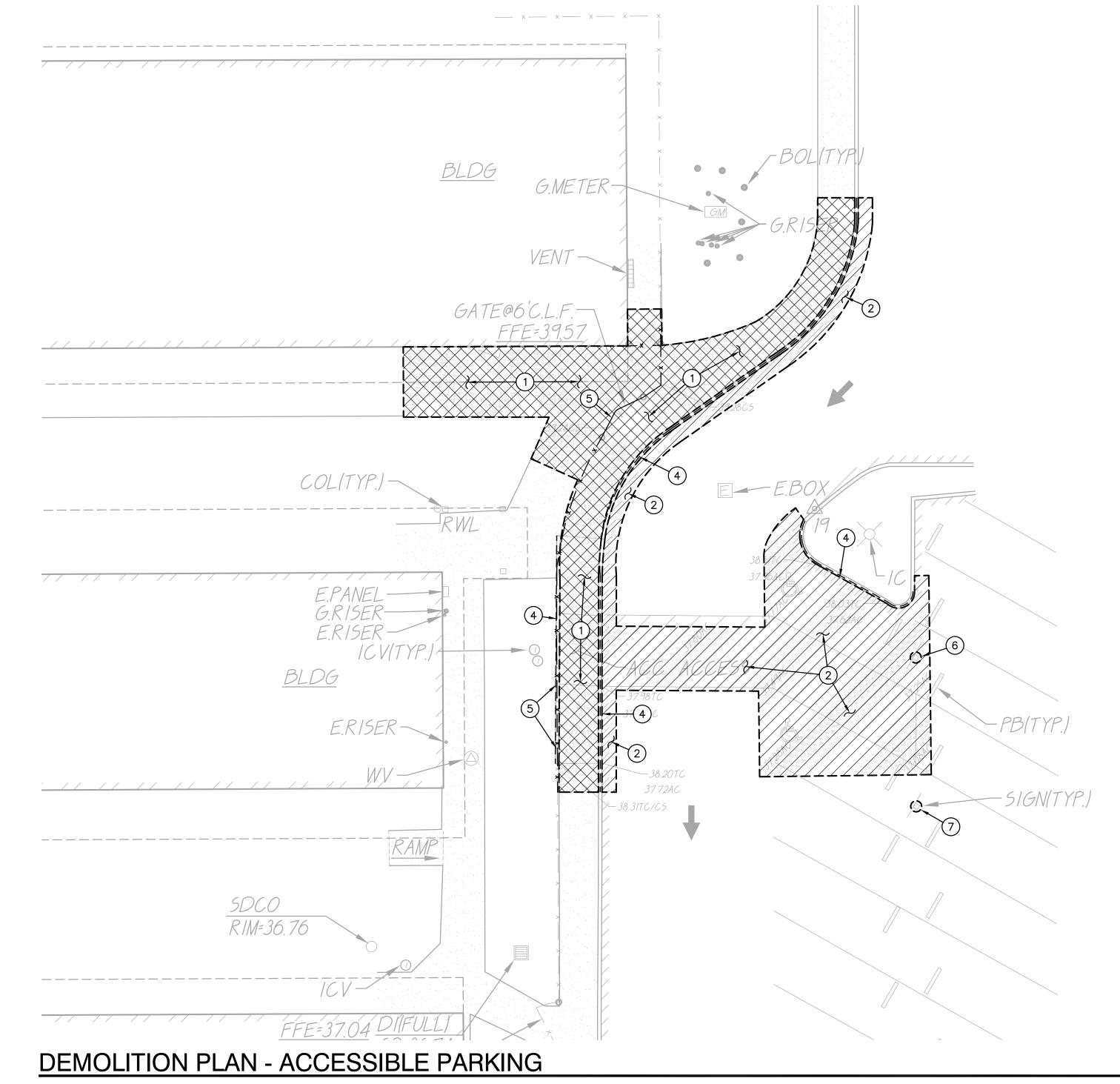
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**CIVIL GENERAL NOTES AND ABBREVIATIONS** 

PROJECT NO. 3/21/2022

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O DEMOLITION NOTES

1. SAWCUT, REMOVE AND DISPOSE OF EXISTING CONCRETE PAVING AND ASSOCIATED AGGREGATE BASE. SAWCUT SHALL BE A NEAT STRAIGHT LINE, MAINTAIN CLEAN, STRAIGHT CUT EDGE UNTIL NEW PAVING IS PLACED.

SAWCUT, REMOVE AND DISPOSE OF EXISTING ASPHALT PAVING AND ASSOCIATED AGGREGATE BASE. SAWCUT SHALL BE A NEAT STRAIGHT LINE, MAINTAIN CLEAN, STRAIGHT CUT EDGE UNTIL NEW PAVING IS PLACED.

3. REMOVE AND DISPOSE OF EXISTING LANDSCAPING, TURF AND ASSOCIATED IRRIGATION PIPING/SPRINKLERS WITHIN AREAS OF WORK. CUT AND CAP ANY MAINLINES NEAR WHERE THEY ENTER THE BOUNDARY OF THE PROJECT. MARK ALL CAPPED LINES WITH AN IRRIGATION VALVE BOX. ALL EXISTING IRRIGATION AREAS OUTSIDE THE PROJECT WORK AREA SHALL BE PRESERVED AND OPERATIONAL. INTEGRITY SHALL BE MAINTAINED WITH PROPER SPRINKLER COVERAGE TO TURF AREAS TO REMAIN.

---- REMOVE AND DISPOSE OF EXISTING CONCRETE CURB.
 x --- x -- REMOVE AND DISPOSE OF EXISTING CHAIN LINK FENCE, GATES, POSTS AND ASSOCIATED FOOTINGS.

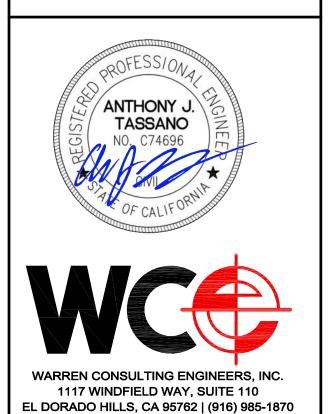
REMOVE AND DISPOSE OF EXISTING SIGN, POST AND ASSOCIATED FOOTINGS.

7. CUT POST FLUSH WITH PAVEMENT AND GROUT FILL POST HOLE.

8. REMOVE AND DISPOSE OF EXISTING TREE, TRUNK AND ASSOCIATED ROOTS.

9. REMOVE AND DISPOSE OF EXISTING PLASTIC APPARATUS CURB.

 REMOVE AND DISPOSE OF EXISTING BARK, PLAY APPARATUS AND ASSOCIATED FOOTINGS. A studio of HMC Architects



SHADE STRUCTURE AT ELDER CRI ELEMENTARY SCHOOL
SACRAMENTO CITY UNIFIED SCHOOL DISTRIC

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DEMOLITION PLAN

GRAPHIC SCALE

10' 0 5' 10' 20'

(IN FEET) I inch = 10 feet

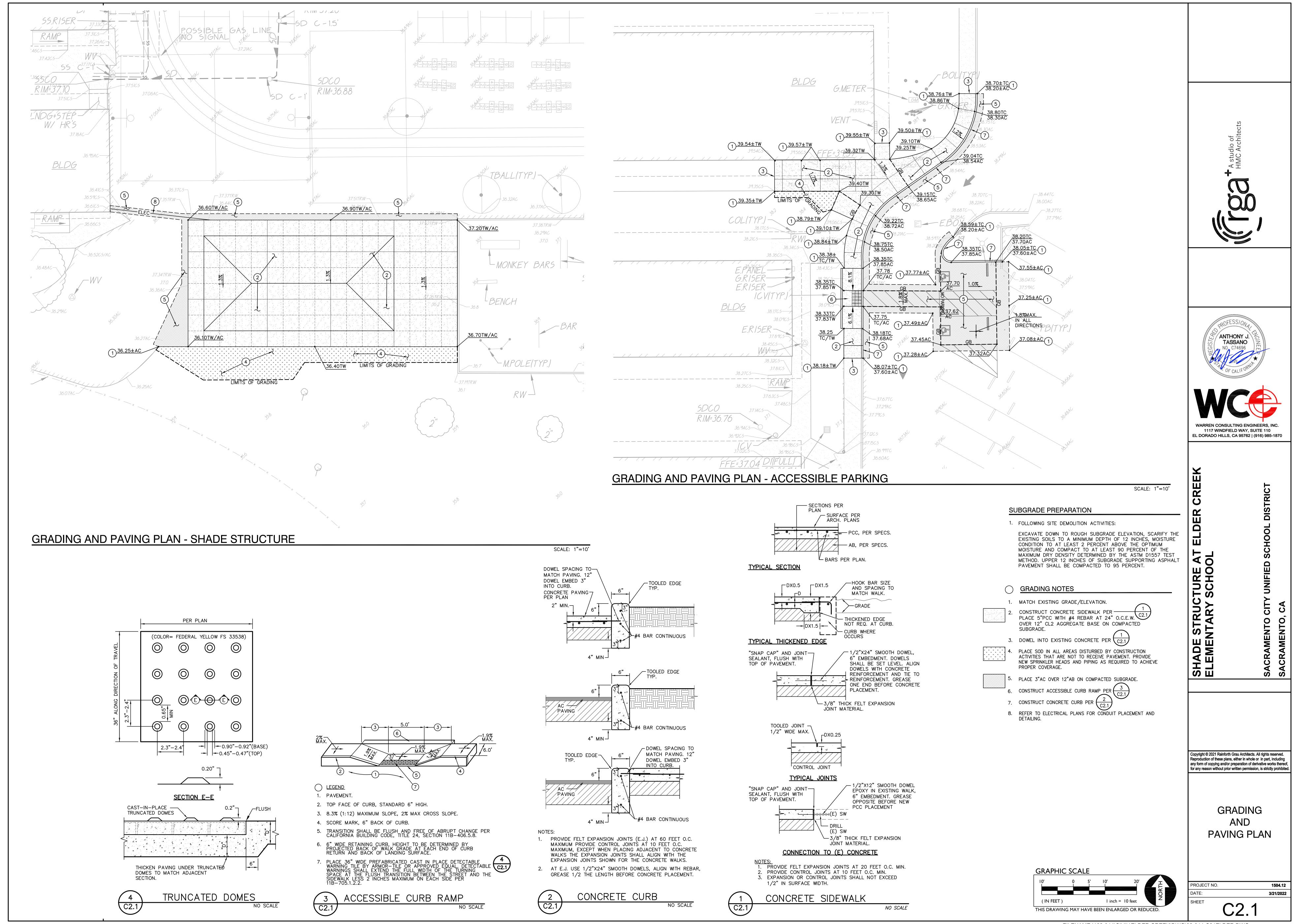
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PROJECT NO. 1504.12

DATE: 3/21/2022

SHEET 1

SCALE: 1"=10'



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**EXISTING PATH OF TRAVEL (POT): ARCHITECT STATEMENT** DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE IN CHARGE STATEMENT: THE POT IDENTIFIED IN THESE CONSTRUCTION DOCUMENTS IS COMPLIANT WITH THE CURRENT APPLICABLE CALIFORNIA BUILDING CODE ACCESSIBILITY PROVISIONS FOR PATH OF TRAVEL REQUIREMENTS FOR ALTERATIONS, ADDITIONS AND STRUCTURAL REPAIRS. AS PART OF THE DESIGN OF THIS

1) HAVE BEEN IDENTIFIED AND 2) THE CORRECTIVE WORK NECESSARY TO BRING THEM INTO COMPLIANCE HAS BEEN INCLUDED WITHIN THE SCOPE OF THIS PROJECT'S WORK THROUGH DETAILS, DRAWINGS, AND SPECIFICATIONS INCORPORATED INTO THESE CONSTRUCTION DOCUMENTS. ANY NONCOMPLIANT ELEMENTS, COMPONENTS OR PORTIONS OF THE POT THAT WILL NOT BE

CORRECTED BY THIS PROJECT BASED ON VALUATION THRESHOLD LIMITATIONS OR A FINDING OF UNREASONABLE HARDSHIP ARE SO INDICATED IN THESE CONSTRUCTION DOCUMENTS.

PROJECT, THE POT WAS EXAMINED AND ANY ELEMENTS, COMPONENTS OR PORTIONS OF THE

DURING CONSTRUCTION, IF POT ITEMS WITHIN THE SCOPE OF THE PROJECT REPRESENTED A CODE COMPLIANT ARE FOUND TO BE NON-CONFORMING BEYOND REASONABLE CONSTRUCT TOLERANCES, THEY SHALL BE BROUGHT TO COMPLIANCE WITH THE CBC AS PART OF THIS PROJECT BY MEANS OF A CONSTRUCTION CHANGE DOCUMENT.

ACCESSIBLE PARKING STALL CALCULATION

TOTAL PARKING STALL COUNT: ACCESS

POT THAT WERE DETERMINED TO BE NON-COMPLIANT

TOTAL PARKING STALL COUNT.	49 3 IALL3
ACCESSIBLE PARKING STALLS	(TABLE 11B-208.2)
REQUIRED ACCESSIBLE STALLS: REQUIRED VAN ACCESSIBLE STALLS: ACCESSIBLE STALLS PROVIDED:	1 (26-50 TOTAL STALLS) 1 (1-6 ACCESSIBLE STALLS) 1 STANDARD & 1 VAN

	PROPOSED SHADE STRUCTURE									
Đ	UNIT	DESCRIPTION	CONSTRUCTION TYPE	OCCUPANCY	ALLOWABLE AREA					
	SS	SHADE	II-B OR	A-3	9,500 SF MAX					
		STRUCTURE	V-B	A-3	6,000 SF MAX					

ING OF	EXISTING BUILDING DESIGNATIONS										
O AS CTION	UNIT	DESCRIPTION	DSA APPLICATION #	AREA (SF) NOTES							
CTION	А	ADMINISTRATION / MULTIPURPOSE	9067, 23022	9,405							
	В	RELOCATABLE CLASSROOMS	80078	6,413							
	С	RELOCATABLE CLASSROOMS	80078	1,315							
	D	CLASSROOMS	13938	4,742							
	E1-E7	RELOCATABLE CLASSROOMS	53491, 02-102428	960 EACH							
	F1-F5	RELOCATABLE CLASSROOMS	53491, 02-102428	960 EACH							
	G1-G3	RELOCATABLE CLASSROOMS	53491, 02-102428	960 EACH							
	Н	RELOCATABLE CLASSROOMS	80078	5,875							
	J1-J2	RELOCATABLE CLASSROOMS	19861, 48230	960 EACH							
	RR	TOILET ROOMS	80078, THIS APPLICATION	480							

# **LEGEND**

— • • • PROPERTY LINE

---- ASSUMED PROPERTY LINE



UNIT DESIGNATION

EXISTING BUILDINGS

EXPANSION JOINT CONCRETE WALK / PAVING - CONTROL JOINT

ASPHALT CONCRETE PAVING

ACCESSIBLE PATH OF TRAVEL SITE WALKWAYS SHALL PROVIDE A BARRIER-FREE P.O.T. ABRUPT CHANGES IN LEVEL ALONG ANY P.O.T. ARE ALLOWED UP TO 1/2". ONLY ABRUPT CHANGES IN ELEVATION UP TO 1/4" ARE ALLOWED TO HAVE A VERTICAL TRANSITION. ABRUPT CHANGES IN ELEVATION BETWEEN 1/4" AND 1/2" SHALL BE BEVELED WITH A SLOPE NO GREATER THAN 1-UNIT VERTICAL TO 2-UNITS HORIZONTAL. WALKWAYS SHALL BE FREE OF GRATINGS WHEREVER POSSIBLE. GRATING WHICH OCCUR WITHIN THE P.O.T. SHALL HAVE OPENINGS WHICH DO NOT EXCEED 1/2" IN THE DIRECTION OF TRAVEL PER CBC SECTION 11B-302.3. AN ABRUPT DROP-OFF CHANGE IN ELEVATION AT THE EDGE OF ANY WALK INTO AN ADJACENT PLANTER SHALL NOT EXCEED 4". SLOPES IN THE DIRECTION OF THE P.O.T. GREATER THAN 1-UNIT VERTICAL TO 20-UNITS HORIZONTAL SHALL BE CONSIDERED A RAMP AND WILL REQUIRE HANDRAILS ON BOTH SIDES PER CBC SECTION 11B-505 SLOPES IN THE DIRECTION OF THE P.O.T. ALONG WALKWAYS SHALL NOT EXCEED 5%. CROSS SLOPES IN THE P.O.T. ALONG WALKWAYS SHALL NOT EXCEED 2%.

ALL WALKWAYS WITHIN THE P.O.T. SHALL BE A MINIMUM OF 48" IN WIDTH. SURFACES WITH A SLOPE OF 5% OR LESS SHALL BE AT LEAST AS SLIP-RESISTANT AS THAT PROVIDED BY A LIGHT BROOM FINISH. SURFACES WITH A SLOPE OF MORE THAN 5% SHALL BE AT LEAST AS SLIP-RESISTANT AS THAT PROVIDED BY A MEDIUM BROOM

OBJECTS PROTRUDING INTO THE P.O.T. SHALL NOT REDUCE THE CLEAR WIDTH OR MANEUVERING SPACE WITHIN THE P.O.T. PER CBC SECTION 11B-307. PASSING SPACES (11B-403.5.3) OF 60" X 60" MIN. ARE LOCATED NOT MORE THAN 200' APART. WALKS WITH CONTINUOUS GRADIENTS SHALL HAVE 60" IN LENGTH LEVEL RESTING AREAS (11B-403.7) NOT MORE THAN 400' APART. P.O.T. SHALL BE MAINTAINÉD FREE OF OVERHANGING OBSTRUCTIONS TO 80" MIN (11B-307.4) AND FREE OF PROTRUDING OBJECTS (11B-307) GREATER THAN 4" PROJECTION FROM WALL ABOVE 27" AND LESS THAN 80". OBJECTS PROTRUDING INTO THE P.O.T SHALL NOT REDUCE THE CLEAR WIDTH OR MANEUVERING SPACE REQUIRED FOR ACCESSIBLE ROUTES (11B-307.5).

#### **SHEET NOTES**

- SN.01 (E) PARKING LOT ENTRANCE SIGN REVIEWED AND VERIFIED PER THIS APPLICATION. SN.02 ACCESSIBLE PARKING STALLS PER THIS APPLICATION SN.03 (E) ACCESSIBLE STAFF TOILET ROOM UPGRADED PER THIS APPLICATION
- SN.04 (E) ACCESSIBLE GIRL'S TOILET ROOM UPGRADED PER THIS APPLICATION
  SN.05 (E) ACCESSIBLE BOY'S TOILET ROOM UPGRADED

#### PER THIS APPLICATION SN.06 (E) ACCESSIBLE DRINKING FOUNTAIN UPGRADED PER THIS APPLICATION

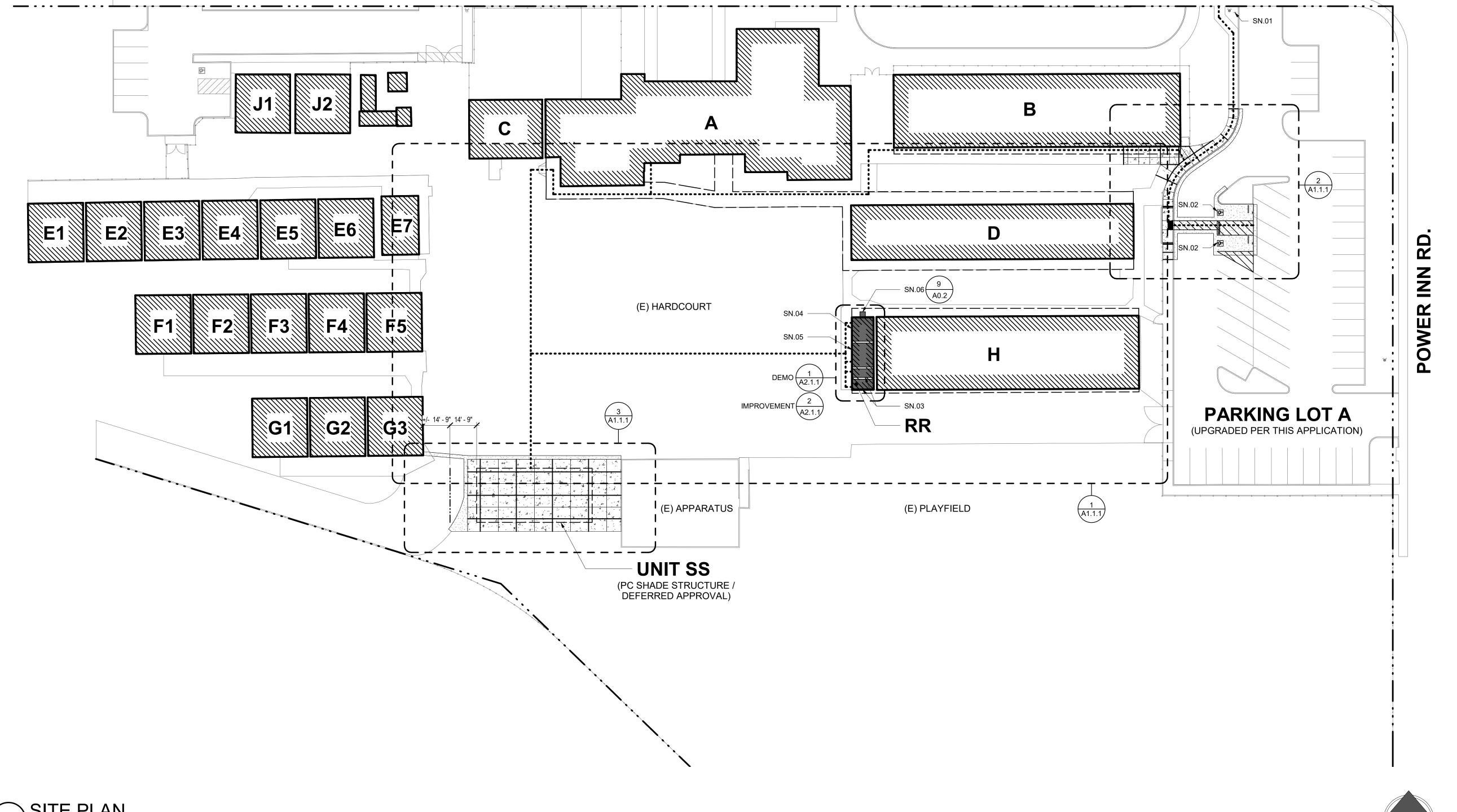
SHADE STRUCTURE ELEMENTARY SCHO

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SITE PLAN AND CODE INFORMATION

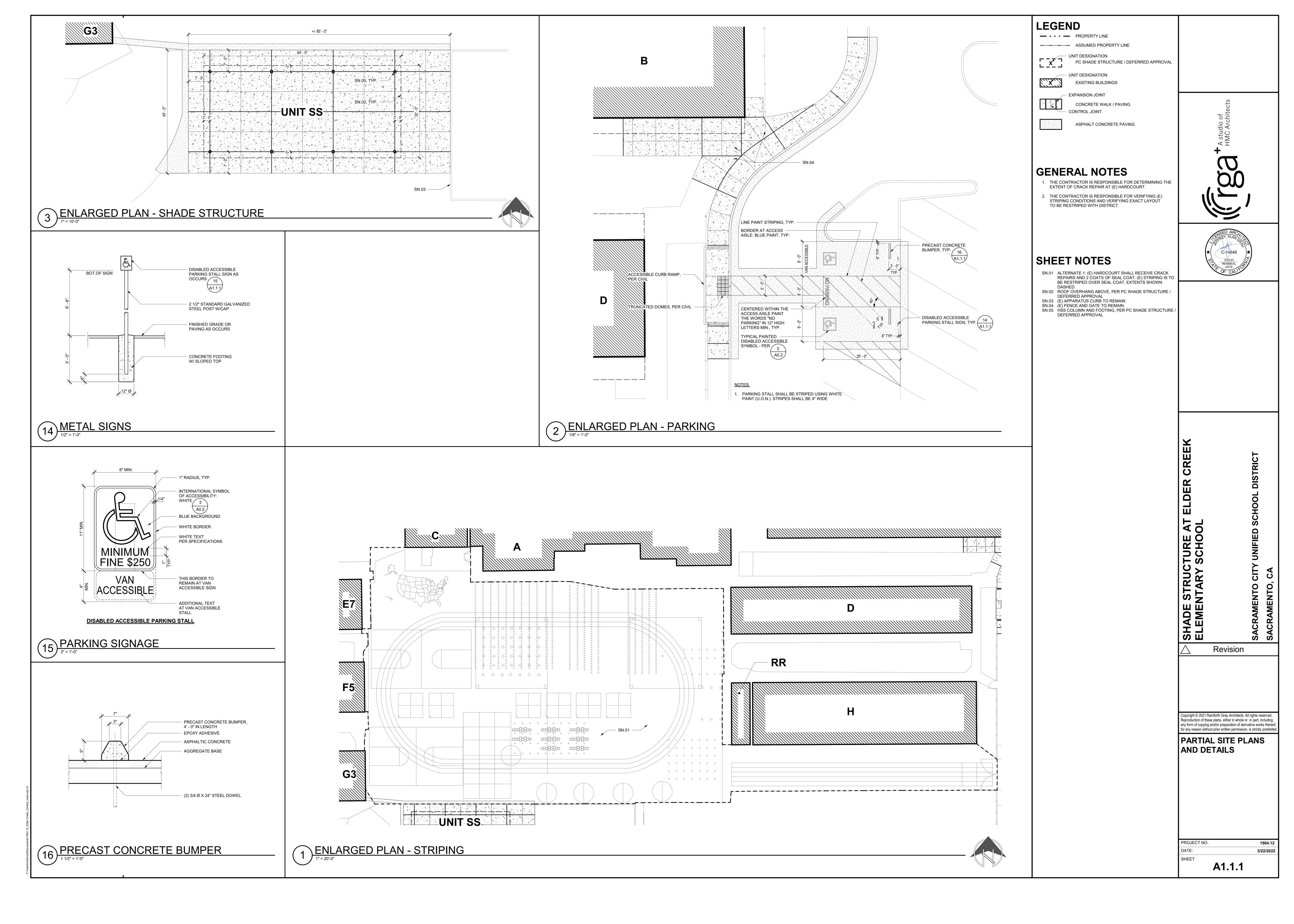
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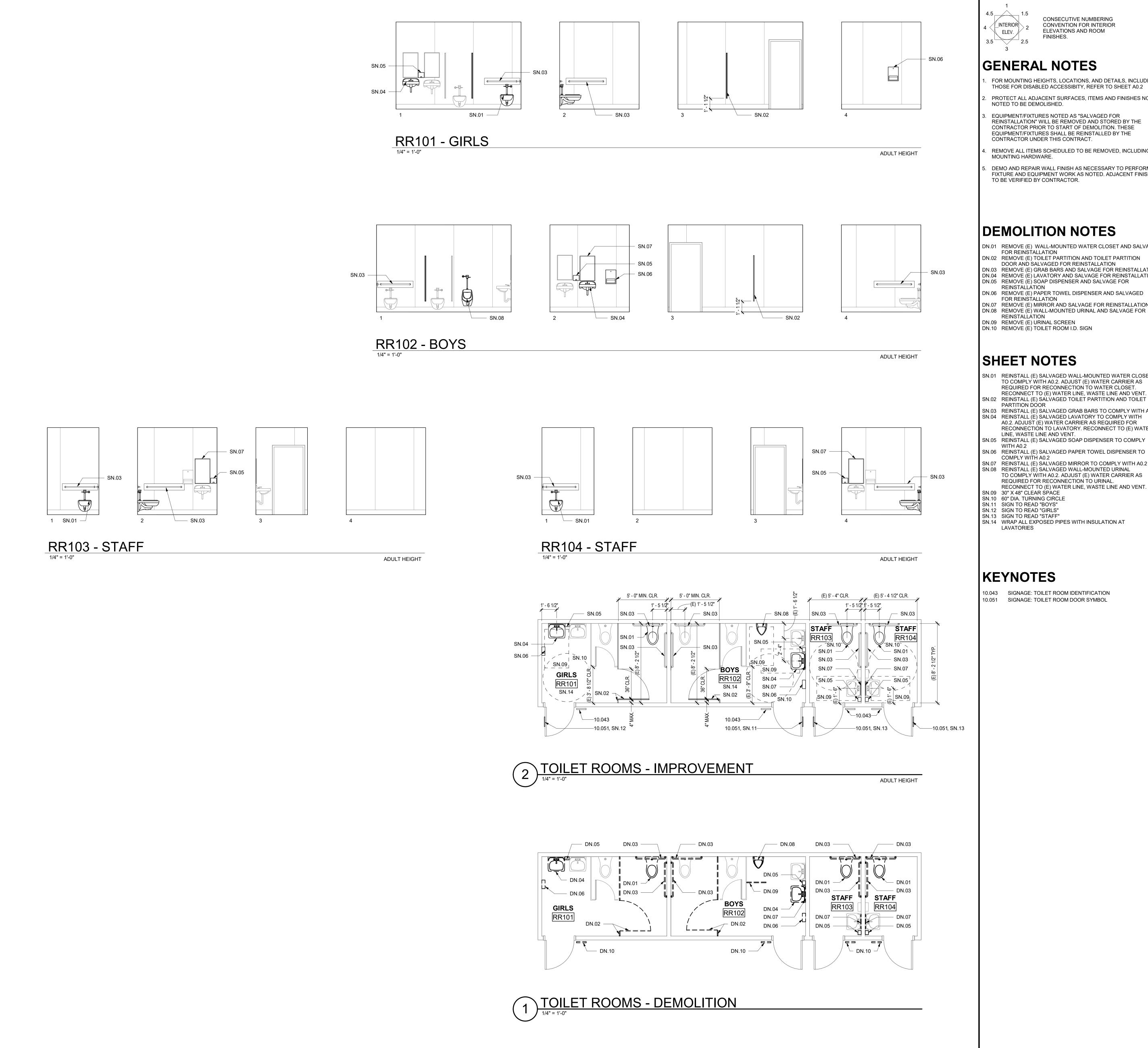


**LEMON HILL AVE.** 

1) SITE PLAN

1" = 30'-0"





#### **LEGEND**

CONSECUTIVE NUMBERING INTERIOR ELEV.

CONVENTION FOR INTERIOR **ELEVATIONS AND ROOM** FINISHES.

# **GENERAL NOTES**

- FOR MOUNTING HEIGHTS, LOCATIONS, AND DETAILS, INCLUDING THOSE FOR DISABLED ACCESSIBITY, REFER TO SHEET A0.2 PROTECT ALL ADJACENT SURFACES, ITEMS AND FINISHES NOT
- EQUIPMENT/FIXTURES NOTED AS "SALVAGED FOR REINSTALLATION" WILL BE REMOVED AND STORED BY THE CONTRACTOR PRIOR TO START OF DEMOLITION. THESE EQUIPMENT/FIXTURES SHALL BE REINSTALLED BY THE
- REMOVE ALL ITEMS SCHEDULED TO BE REMOVED, INCLUDING MOUNTING HARDWARE.
- DEMO AND REPAIR WALL FINISH AS NECESSARY TO PERFORM FIXTURE AND EQUIPMENT WORK AS NOTED. ADJACENT FINISHES TO BE VERIFIED BY CONTRACTOR.

#### **DEMOLITION NOTES**

- DN.01 REMOVE (E) WALL-MOUNTED WATER CLOSET AND SALVAGE FOR REINSTALLATION DN.02 REMOVE (E) TOILET PARTITION AND TOILET PARTITION DOOR AND SALVAGED FOR REINSTALLATION
- DN.03 REMOVE (E) GRAB BARS AND SALVAGE FOR REINSTALLATION DN.04 REMOVE (E) LAVATORY AND SALVAGE FOR REINSTALLATION DN.05 REMOVE (E) SOAP DISPENSER AND SALVAGE FOR REINSTALLATION
- DN.06 REMOVE (E) PAPER TOWEL DISPENSER AND SALVAGED FOR REINSTALLATION
- DN.07 REMOVE (E) MIRROR AND SALVAGE FOR REINSTALLATION DN.08 REMOVE (E) WALL-MOUNTED URINAL AND SALVAGE FOR REINSTALLATION
- DN.09 REMOVE (E) URINAL SCREEN DN.10 REMOVE (E) TOILET ROOM I.D. SIGN

#### SHEET NOTES

- SN.01 REINSTALL (E) SALVAGED WALL-MOUNTED WATER CLOSET TO COMPLY WITH A0.2. ADJUST (E) WATER CARRIER AS REQUIRED FOR RECONNECTION TO WATER CLOSET. RECONNECT TO (E) WATER LINE, WASTE LINE AND VENT.
- SN.02 REINSTALL (E) SALVAGED TOILET PARTITION AND TOILET PARTITION DOOR SN.03 REINSTALL (E) SALVAGED GRAB BARS TO COMPLY WITH A0.2 SN.04 REINSTALL (E) SALVAGED LAVATORY TO COMPLY WITH A0.2. ADJUST (E) WATER CARRIER AS REQUIRED FOR
- RECONNECTION TO LAVATORY. RECONNECT TO (E) WATER LINE, WASTE LINE AND VENT. SN.05 REINSTALL (E) SALVAGED SOAP DISPENSER TO COMPLY WITH A0.2
- COMPLY WITH A0.2 SN.07 REINSTALL (E) SALVAGED MIRROR TO COMPLY WITH A0.2 SN.08 REINSTALL (E) SALVAGED WALL-MOUNTED URINAL TO COMPLY WITH A0.2. ADJUST (E) WATER CARRIER AS REQUIRED FOR RECONNECTION TO URINAL. RECONNECT TO (E) WATER LINE, WASTE LINE AND VENT.
- SN.09 30" X 48" CLEAR SPACE SN.10 60" DIA. TURNING CIRCLE SN.11 SIGN TO READ "BOYS"
- SN.12 SIGN TO READ "GIRLS" SN.13 SIGN TO READ "STAFF"
  SN.14 WRAP ALL EXPOSED PIPES WITH INSULATION AT LAVATORIES

# KEYNOTES

10.043 SIGNAGE: TOILET ROOM IDENTIFICATION 10.051 SIGNAGE: TOILET ROOM DOOR SYMBOL

TURE SHADE STRUCT ELEMENTARY S

ELDER

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TOILET ROOM **DEMOLITION AND** IMPROVEMENT PLANS AND INTERIOR ELEVATIONS

UNIT RR A2.1.1

#### ABBREVIATION LIST AMPERE ALTERNATING CURRENT AIR CONDITIONING ARC ENERGY REDUCTION AMP FRAME ABOVE FINISHED FLOOR AMPERES INTERRUPTING CAPACITY AMP TRIP SETTING AMERICAN WIRE GAUGE BARE COPPER BELOW FINISHED CEILING BREAKER BUILDING **BOOSTER POWER SUPPLY** CONDUIT CIRCUIT BREAKER CONTRACTOR FURNISHED. CONTRACTOR INSTALLED CIRCUIT CEILING CONDUIT ONLY, WITH PULL LINE CONT CONTINUOUS METALLIC COLD WATER PIPE DEMOLISH DIRECT CURRENT DISCONNECT DISTRIBUTION PANEL EXISTING EACH WITH **EVENING LIGHT** ELECTRIC EMERGENCY ELECTRICAL METALLIC TUBING END OF LINE DEVICE **EQUIPMENT** EXISTING RELOCATED ELECTRICAL WATER COOLER ELECTRIC WATER HEATER FIRE ALARM CONTROL PANEL FAEP FIRE ALARM EXTENDER PANEL FATC FIRE ALARM TERMINAL CABINET FURNISHED BY OTHERS **FLUOR** FLUORESCENT GROUND FAULT CIRCUIT INTERRUPT GENERAL LIGHTING ZONE METALLIC GAS PIPE GYPSUM HIGH INTENSITY DISCHARGE HORSE POWER HEIGHT HERTZ INTERMEDIATE METALLIC CONDUIT SHORT CIRCUIT CURRENT (RMS SYMMETRICAL) ISOLATED J-B0X JUNCTION BOX THOUSAND CIRCULAR MILLS KCMIL KILO VOLT AMP KILOWATT LIGHTING CONTROL PANEL LOW VOLTAGE THOUSAND CIRCULAR MILLS MECHANICAL MAIN DISTRIBUTION PANEL METAL HALIDE MISCELLANEOUS MAIN LUGS ONLY MAIN POINT OF ENTRY MAIN SWITCHBOARD NOT IN CONTRACT NOT IN ELECTRICAL SECTION OF THESE PLANS & SPECS. NIGHT LIGHT NUMBER NOT TO SCALE ON CENTER OFCI OWNER FURNISHED, CONTRTRACTOR INSTALLED OFOI OWNER FURNISHED, OWNER INSTALLED PULL BOX PROVISION FOR FUTURE BREAKER W/ PFB MOUNTING HARDWARE PRIMARY DAYLIT ZONE PROVISION FOR FUTURE CURRENT TRANSFORMER PHASE PLYWOOD PLYWD PANEL PNLPAIR POLYVINYL CHLORIDE CONDUIT RELOCATE / RELOCATED (R) REQ'D REQUIRED ROOM RIGID METAL CONDUIT REMOVE AND REPLACE SECONDARY DAYLIT ZONE SKYLIGHT DAYLIT ZONE SPEC SPECIFICATION SIGNAL TERMINAL CABINET SQUARE SWITCH TELEPHONE TELECOMMUNICATIONS GROUNDING TELECOMMUNICATIONS MAIN GROUNDING BUSBAR TELEPHONE TERMINAL BOARD TYPICAL UNDERGROUND UNLESS OTHERWISE NOTED UON VOLTS WEATHERPROOF WEIGHT WATT TRANSFORMER

#### **GENERAL NOTES**

- 1. PLANS ARE NOT FOR CONSTRUCTION UNTIL APPROVED BY THE AUTHORITY HAVING JURISDICTION. THE CONTRACTOR SHALL NOT ORDER ANY MATERIALS OR INSTALL ANY EQUIPMENT, PIPING, ETC. UNTIL PLANS ARE APPROVED BY THE AUTHORITY HAVING JURISDICTION.
- 2. ALL WORK SHALL BE DONE AT SUCH TIME AND IN SUCH MANNER AS PRESCRIBED BY THE SCHOOL'S REPRESENTATIVE.
- 3. PROTECT EXISTING EQUIPMENT AND FURNISHINGS FROM ANY DAMAGE DUE TO DUST, MOISTURE OR CONTACT WITH WORK CREW OR MATERIALS.
- 4. THE SCHOOL SHALL BE NOTIFIED AT LEAST FORTY-EIGHT (48) HOURS IN ADVANCE OF ANY POWER SHUTDOWN OF EXISTING PANELS OR SERVICE. SCHEDULE OF SHUTDOWNS SHALL BE AT CONVENIENCE OF THE SCHOOL. THE SCHOOL MAY, AT THEIR OPTION, HAVE A REPRESENTATIVE PRESENT DURING SHUTDOWN. ALL WORK REQUIRING SHUTDOWNS OF EXISTING PANELS OR SERVICE SHALL BE DONE BETWEEN 12:00 AM MIDNIGHT AND 6:00AM WEEKDAYS OR ON SATURDAY AND SUNDAY. REQUIRED SHUTDOWNS SHALL BE KEPT TO A MINIMUM.
- 5. ADEQUATELY STRAP AND SUPPORT ALL CONDUIT WORK PER CEC. IN GENERAL, SUPPORT ALL CONDUIT WITHIN THREE FEET (3') OF OUTLET BOX, CABINET OR PANEL AND MAXIMUM TEN FEET (10') ON CENTER THEREAFTER.
- 6. CORE BORE SHALL BE 1" DIAMETER LARGER THAN EACH CONDUIT. SPACE CONDUIT HOLES 3" APART. SEAL AROUND CONDUIT WITH NON-SHRINK, NON-METALLIC GROUT.
- 7. ALL CONDUCTORS INSTALLED IN PANELBOARDS SHALL BE TRAINED, LACED, AND INSTALLED WITH PHASE TAPE ON ALL CONDUCTORS.
- 8. LABEL DEVICES (I.E. RECEPTACLES, ETC.) ON EACH COVER PLATE IDENTIFYING CIRCUIT AND PANEL DEVICE IS CONNECTED TO.
- 9. CLEAN ALL EXTERIOR AND INTERIOR SURFACES OF PANELS AND ALL MATERIAL AND METAL SHAVINGS FROM PANEL AND CABINET INTERIORS. ALL OPENINGS SHALL BE SEALED AND APPLY TOUCH-UP SPRAY PAINT WHERE NEEDED.
- 10. FIELD COORDINATE DEVICE LOCATIONS PRIOR TO ROUGH-IN.
- 11. CONTRACTOR WILL PROVIDE WARNING LABELS NOTING THE POTENTIAL FOR ELECTRIC ARC FLASH HAZARDS PER CEC 110.16. PROVIDE LABELS ON EQUIPMENT SUCH AS SWITCHBOARDS, SWITCHGEAR, PANELBOARDS, INDUSTRIAL CONTROL PANELS, METER SOCKET ENCLOSURES, MOTOR CONTROL CENTERS, MOTOR STARTER / CONTACTOR PANELS, DISCONNECTS, ETC.. PROVIDE WARNING LABELS BY BRADY, MODEL NO. 101517, OR EQUAL, ON ALL
- 12. INSTALLATION SHALL COMPLY WITH CEC 210.4 EACH MULTIWIRE BRANCH CIRCUIT SHALL BE PROVIDED WITH A MEANS THAT WILL SIMULTANEOUSLY DISCONNECT ALL UNGROUNDED CONDUCTORS AT THE POINT WHERE THE BRANCH CIRCUIT ORIGINATES. THEREFORE ANY CIRCUIT SHARING A COMMON NEUTRAL SHALL BE CAPABLE OF SIMULTANEOUS DISCONNECT OR DEDICATED NEUTRALS SHALL BE INSTALLED.
- 13. SUPPORT ENCLOSURES, BOXES AND CONDUIT INSTALLATIONS PER CEC 314.23 (A) THROUGH (H).
- 14. SEAL CONDUIT OPENINGS THROUGH WALLS AND CEILINGS. INSTALL ESCUTCHEON PLATES AT BUILDING INTERIOR. WHERE EQUIPMENT IS INSTALLED ON THE EXTERIOR WALL, STUB CONDUITS THROUGH WALL AND SEAL CONDUIT OPENINGS, THEN INSTALL EXTERIOR EQUIPMENT. ALSO, SEAL AROUND THE PERIMETER EDGE OF THE EQUIPMENT ENCLOSURE BETWEEN THE ENCLOSURE AND BUILDING.
- 15. CONDUITS INSTALLED ON ROOF AND BUILDING EXTERIOR SHALL BE RIGID GALV. STEEL (HEAVY WALL) WITH THREADED FITTINGS. CONDUIT AND WALL TO BE PAINTED OUT TO MATCH EXTERIOR FINISH.
- 16. SPLICES AND TERMINALS SHALL BE COMPRESSION TYPE OF SEAMLESS PURE COPPER, TIN PLATED, LONG BARREL (TERMINALS WITH TWO-HOLE PAD AND INSPECTION WINDOW WITH NEMA DRILLING), AS MANUFACTURED BY BURNDY TYPE YS, YAZ-2N OR EQUAL. CLEAN ALL SURFACES AND INSTALL WITH OXIDE INHIBITING COMPOUND, BURNDY PENETROX-E OR EQUAL. APPLY COMPOUND BETWEEN BUS AND LUG PAD AND BETWEEN CONDUCTOR AND LUG BARREL. INSTALL COMPRESSION CONNECTORS WITH 360° CIRCUMFERENTIAL COMPRESSION DYE, BURNDY HYPRESS OR EQUAL. THE INDENTER OR OTHER TYPE TOOLS WILL NOT BE ACCEPTABLE.
- 17. INSTALL 'MECHANICALLY FASTENED PHENOLIC NAMEPLATE WITH WHITE LETTERING ON BLACK BACKGROUND ON ALL EQUIPMENT, INCLUDING PULL BOXES, WITH DESCRIPTION INDICATED ON DRAWINGS. NAMEPLATES SHALL READ EXACTLY AS DESCRIBED ON THE DRAWINGS. IN GENERAL NAMEPLATE LETTERING SIZE SHALL BE 3/16" HIGH FOR ALL NAMEPLATES SERVING FEEDER AND BRANCH CIRCUIT BREAKERS. ON MAIN SERVICE PANEL. DISTRIBUTION PANELS AND ALL OTHER NAMEPLATES LETTERING SHALL BE 1/4" HIGH.
- 17.1. ALL SWITCHBOARDS, SWITCHGEAR, PANELBOARDS, VFD'S, MOTORS, JUNCTION BOXES, PULL BOXES, DISCONNECT SWITCHES, ETC., SHALL BE MARKED TO INDICATE EACH DEVICE OR EQUIPMENT WHERE THE POWER ORIGINATES PER CEC 408.4, FIELD IDENTIFICATION REQUIRED, (B) SOURCE OF SUPPLY.
- 18. COORDINATE EQUIPMENT LOCATIONS, CONTROL AND POWER WIRING REQUIREMENTS AND CONNECT POINTS WITH ALL APPLICABLE DISCIPLINES.
- 19. PROVIDE AND INSTALL FUSES PER UNIT NAMEPLATE DATA ON THE EQUIPMENT PROVIDED.
- 20. A LAMINATED COPY OF THE FINAL RECORD ONE LINE DIAGRAM SHALL BE PLACED IN ELEC ROOM.
- 21. PROVIDE WIRING DEVICES AND COVER PLATES IN COLOR(S) SELECTED BY ARCHITECT. THE COLOR OF THE WIRING DEVICE AND COVER PLATE SHALL BE THE SAME UNLESS SPECIFICALLY NOTED OTHERWISE.
- 22. RECEPTACLE WEATHERPROOF COVERS SHALL BE LISTED "EXTRA DUTY", LOCAKBLE, METAL, IN-USE TYPE.
- 23. REINSTALL EXISTING ELECTRICAL INSTALLATIONS DISTURBED. CERTAIN EXISTING ELECTRICAL INSTALLATIONS MAY BE LOCATED IN WALLS. CEILINGS OR FLOORS THAT ARE TO BE REMOVED AND ARE ESSENTIAL FOR THE OPERATION OF OTHER REMAINING INSTALLATIONS. WHERE THIS CONDITIONS OCCURS, PROVIDE A NEW EXTENSION OF ORIGINAL CIRCUITS, RACEWAYS, EQUIPMENT AND OUTLETS TO RETAIN SERVICE CONTINUITY. INSTALLATIONS SHALL BE CONCEALED IN FINISHED AREAS.
- 24. FOR ROOF PENETRATIONS, REFER TO ARCHITECTURAL PLANS FOR INSTALLATION REQUIREMENTS.
- 25. FOR WALL PENETRATION INSTALLATIONS, REFER TO ARCHITECTURAL PLANS FOR REQUIREMENTS.
- 26. PROVIDE "LOCK-ON" DEVICE FOR ALL CIRCUIT BREAKERS ON EMERGENCY DEDICATED CIRCUITS.
- 27. DRAWINGS ARE TO BE CONSIDERED DIAGRAMMATIC. CONTRACTOR SHALL ACCEPT RESPONSIBILITY IN FAMILIARIZING THEMSELVES WITH ARCHITECTURAL AND STRUCTURAL CONDITIONS ALONG WITH INHERENT SPACE LIMITATIONS. WITH THAT UNDERSTANDING SHALL PROVIDE ALL ITEMS OF LABOR, MATERIALS AND TOOLS REQUIRED TO PROVIDE A COMPLETE INSTALLATION.
- 28. MAINTAIN A MINIMUM OF 12" SEPARATION BETWEEN ANY CONDUIT AND (E) UTILITY CONDUIT.
- 29. FOR INTERSECTING TRENCHED CONDUIT, MAINTAIN OR EXCEED THE MINIMUM CONDUIT DEPTH REQUIREMENTS.

#### MEP COMPONENT ANCHORAGE NOTE

ALL MECHANICAL, PLUMBING AND ELECTRICAL COMPONENTS SHALL BE ANCHORED AND INSTALLED PER THE DETAILS ON THE DSA APPROVED CONSTRUCTION DOCUMENTS. THE FOLLOWING COMPONENTS SHALL BE ANCHORED AND BRACED TO MEET THE FORCE AND DISPLACEMENT REQUIREMENTS PRESCRIBED IN THE 2019 CBC SECTIONS 1617A.1.18 THROUGH 1617A.1.26 AND ASCE 7-16 CHAPTERS 13, 26 AND 30:

- ALL PERMANENT EQUIPMENT AND COMPONENTS. TEMPORARY, MOVEABLE OR MOBILE EQUIPMENT THAT IS PERMANENTLY ATTACHED (E.G. HARD WIRED) TO THE BUILDING UTILITY SERVICES SUCH AS ELECTRICITY, GAS OR WATER. "PERMANENTLY ATTACHED" SHALL INCLUDE ALL ELECTRICAL CONNECTIONS EXCEPT PLUGS FOR 110/20 VOLT RECEPTACLES HAVING A FLEXIBLE CABLE.
- 3. TEMPORARY, MOVEABLE OR MOBILE EQUIPMENT WHICH IS HEAVIER THAN 400 POUNDS OR HAS A CENTER OF MASS LOCATED 4 FEET OR MORE ABOVE THE ADJACENT FLOOR OR ROOF LEVEL THAT DIRECTLY SUPPORTS THE COMPONENT IS REQUIRED TO BE RESTRAINED IN A MANNER APPROVED BY DSA.
- THE FOLLOWING MECHANICAL AND ELECTRICAL COMPONENTS SHALL BE POSITIVELY ATTACHED TO THE STRUCTURE, BUT NEED NOT DEMONSTRATE DESIGN COMPLIANCE WITH THE REFERENCES NOTED ABOVE. THESE COMPONENTS SHALL HAVE FLEXIBLE CONNECTIONS PROVIDED BETWEEN THE COMPONENT AND ASSOCIATED DUCTWORK, PIPING, AND CONDUIT. FLEXIBLE CONNECTIONS MUST ALLOW MOVEMENT IN BOTH TRANSVERSE AND LONGITUDINAL DIRECTIONS:
- A. COMPONENTS WEIGHING LESS THAN 400 POUNDS AND HAVING A CENTER OF MASS LOCATED 4 FEET OR LESS ABOVE THE ADJACENT FLOOR OR ROOF LEVEL THAT DIRECTLY SUPPORTS THE COMPONENT.
- B. COMPONENTS WEIGHING LESS THAN 20 POUNDS, OR IN THE CASE OF DISTRIBUTED SYSTEMS, LESS THAN 5 POUNDS PER FOOT, WHICH ARE SUSPENDED FROM A ROOF OR FLOOR OR HUNG FROM A WALL.

THE ANCHORAGE OF ALL MECHANICAL, ELECTRICAL AND PLUMBING COMPONENTS SHALL BE SUBJECT TO THE APPROVAL OF THE DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE CHARGE OR STRUCTURAL ENGINEER DELEGATED RESPONSIBILITY AND ACCEPTANCE BY DSA. THE PROJECT INSPECTOR WILL VERIFY THAT ALL COMPONENTS AND EQUIPMENT HAVE BEEN ANCHORED IN ACCORDANCE WITH THE ABOVE REQUIREMENTS.

#### PIPING, DUCTWORK, AND ELECTRICAL DISTRIBUTION SYSTEM BRACING NOTE

PIPING, DUCTWORK, AND ELECTRICAL DISTRIBUTION SYSTEMS SHALL BE BRACED TO COMPLY WITH THE FORCES AND DISPLACEMENTS PRESCRIBED IN ASCE 7-16 SECTION 13.3 AS DEFINED IN ASCE 7-16 SECTIONS 13.6.5, 13.6.6, 13.6.7, 13.6.8 AND 2019 CBC, SECTIONS 1617A.1.24, 1617A.1.25 AND 1617A.1.26.

THE METHOD OF SHOWING BRACING AND ATTACHMENTS TO THE STRUCTURE FOR THE IDENTIFIED DISTRIBUTION SYSTEM ARE AS NOTED BELOW. WHEN BRACING AND ATTACHMENTS ARE BASED ON A PREAPPROVED INSTALLATION GUIDE (E.G., OSHPD OPM FOR 2013 CBC OR LATER), COPIES OF THE BRACING SYSTEM INSTALLATION GUIDE OR MANUAL SHALL BE AVAILABLE ON THE JOBSITE PRIOR TO THE START OF AND DURING THE HANGING AND BRACING OF THE DISTRIBUTION SYSTEMS. THE STRUCTURAL ENGINEER OF RECORD SHALL VERIFY THE ADEQUACY OF THE STRUCTURE TO SUPPORT THE HANGER AND BRACE LOADS.

MECHANICAL PIPING (MP), MECHANICAL DUCTS (MD), PLUMBING PIPING (PP), ELECTRICAL DISTRIBUTION SYSTEMS (E): MP ☐ MD ☐ PP ☐ E ■ OPTION 1: DETAILED ON THE APPROVED DRAWINGS WITH PROJECT SPECIFIC NOTES AND DETAILS.

MP ☐ MD ☐ PP ☐ E ☐ OPTION 2: SHALL COMPLY WITH THE APPLICABLE OSHPD PRE-APPROVAL (OPM #)

#### SYMBOLS LIST

- F' FUSED DISCONNECT SWITCH
- ➡ DUPLEX CONVENIENCE OUTLET
- DOUBLE DUPLEX CONVENIENCE OUTLET GROUND FAULT CIRCUIT INTERRUPTER DUPLEX OUTLET
- GROUND FAULT CIRCUIT INTERRUPTER DOUBLE DUPLEX OUTLET
- SPECIAL OUTLET TO MATCH CAP PROVIDED WITH MACHINE
- FLUSH FLOOR BOX OR "POKE-THRU" UNIT EQUIPPED WITH FLUSH OR PEDESTAL DUPLEX RECEPTACLE AND VOICE/DATA OUTLETS
- AS NOTED, OR REFER TO SCHEDULE ON DRAWINGS.
- PLUGMOLD/WIREMOLD RECEPTACLE SYSTEM △ TRANSFORMER
- JUNCTION BOX, SIZE AS REQUIRED BY CODE
- FLEX CONNECTION TO FIXTURE
- PANELBOARD, RECESSED MOUNTED
- PANELBOARD, SURFACE MOUNTED
- MAIN SWITCHBOARD
- TERMINAL CABINET, RECESSED MOUNTED ☐ TERMINAL CABINET, SURFACE MOUNTED
- → HOMERUN TO PANELBOARD OR RESPECTIVE TERMINAL
- III CONDUIT RUN CONCEALED IN CEILING OR WALL, SEE SYMBOLS LIST NOTES ---- CONDUIT RUN UNDERGROUND OR UNDER FLOOR
- —EM— EMERGENCY SYSTEM CONDUIT AND WIRES
- INSULATED GREEN GROUND CONDUCTOR
- -----O CONDUIT RISER — - — EXISTING EQUIPMENT, LIGHTING, DEVICES, CONDUIT, WIRING, ETC., ARE SHOWN
- ETC., ARE SHOWN DARK. X X EXISTING ELECTRICAL EQUIPMENT TO BE REMOVED

LIGHT. NEW OR RELOCATED EQUIPMENT, LIGHTING, DEVICES, CONDUIT, WIRING,

- WIREMOLD SURFACE RACEWAY(S) WITH OUTLETS AS SHOWN OR NOTED,
- (1) 1> SYMBOLS REFERRING TO KEYED NOTES ON SAME SHEET

SEE SURFACE RACEWAY SCHEDULE

- MECHANICAL EQUIPMENT BY OTHERS, CONNECTED BY ELECTRICAL CONTRACTOR DETAIL DESIGNATION, "A" SIGNIFIES DETAIL, "E-1" SIGNIFIES SHEET NUMBER
- (1)1-1/2"C  $\leftarrow$  INDICATES SIZE OF CONDUIT = ONE AND ONE HALF INCH CONDUIT — NUMBER WITHIN PARENTHESIS INDICATES QUANTITY OF CONDUITS

#### SYMBOLS LIST NOTES:

- 1. MOUNT SWITCH BOXES AT +48" TO TOP OF BOX UNLESS OTHERWISE NOTED.
- 2. MOUNT OUTLET BOXES AT +15" TO BOTTOM OF BOX UNLESS OTHERWISE NOTED.
- 3. "A" ADJACENT TO OUTLET INDICATES OUTLET BOX TO BE MOUNTED ABOVE COUNTER. COORDINATE WITH COUNTER HEIGHT AND DEPTH PRIOR TO ROUGH IN. MOUNT OUTLET ABOVE COUNTERS AT:
- 3.1. +46" MAX TO TOP OF BOX WHERE BOX IS INSTALLED OVER BASE CABINET. 3.2. +44" MAX TO TOP OF BOX WITH OPEN COUNTERS WITH FORWARD APPROACH.
- 4. OUTLET BOXES SHALL BE: 4.1. WALL MOUNTED -4" SQ.  $\times 2-1/8$ " DEEP MINIMUM
- 4.2. CEILING MOUNTED -4" SQ. OR 4" OCT.  $\times 2-1/8$ " DEEP MINIMUM
- 3-1/4" DEEP MINIMUM. 6. FLUSH MOUNTED OUTLET BOXES SHALL UTILIZE TRIM RINGS. COORDINATE TRIM RING

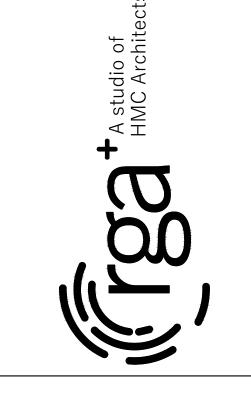
5. OUTLET BOXES REQUIRING 1-1/4", 1-1/2" OR 2" CONDUITS SHALL BE 4-11/16" x

- DEPTH WITH WALL FINISH PRIOR TO ROUGH-IN.
- 7. NO CROSSBARS ON CONDUIT RUN INDICATES MINIMUM 1" CONDUIT, TWO #10 CU CONDUCTORS PLUS 1#10 CU GND. CROSSBARS INDICATE NUMBER OF #10 CU CONDUCTORS IN CONDUIT. CONDUCTOR SIZES OTHER THAN #10 NOTED ON DRAWINGS. INCREASE CONDUIT SIZE AS REQUIRED TO ACCOMMODATE C.E.C. WIRE FILL REQUIREMENTS. INCLUDE ADDITIONAL BOND WIRE IN ALL PVC AND FLEXIBLE CONDUIT. LONG CROSSBAR INDICATES NEUTRAL CONDUCTOR, SHORT CROSSBARS INDICATE PHASE CONDUCTORS.
- 8. INCREASE BRANCH CIRCUIT CU CONDUCTOR SIZES AS REQUIRED BY THE 120V BRANCH CIRCUIT VOLT DROP CONDUCTOR LENGTH CHART BELOW. USE CONDUCTOR LENGTHS AS FIELD MEASURED, BASED UPON MEASURED FIELD ROUTING LENGTHS. INCREASE MINIMUM CONDUIT SIZE AS REQUIRED TO ACCOMMODATE A MAXIMUM 40% CONDUCTOR FILL OF THE BRANCH CIRCUIT CONDUCTORS. WHERE NECESSARY, PROVIDE A JUNCTION BOX AT ACCESSIBLE CEILING SPACE TO CONVERT THE LAST 15 FEET OF CONDUCTORS TO #10 AWG TO ACCOMMODATE TERMINATION OF CONDUCTORS AT WIRING DEVICES, LIGHTING FIXTURES, CIRCUIT BREAKER, ETC.
- 9. INSTALL CU GROUND CONDUCTOR IN ALL BRANCH CIRCUITS FOR LIGHT FIXTURES AND POWER DEVICES.

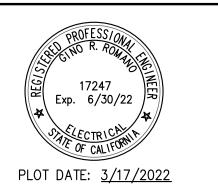
#### 120V BRANCH CIRCUIT VOLT DROP CONDUCTOR LENGTH CHART

LOAD IN	LENGTH OF CONDUCTOR									
VOLT	WIRE SIZE IN (GAUGE)									
AMPERES	#12	#10	#8	#6	#4					
1200VA	74	121	183	284	434					
1560VA	57	93	141	218	334					
1800VA	49	81	122	189	289					
1920VA	46	76	115	178	271					
2340VA	Х	62	94	146	223					
2880VA	Х	51	76	118	181					
3000VA	Х	48	73	114	174					
3900VA	Х	Χ	56	87	134					
4800VA	Χ	Χ	46	71	108					

- 1. THIS CHART IS FOR COPPER CONDUCTORS ONLY. THIS CHART ASSUMES AN 80% POWER FACTOR AND STEEL RACEWAYS. 3. 2019 CALIFORNIA ENERGY CODE, 130.5(c) ALLOWS A MAXIMUM COMBINED VOLTAGE DROP OF 5%. THIS CHART ASSUMES A MAXIMUM DROP OF 3% FOR FEEDERS. THIS CHART PROVIDES
- THE MAXIMUM LENGTH OF CONDUCTORS FOR LESS THAN 2% VOLTAGE DROP ON A BRANCH CIRCUIT AT GIVEN VA LOAD. 4. USE WIRE SIZE FROM THIS CHART UNLESS LARGER CONDUCTOR SIZES ARE NOTED ON THE
- 5. FOR VA VALUES NOT SHOWN USE NEXT HIGHEST VALUE FROM THE CHART







R H

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S E

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S ED 0

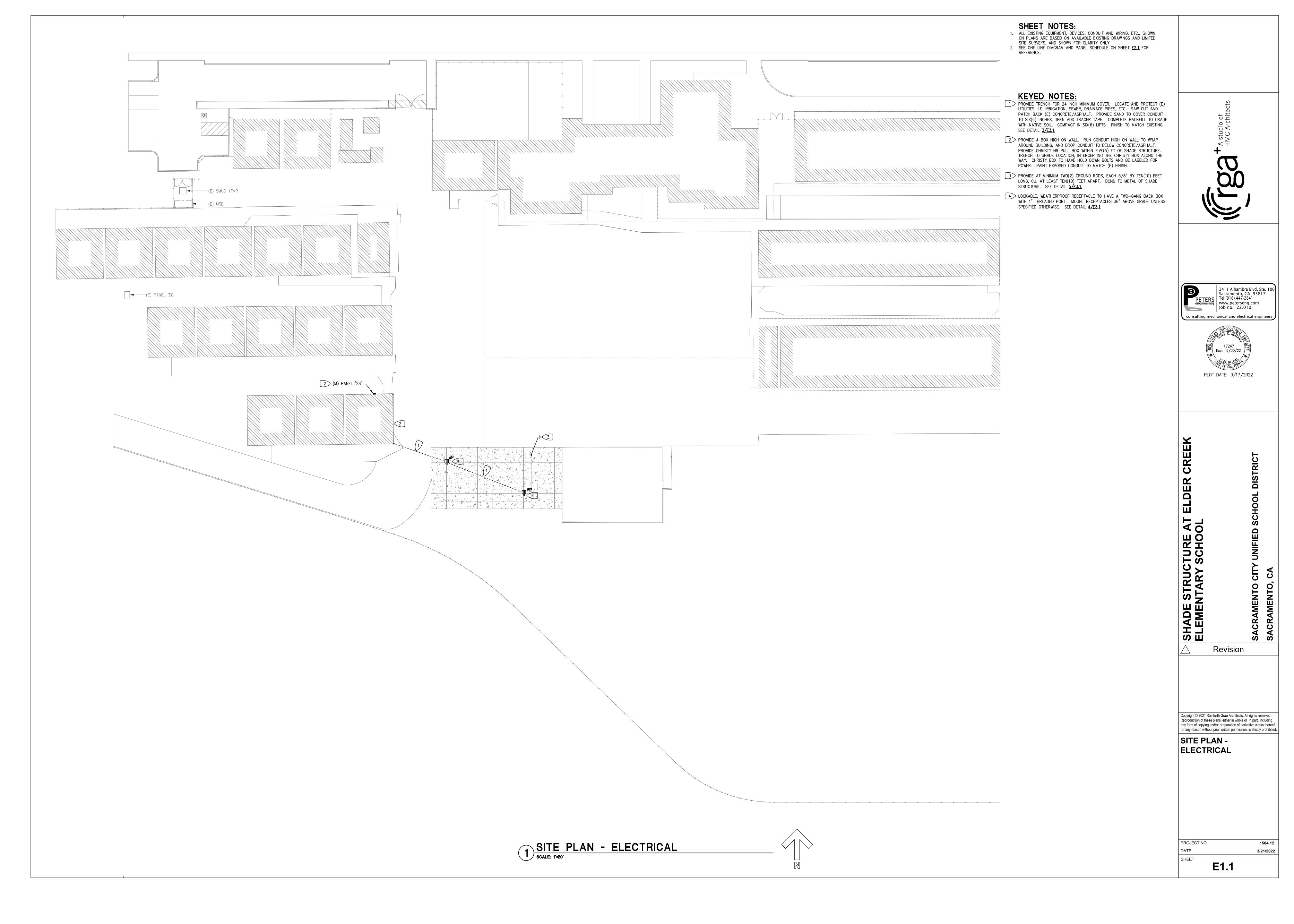
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Revision

SYMBOLS, NOTES

E0.1



PANEL:			: 100/2			VICE:		MOUNT		ENCLOSURE:	10K	AIC
28	TYPE:	LOAD CENTER BUSS	: 100	AMP			VOL	Γ	SURFACE	WIDTH: 14"	100%	NEU.
20		FEEDER RATING	: 125	AMP		Ø, 3\				DEPTH: 4.25"		
AØ	ВØ	DIRECTORY		BRKR	CKT		CKT	BRKR	[	DIRECTORY	AØ	В
		MAIN			1	•	2		DO NOT REM	MOVE THIS K.O.		
		lu lu		-	3	•	4		"			
1000		PHOTO CELLS		20/1	5	•	6	20/1	RECEPTS		1200	
		A LIGHT			7	•	8	20/1	RECEPTS			120
		B LIGHT		20/1	9	•	10	20/1	RECEPTS		1200	
	360	RECEPTS - SHADE STRUCT. [5]			11	•	12	PFB	SPACE	ACE,		
		SPACE		PFB	13	•	14	60/2	H1A		4160	
		SPACE		PFB	15	•	16	-	"			416
		NEW LOAD		DEMAN	DEMAND READ			PEAK	DEMAND @	125% + (N) LOAD	TOTAL	DEMA
		TOTAL PANEL VA	AMPS	AMPS	@1	25%		ΑN	IPS	VA	LC	DAD
	AØ =	7560 VA	63.0	9.5		11.9		74.9	Α	8985 VA	15275	5 VA
	BØ =	5720 VA	47.7	3.8		4.8	52.4		Α	6290 VA	74.9	9 AMP

	Voltage Drop Calculations Copper											
ob Name: Elder Creek Elementary School - Shade Structure Job #: 22.02										22.020		
Date:	Date: 3/10/2022											
VOLTAGE: 120 PHASE: 1 POWER FACTOR: 80% CONDUIT: Steel										teel		
FEEDER	AMPS AT	KVA	VOLTS	DISTANCE	DISTANCE	WIRES/	LOAD/	WIRE	WIRE	VOLTS	PERCENT	
NUMBER	LOAD	TOTAL	AT LOAD	FEET	TOTAL	PHASE	WIRE	SIZE	FACTOR	DROP	VOLT DROP	
ECEPT-1	3.0	0.4	119.29	119	119	1	3.00	10	1995	0.71	0.59%	
ECEPT-2	1.5	0.2	119.10	62	181	1	1.50	10	1995	0.90	0.75%	

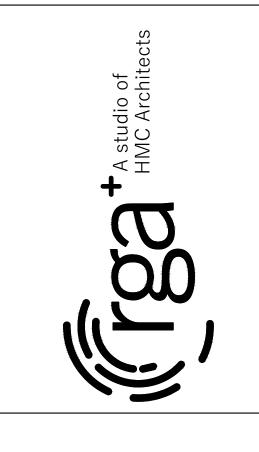
2. BRANCH BREAKERS ARE WESTINGHOUSE TYPE BR
3. PROVIDE TYPE-WRITTEN PANEL DIRECTORY
4. ALL NEW BREAKERS TO MATCH EXISTING TYPES
5. PROVIDE NEW 20 AMP, SINGLE-POLE BREAKER.

SHEET NOTES:

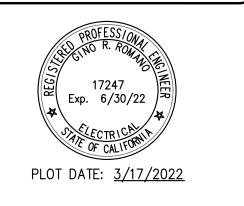
1. ALL EXISTING EQUIPMENT, DEVICES, CONDUIT AND WIRING, ETC., SHOWN ON PLANS ARE BASED ON AVAILABLE EXISTING DRAWINGS AND LIMITED SITE SURVEYS, AND SHOWN FOR CLARITY ONLY.

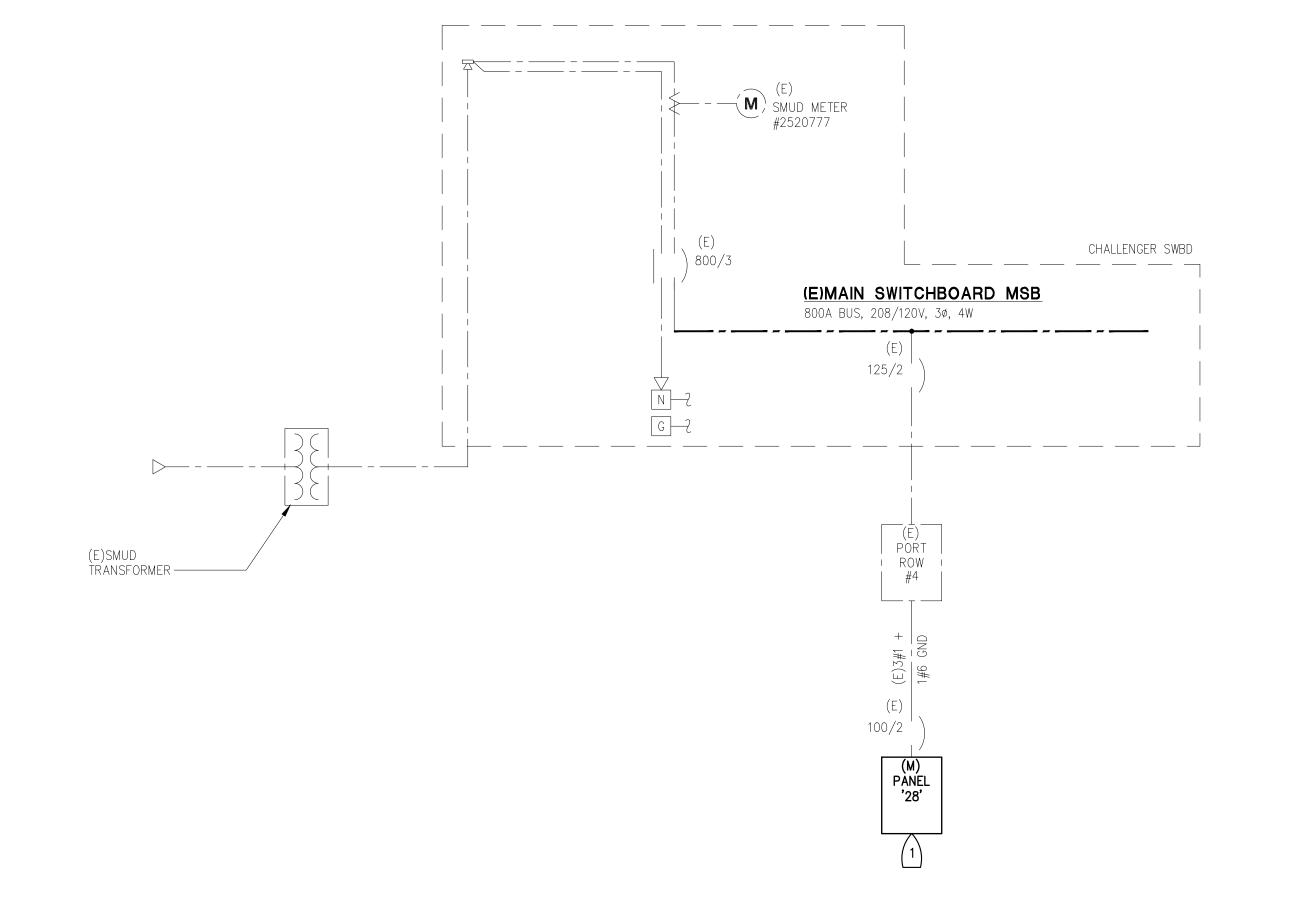
KEYED NOTES:

MODIFIED PANEL SERVES EQUIPMENT BEING ADDED IN THIS PROJECT. SEE PANEL SCHEDULE ON THIS SHEET FOR REFERENCE.







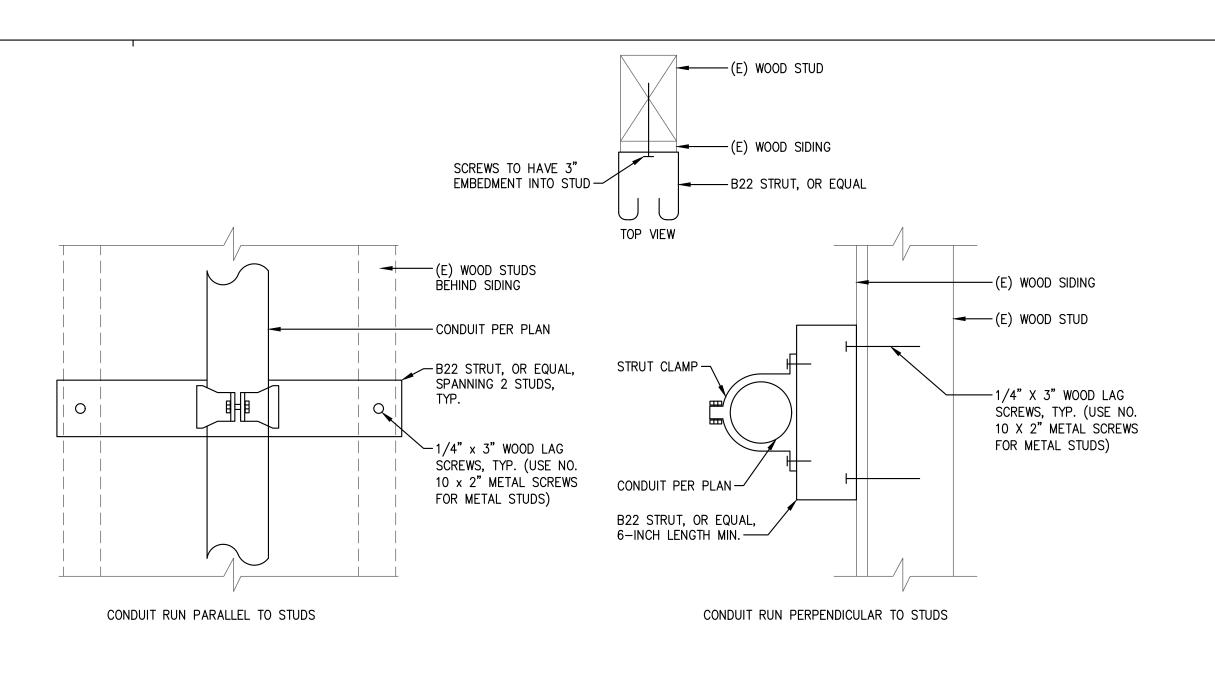


SHADE STRUCTURE ELEMENTARY SCHO Revision

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ONE LINE DIAGRAM

PROJECT NO.	1504.1
DATE:	3/21/202
SHEET	
E2.1	



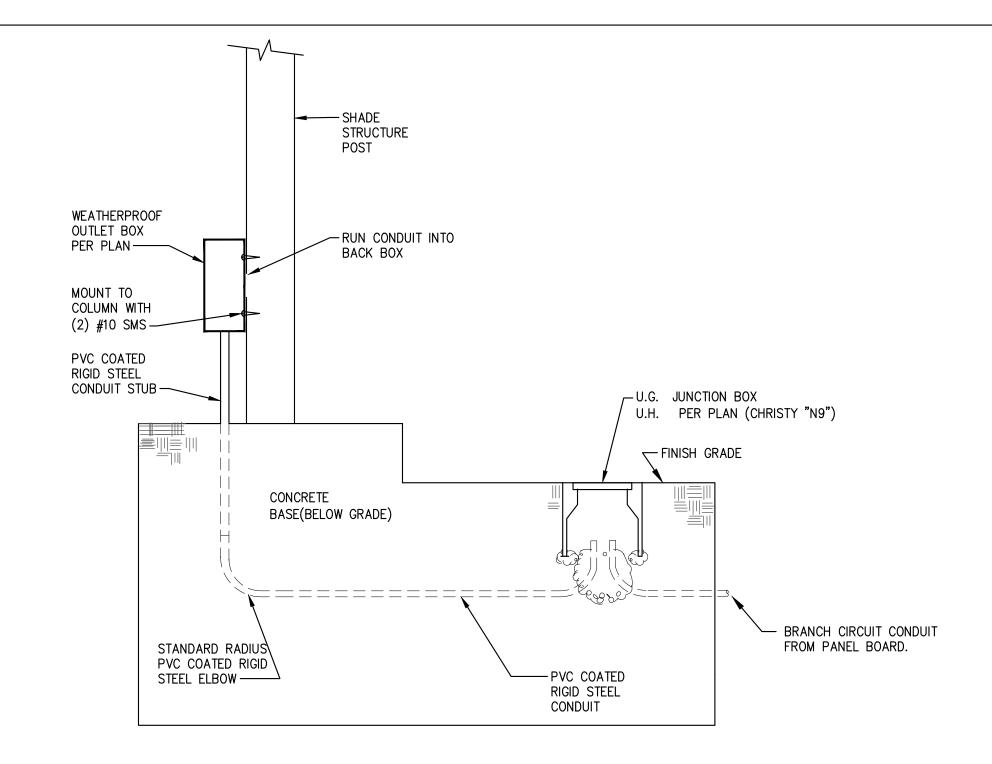
NOTES:

1. CONDUIT SHALL BE SUPPORTED AT INTERVALS NOT EXCEEDING TEN(10)
FEET AND NOT MORE THAN THREE(3) FEET FROM THE OUTLET AND AT
ANY POINT WHERE IT CHANGES DIRECTION.

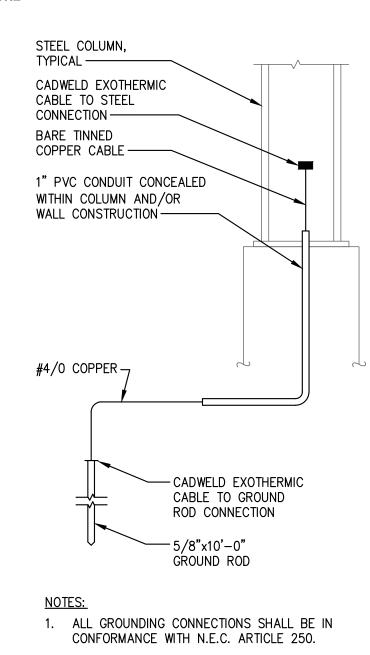
2. PERFORATED STRAP AND PLUMBER'S TAPE SHALL NOT BE PERMITTED.

3. MAXIMUM CONDUIT AND CONDUCTOR WEIGHT IS 1.83LBS PER LINEAR FOOT.

7 CONDUIT MOUNTING DETAIL - STUD WALLS
SCALE: NONE



4 CONDUIT STUB IN POST DETAIL
SCALE: NONE

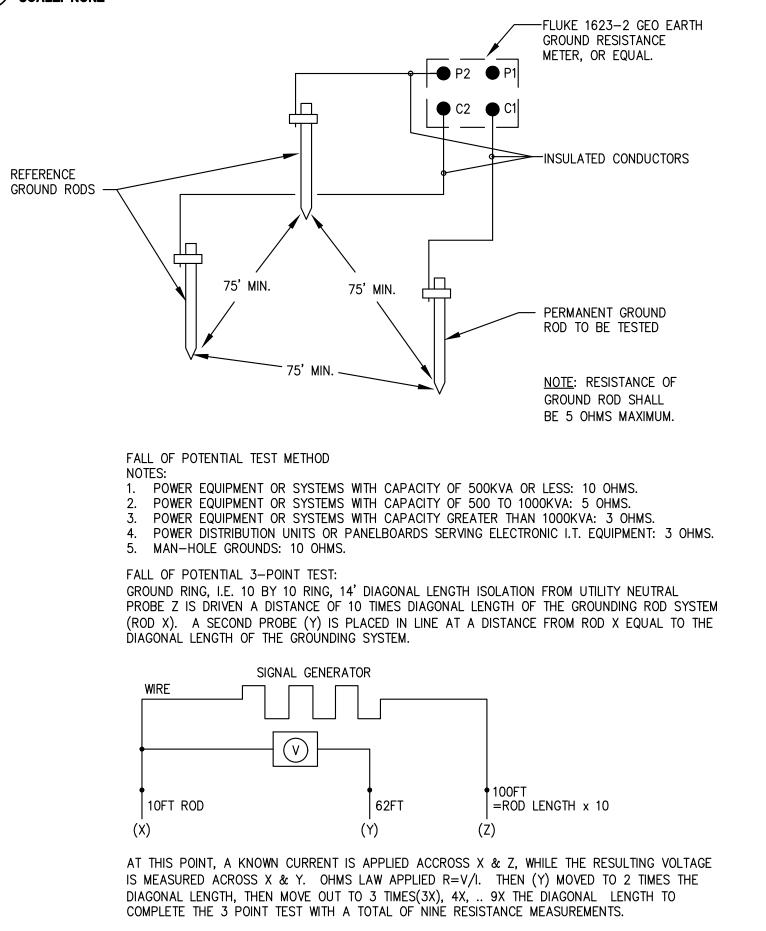


2. FOR ALL ADDITIONAL REQUIREMENTS REFER TO SPEC SECTIONS 26 05 26.

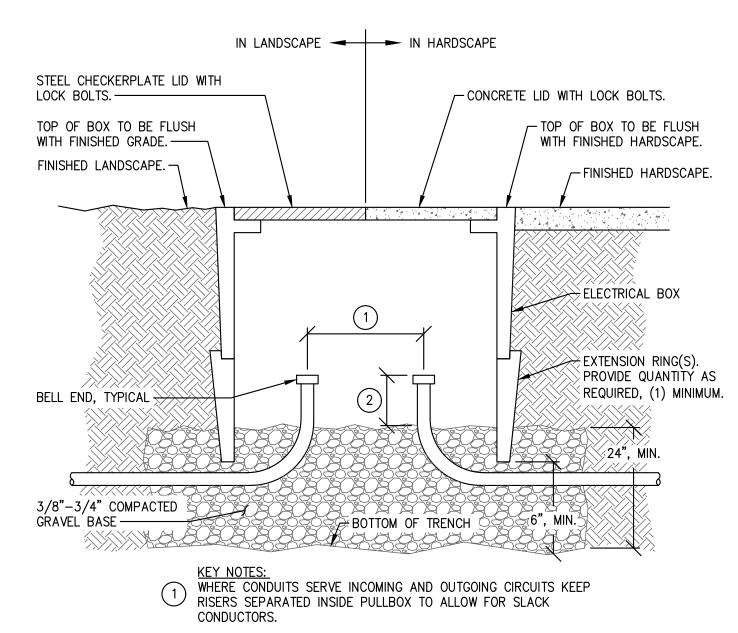
TYPICAL STEEL COLUMN

& REBAR GROUNDING DETAIL

SCALE: NONE

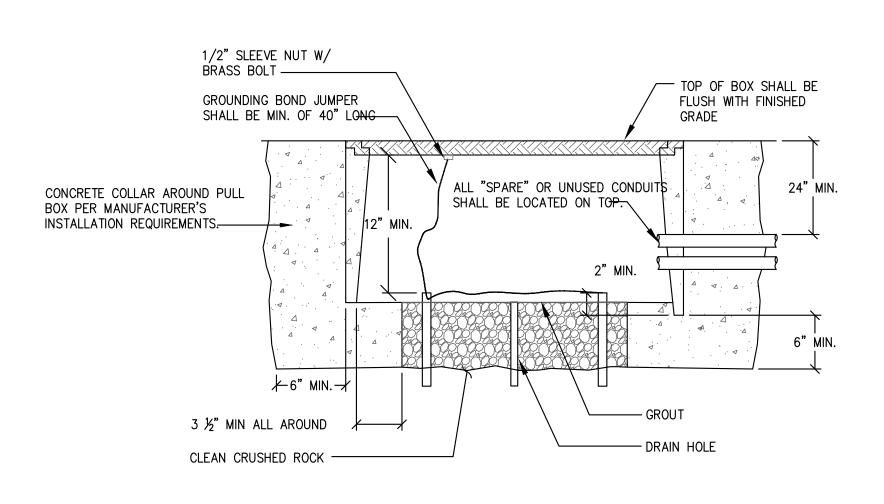


6 METHOD OF TESTING GROUND RODS DETAIL SCALE: NONE



TOPS OF CONDUITS MUST NOT EXTEND INTO PULLBOX MORE THAN 1/3 OF THE TOTAL AVAILABLE INSIDE BOX HEIGHT. IN ORDER TO ALLOW ADEQUATE SPACE FOR CABLE SLACK.

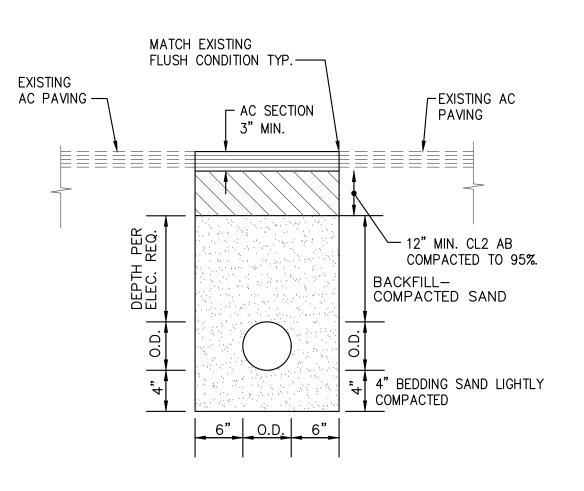
# 1 NON-TRAFFIC RATED PULL BOX SCALE: NONE



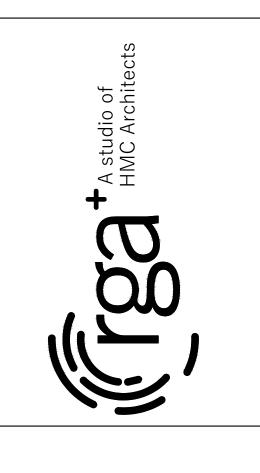
1. HANDHOLES SHALL BE PROVIDE WITH A MINIMUM OF (4) GALVANIZED PULLING PLATES IN BOTTOM OF PULLBOX.

- 2. PULLBOXES SHALL BE PROVIDED WITH CAST IN PLACE VERTICAL CABLE RACKS. ALL CABLES SHALL BE NEATLY BUNDLED, ORGANIZED AND SUPPORTED BY CABLE RACKS.
- 3. WHERE ADDITIONAL CONDUIT ENTRIES ARE REQUIRED BEYOND QUANTITY OF TERMINATORS SHOWN. CONTRACTOR SHALL FIELD CORE DRILL AS REQUIRED. WHERE 4" TERMINATORS ARE PROVIDED CONTRACTOR SHALL PROVIDE CONDUIT REDUCERS TO MATCH SITE CONDUIT SIZE REQUIREMENTS.
- 4. FOR ALTERNATE STYLE PULLBOXES CONTRACTOR SHALL FIELD CORE DRILL ALL CONDUIT ENTRIES 2" DIA AND SMALLER.
- 5. CONTRACTOR SHALL PROVIDE THE MANUFACTURER'S INSTALLATION INSTRUCTIONS FOR TRAFFIC RATING REQUIREMENTS AS PART OF THE SUBMITTALS.

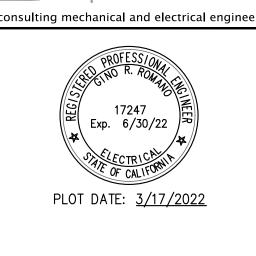
# 2 TRAFFIC RATED PULL BOX SCALE: NONE



3 TYPICAL TRENCH DETAIL
SCALE: NONE







TRUCTURE AT ELDER CREEK FARY SCHOOL

SACRAMENTO CITY U

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HADE

SI

PROJECT NO.		1504.
DATE:		3/21/20
SHEET	E3.1	
	DATE:	DATE:

SOCF DEAD LOAD (SUPERIMPOSED ON FRAME)   5 PSF	
ROOF LIVE LOAD         20 P           ROOF DEAD LOAD (SUPERIMPOSED ON FRAME)         5 PSF           ROOF PANEL DEAD LOAD         M=1.1 PSF, G=1.2           COLLATERAL DEAD LOAD         M=3.9 PSF, G=3.1           ROOF SNOW LOAD.         20 P           SROUND SNOW LOAD. Pg         20 P           ROOF SNOW LOAD: SLOPED, Pg         20 P           SITE APPLICATION DSA REVIEWER SHALL VERIFY THE STRUCTURE BE LOCATED AT LEAST 20 FEET FROM A SNOW LOAD LOPE FACTOR, Cg         11.6           SNOW EXPOSURE FACTOR, Cg         11.6           SNOW LOAD INPORTANCE FACTOR, Ig         1.1           HERRINA FACTOR, Cg         1.1           SASIC WIND SPEED (3 SECOND GUST), V <sub>cl</sub> 10.0           RISK CATEGORY         0           SASIC WIND SPEED (3 SECOND GUST), V <sub>cl</sub> 10.0           RISK CATEGORY         0           EXPOSURE CATEGORY         0           CACTORS, Kg, Kg, Sc         0.855, 1           Rg DOWER PASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED         CASE A (1.1 / -1.2) G           Cow, PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED         CASE A (0.6 / -0.9)           COMPONENTS & CLADDING - Cg (1 PRESSURE/SUCTION) CLEAR / OBSTRUCTED         CASE A (0.6 / -0.9)           COMPER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED         CASE A (0.6 /	VALUES
SOSP EAD LOAD (SUPERIMPOSED ON FRAME)   5 PSF	DOE
ROOF PANEL DEAD LOAD	_
COLLATERAL DEAD LOAD   ROOF SNOW LOAD	
ROOF SNOW LOAD   ROOF SNOW LOAD   ROOF SNOW LOAD   ROOF SNOW LOAD   ROOF SNOW LOAD SLOPED, P.   20 P   RISK CATEGORY   ■   ■   ROOF SNOW LOAD SLOPED, P.   20 P   STITE APPLICATION DSA REVIEWER SHALL VERIFY THE STRUCTURE BE LOCATED AT LEAST 20 FEET FROM A SNOW LOAD SLOPE FACTOR, C.   1.0   1	
I   ROBERT   ROBER	,
ROOF SNOW LOAD: SLOPED, P <sub>8</sub> STE APPLICATION DSA REVIEWER SHALL VERIFY THE STRUCTURE BE LOCATED AT LEAST 20 FEET FROM A SNOW LOAD SLOPE FACTOR, C <sub>9</sub> 1.6  SNOW LOAD SLOPE FACTOR, C <sub>9</sub> 1.7  SNOW LOAD SLOPE FACTOR, C <sub>9</sub> 1.8  SNOW LOAD IMPORTANCE FACTOR, I <sub>8</sub> 1.9  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 100 M  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 100 M  SISK CATEGORY  1.8  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 100 M  SISK CATEGORY  1.8  CATORS: K <sub>2</sub> , K <sub>2</sub> , K <sub>3</sub> 1.9  1.9  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC	PSF
SITE APPLICATION DSA REVIEWER SHALL VERIFY THE STRUCTURE BE LOCATED AT LEAST 20 FEET FROM A SNOW LOAD SLOPE FACTOR, C <sub>0</sub> 1.0.  SNOW EXPOSURE FACTOR, C <sub>0</sub> 1.0.  SNOW EXPOSURE FACTOR, C <sub>0</sub> 1.1.  SNOW EXPOSURE FACTOR, C <sub>0</sub> 1.1.  SNOW EXPOSURE FACTOR, C <sub>0</sub> 1.2.  SNOW EXPOSURE FACTOR, C <sub>0</sub> 1.3.  SNOW EXPOSURE FACTOR, C <sub>0</sub> 1.4.  SNOW EXPOSURE FACTOR, C <sub>0</sub> 1.5.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.6.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.7.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.8.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.9.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.	1
SNOW EXPOSURE FACTOR, C <sub>e</sub> 1.1. SNOW LOAD IMPORTANCE FACTOR, I <sub>e</sub> 1.2. THERMAL FACTOR, C <sub>1</sub> 3.  WIND DESIGN  BASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.  BASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.  BASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.  1.  1.  1.  1.  1.  1.  1.  1.  1	PSF
SNOW EXPOSURE FACTOR, C <sub>0</sub> 1.1.  SNOW LOAD IMPORTANCE FACTOR, I <sub>k</sub> 1.1.  THERMAL FACTOR, C <sub>1</sub> BASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 100 M  RISK CATEGORY  CACTORS: K <sub>0</sub> , K <sub>th</sub> , K <sub>d</sub> Question (3 M  Question (4 M  Question	ADJACENT STRUCTURE
SENOW LOAD IMPORTANCE FACTOR, I <sub>1</sub> THERMAL FACTOR, C <sub>1</sub> MIND DESIGN  BASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 100 M  RISK CATEGORY  RISK CATEGORY  CEXPOSURE CATEGORY  COACTION  A <sub>1</sub> = 0.00256 K, K <sub>1</sub> , K <sub>2</sub> K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>4</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>4</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>4</sub> B <sub>5</sub> = 0.00256 K, K <sub>4</sub> B <sub>4</sub> = 0.00256 K, K <sub>4</sub> B <sub>5</sub> = 0.00256 K, K <sub>4</sub> B <sub>4</sub> =	.0
THERMAL FACTOR, C, WIND DESIGN  BASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> . 100 M RISK CATEGORY   II  EXPOSURE CATEGORY   II  EXPOSURE CATEGORY   C  FACTORS, K <sub>x</sub> , K <sub>x</sub> , K <sub>x</sub>   0.85, 1  g <sub>x</sub> = 0.00256 K, K <sub>x</sub> , K <sub>x</sub>   0.85, 1  g <sub>x</sub> = 0.00256 K, K <sub>x</sub> , K <sub>x</sub>   0.85, 1  g <sub>x</sub> = 0.00256 K, K <sub>x</sub>   0.80 F ANGLE 18.43 - CLEAR / OBSTRUCTED   CASE A (1.1 /-1.2)    C <sub>ML</sub> PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED   CASE A (0.17 /-1.09)    C <sub>ML</sub> PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED   CASE A (-0.17 /-1.09)    C <sub>ML</sub> PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED   CASE A (-0.17 /-1.09)    COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED   CONE 3 - (0.29 /-  ZONE 2 - (1.77 /-  ZONE 1 - (1.15 /-  SEISMIC DESIGN   STEEL - ORDINARY CA  ANALYSIS PROCEDURE   STEEL - ORDINARY CA  SEISMIC IMPORTANCE FACTOR, Ie   EQUIVALENT LA  SEISMIC SITE CLASS   D  MCE <sub>R</sub> SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>3</sub>   0.9  SHORT PERIOD SITE COEFFICIENT, F <sub>a</sub>   1.2  LONG PERIOD COEFFICIENT, F <sub>a</sub>   1.7  FUNDAMENTAL PERIOD OF THE STRUCTURE, T   0.15  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> - USED   2.08 * 0.70  TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3)   1.0  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub>   1.0  SEISMIC DESIGN CATEGORY   1.2  RESPONSE MODIFICATION FACTOR, R   1.2  OVERSTRENGTH FACTOR, Ω   1.2  RESDUNDANCY FACTOR, Ω   1.2  RESD	.0
THERMAL FACTOR, C, WIND DESIGN  BASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> . 100 M RISK CATEGORY   II  EXPOSURE CATEGORY   II  EXPOSURE CATEGORY   C  FACTORS, K <sub>x</sub> , K <sub>x</sub> , K <sub>x</sub>   0.85, 1  g <sub>x</sub> = 0.00256 K, K <sub>x</sub> , K <sub>x</sub>   0.85, 1  g <sub>x</sub> = 0.00256 K, K <sub>x</sub> , K <sub>x</sub>   0.85, 1  g <sub>x</sub> = 0.00256 K, K <sub>x</sub>   0.80 F ANGLE 18.43 - CLEAR / OBSTRUCTED   CASE A (1.1 /-1.2)    C <sub>ML</sub> PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED   CASE A (0.17 /-1.09)    C <sub>ML</sub> PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED   CASE A (-0.17 /-1.09)    C <sub>ML</sub> PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED   CASE A (-0.17 /-1.09)    COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED   CONE 3 - (0.29 /-  ZONE 2 - (1.77 /-  ZONE 1 - (1.15 /-  SEISMIC DESIGN   STEEL - ORDINARY CA  ANALYSIS PROCEDURE   STEEL - ORDINARY CA  SEISMIC IMPORTANCE FACTOR, Ie   EQUIVALENT LA  SEISMIC SITE CLASS   D  MCE <sub>R</sub> SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>3</sub>   0.9  SHORT PERIOD SITE COEFFICIENT, F <sub>a</sub>   1.2  LONG PERIOD COEFFICIENT, F <sub>a</sub>   1.7  FUNDAMENTAL PERIOD OF THE STRUCTURE, T   0.15  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> - USED   2.08 * 0.70  TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3)   1.0  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub>   1.0  SEISMIC DESIGN CATEGORY   1.2  RESPONSE MODIFICATION FACTOR, R   1.2  OVERSTRENGTH FACTOR, Ω   1.2  RESDUNDANCY FACTOR, Ω   1.2  RESD	.0
WIND DESIGN           BASIC WIND SPEED (3 SECOND GUST), V <sub>ol</sub> R         100 M           RISK CATEGORY         0.85, 1           EXPOSURE CATEGORY         0.85, 1           10 <sub>a</sub> = 0.00256 K <sub>x</sub> K <sub>xt</sub> V <sub>x</sub> FOR ALL EAVE HEIGHTS (8', 10' & 12')         18.50           0.2 <sub>MV</sub> PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED         CASE A (1.1 / -1.2)           0.8 <sub>MV</sub> PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED         CASE A (-0.17 / -1.09)           0.2 <sub>MV</sub> PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED         CASE A (-0.6 / -0.9)           0.00MPONENTS & CLADDING - C <sub>N</sub> (PRESSURE/SUCTION) CLEAR / OBSTRUCTED         CASE A (-0.6 / -0.9)           0.00MPONENTS & CLADDING - C <sub>N</sub> (PRESSURE/SUCTION) CLEAR / OBSTRUCTED         ZONE 3 - (2.29 / -2.20	2
BASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> RISK CATEGORY  RISK CATEGORY  CATEGORY  CASEA (-0.85, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1,	
RISK CATEGORY  EXPOSURE CATEGORY  C EXPOSURE CATEGORY  C DATE OF THE EXPOSURE CATEGORY  RESPONSE MODIFICATION FACTOR, ρ  RESPONSE MODIFICATION FOR TICK. 20' WIDE, 30' WIDE, 40' WIDE)  1.10 DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  1.11 DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  1.11 DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  1.12 DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  1.14 DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  1.15 DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  1.16 DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  1.17 PENDAMENTAL OF CATEGORY  C DASS A (-0.0 f, 0.15)  C ASE A (-0.1 f, -1.2)	MPH
EXPOSURE CATEGORY FACTORS: K <sub>2</sub> , K <sub>2</sub> , K <sub>3</sub> 0.85, 1  The = 0.00256 K <sub>2</sub> K <sub>2</sub> K <sub>4</sub> V <sub>2</sub> Por All EAVE HEIGHTS (8', 10' & 12') 1.9, = 0.00256 K <sub>2</sub> K <sub>2</sub> K <sub>4</sub> V <sub>2</sub> Por All EAVE HEIGHTS (8', 10' & 12') 1.9, = 0.00256 K <sub>3</sub> K <sub>4</sub> K <sub>4</sub> V <sub>2</sub> Por All EAVE HEIGHTS (8', 10' & 12') 1.9, PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED 1.0, PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED 1.0, PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED 1.0, PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED 1.0, PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED 1.0, PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED 1.0, PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED 1.0, PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED 1.0, PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED 1.0, PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED 1.0, PER SUBJECT OBSTRUCTER, TO 1.1, PER SUBJECT OBSTRUCTED 1.0, PER SUBJECT OBSTRUCTER, TO 1.1, PER SUBJECT OBSTRUCTED 1.0, PER SUBJECT OBSTRUCTURE, TO 1.1, PER SUBJECT OBSTRUCTED 1.0, PER SUBJECT OBSTRUCTURE, TO 1.1, PER SUBJECT OBSTRUCTED	
FACTORS: K <sub>2</sub> , K <sub>2</sub> , K <sub>3</sub> Q <sub>1</sub> = 0.00256 K <sub>2</sub> K <sub>2</sub> , K <sub>3</sub> V <sup>2</sup> FOR ALL EAVE HEIGHTS (8', 10' & 12')  18.50 C <sub>MW</sub> PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED CASE A (1.1 / -1.2) C <sub>ML</sub> PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED CASE A (-0.17 / -1.09) C <sub>M</sub> PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLEAR - (OBSTRUCTED COMPONENTS & CLAD - (OBSTRUCTED COMPONENTS & CLACE COMPONENTS & COMPONENTS	
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	1, 0.85
C <sub>MM</sub> PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED         CASE A (-0.17 /-1.09)           C <sub>NL</sub> PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED         CASE A (-0.17 /-1.09)           C <sub>N</sub> PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED         CASE A (-0.6 /-0.9)           COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED         ZONE 3 - (2.29 /-2           ZONE 2 - (1.77 /-1         ZONE 1 - (1.15 /-1           SEISMIC DESIGN         ZONE 1 - (1.15 /-1           LATERAL FORCE RESISTING SYSTEM         STEEL - ORDINARY C/A           ANALYSIS PROCEDURE         EQUIVALENT LA           SEISMIC IMORTANCE FACTOR, Ie         11.0           SEISMIC SITE CLASS         D           MCE <sub>R</sub> SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>s</sub> 2.6           MCE <sub>R</sub> SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>s</sub> 2.6           MCE <sub>R</sub> SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>s</sub> 1.2           LONG PERIOD COEFFICIENT, F <sub>s</sub> 1.2           LONG PERIOD COEFFICIENT, F <sub>s</sub> 1.2           DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> - USED         2.08 * 0.70           DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> - USED         2.08 * 0.70           DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> 1.0           DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-	
$C_{NL} \ PER \ ASCE FIGURE 27.4-5 \ ROOF \ ANGLE 18.43 - CLEAR / OBSTRUCTED                                    $	
$ \begin{array}{c} C_N  \text{PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED} & CASE A  (-0.6  /  -0.9) \\ \hline COMPONENTS & CLADDING - C_N  (  \text{PRESSURE/SUCTION})  \text{CLEAR / OBSTRUCTED}} & ZONE  3 -  (2.29  /  \cdot  20  \text{Ne}  2 -  (1.77  /  -  20  /  -  20  \text{NE}  2 -  (1.28  /  -  20  ,  -  20  \text{NE}  2 $	· · · · · · · · · · · · · · · · · · ·
COMPONENTS & CLADDING - $C_N$ ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED  ZONE 3 - $(2.29)$ / $(2.29)$ / $(2.20)$	
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	· · · · · · · · · · · · · · · · · · ·
$ \begin{array}{c} \text{ZONE 1 - } 1.15 I - \\ \text{SEISMIC DESIGN} \\ \\ \text{LATERAL FORCE RESISTING SYSTEM} \\ \text{STEEL - ORDINARY CAMALYSIS PROCEDURE} \\ \text{SESIMIC IMORTANCE FACTOR, Ie} \\ \text{SEISMIC SITE CLASS} \\ \text{MCE}_R \text{ SPECTRAL RESPONSE ACCELERATION @ 0.2 \text{ s, S}_S } \\ \text{2.6} \\ \text{MCE}_R \text{ SPECTRAL RESPONSE ACCELERATION @ 0.2 \text{ s, S}_1 } \\ \text{3.1} \\ \text{3.2} \\ \text{4.1} \\ \text{5.2} \\ \text{5.2} \\ \text{5.3} \\ \text{5.4} \\ \text{5.2} \\ \text{6.4} \\ \text{6.2} \\ \text{6.2} \\ \text{6.2} \\ \text{6.2} \\ \text{6.2} \\ \text{6.2} \\ \text{6.3} \\ \text{6.2} \\ \text{6.2} \\ \text{6.3} \\ \text{6.2} \\ \text{6.2} \\ \text{6.3} \\ \text{6.2} \\ \text{6.3} \\ \text{6.2} \\ \text{6.3} \\ \text{6.3} \\ \text{6.4} \\ \text{6.2} \\ \text{6.2} \\ \text{6.2} \\ \text{6.2} \\ \text{6.3} \\ \text{6.2} \\ \text{6.3} \\ \text{6.3} \\ \text{6.4} \\ \text{6.2} \\ \text{6.2} \\ \text{6.3} \\ \text{6.2} \\ \text{6.3} \\ \text{6.3} \\ \text{6.3} \\ \text{6.3} \\ \text{6.4} \\ \text{6.4} \\ \text{6.2} \\ \text{6.2} \\ \text{6.4} \\ \text{6.2} \\ \text{6.4} \\ 6.4$	
LATERAL FORCE RESISTING SYSTEM  ANALYSIS PROCEDURE  EQUIVALENT LA SESIMIC IMORTANCE FACTOR, Ie  SEISMIC SITE CLASS  D $MCE_R$ SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>S</sub> $C.6$ $MCE_R$ SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>1</sub> $C.6$ SHORT PERIOD SITE COEFFICIENT, F <sub>a</sub> $C.6$ LONG PERIOD COEFFICIENT, F <sub>v</sub> $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $C.6$ DESIGN CATEGORY  EQUIVALENT LA  DESIGN CATEGORY  EQUIVALENT LA  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $C.6$	-1.05) / (0.5 / -1.5)
ANALYSIS PROCEDURE  SESIMIC IMORTANCE FACTOR, le  1.6  SEISMIC SITE CLASS $MCE_R$ SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>S</sub> 2.6 $MCE_R$ SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>S</sub> 3.7  SHORT PERIOD SITE COEFFICIENT, F <sub>a</sub> 1.7  FUNDAMENTAL PERIOD OF THE STRUCTURE, T  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> 2.0  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> - USED  TO DETERMINE CS (WITH CAP PER ASCE-7 12.8.1.3)  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> 1.0  SEISMIC DESIGN CATEGORY  E  RESPONSE MODIFICATION FACTOR, R  1.2  OVERSTRENGTH FACTOR, $\Omega$ 1.2  REDUNDANCY FACTOR, $\rho$ 1.4  HORIZONTAL OR VERTICAL IRREGULARITIES  SEISMIC RESPONSE COEFFICIENT, CS (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  1.2.73 PSF, 13.41	ANTILEVER COLUMN
SEISMIC SITE CLASS $MCE_R$ SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>S</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>S</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> - USED $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> - USED $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S <sub>D2</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S <sub>D2</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S <sub>D2</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S <sub>D2</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S <sub>D2</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S <sub>D2</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S <sub>D2</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S <sub>D2</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S <sub>D2</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S <sub>D2</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT S	
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	.0
MCE <sub>R</sub> SPECTRAL RESPONSE ACCELERATION @ $0.2 \text{ s}$ , $S_1$ SHORT PERIOD SITE COEFFICIENT, $F_a$ LONG PERIOD COEFFICIENT, $F_v$ 1.7  FUNDAMENTAL PERIOD OF THE STRUCTURE, T  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ 2.0  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ - USED  TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3)  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, $S_{D1}$ 1.0  SEISMIC DESIGN CATEGORY  ERSPONSE MODIFICATION FACTOR, R  OVERSTRENGTH FACTOR, $D$ 1.2  REDUNDANCY FACTOR, $D$ 1.4  HORIZONTAL OR VERTICAL IRREGULARITIES  SEISMIC RESPONSE COEFFICIENT, Cs ( $D_{CO}$ WIDE, $D_{CO}$ W	)
SHORT PERIOD SITE COEFFICIENT, $F_a$ LONG PERIOD COEFFICIENT, $F_v$ 1.7  FUNDAMENTAL PERIOD OF THE STRUCTURE, T  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ 2.0  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ - USED  TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3)  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, $S_{D1}$ 1.0  SEISMIC DESIGN CATEGORY  ERSPONSE MODIFICATION FACTOR, R  OVERSTRENGTH FACTOR, $\Omega$ 1.2  REDUNDANCY FACTOR, $\rho$ HORIZONTAL OR VERTICAL IRREGULARITIES  NON  SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  12.73 PSF, 13.41	60
LONG PERIOD COEFFICIENT, $F_V$ FUNDAMENTAL PERIOD OF THE STRUCTURE, T  0.15.  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ 2.0  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ - USED  TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3)  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, $S_{D1}$ 1.0  SEISMIC DESIGN CATEGORY  ERSPONSE MODIFICATION FACTOR, R  1.2  OVERSTRENGTH FACTOR, $\Omega$ 1.2  REDUNDANCY FACTOR, $\rho$ HORIZONTAL OR VERTICAL IRREGULARITIES  NON  SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  12.73 PSF, 13.41	90
FUNDAMENTAL PERIOD OF THE STRUCTURE, T 0.15.  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ 2.0  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ - USED 2.08 * 0.70  TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3)  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, $S_{D1}$ 1.0  SEISMIC DESIGN CATEGORY E RESPONSE MODIFICATION FACTOR, R 1.2  OVERSTRENGTH FACTOR, $\Omega$ 1.2  REDUNDANCY FACTOR, $\rho$ 1.0  HORIZONTAL OR VERTICAL IRREGULARITIES NON SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE) 1.16  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE) 12.73 PSF, 13.41	20
FUNDAMENTAL PERIOD OF THE STRUCTURE, T 0.15.  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ 2.0  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ - USED 2.08 * 0.70  TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3)  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, $S_{D1}$ 1.0  SEISMIC DESIGN CATEGORY E RESPONSE MODIFICATION FACTOR, R 1.2  OVERSTRENGTH FACTOR, $\Omega$ 1.2  REDUNDANCY FACTOR, $\rho$ 1.0  HORIZONTAL OR VERTICAL IRREGULARITIES NON SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE) 1.16  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE) 12.73 PSF, 13.41	70
DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ 2.0  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ - USED 2.08 * 0.70  TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3)  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, $S_{D1}$ 1.0  SEISMIC DESIGN CATEGORY  RESPONSE MODIFICATION FACTOR, R 1.2  OVERSTRENGTH FACTOR, $\Omega$ 1.2  REDUNDANCY FACTOR, $\rho$ 1.0  HORIZONTAL OR VERTICAL IRREGULARITIES NON  SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE) 1.16  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE) 12.73 PSF, 13.41	52 s
DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ - USED  TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3)  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, $S_{D1}$ SEISMIC DESIGN CATEGORY  RESPONSE MODIFICATION FACTOR, R  OVERSTRENGTH FACTOR, $\Omega$ REDUNDANCY FACTOR, $\rho$ HORIZONTAL OR VERTICAL IRREGULARITIES  SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  12.73 PSF, 13.41	
TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3)  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> SEISMIC DESIGN CATEGORY  RESPONSE MODIFICATION FACTOR, R  OVERSTRENGTH FACTOR, Ω  REDUNDANCY FACTOR, ρ  HORIZONTAL OR VERTICAL IRREGULARITIES  SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  12.73 PSF, 13.41	08
TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3)  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> SEISMIC DESIGN CATEGORY  RESPONSE MODIFICATION FACTOR, R  OVERSTRENGTH FACTOR, Ω  REDUNDANCY FACTOR, ρ  HORIZONTAL OR VERTICAL IRREGULARITIES  SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  12.73 PSF, 13.41	
DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> SEISMIC DESIGN CATEGORY  RESPONSE MODIFICATION FACTOR, R  OVERSTRENGTH FACTOR, Ω  REDUNDANCY FACTOR, ρ  HORIZONTAL OR VERTICAL IRREGULARITIES  SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  12.73 PSF, 13.41	<b>7</b> 0 = 1.456
SEISMIC DESIGN CATEGORY  RESPONSE MODIFICATION FACTOR, R  1.2  OVERSTRENGTH FACTOR, Ω  REDUNDANCY FACTOR, ρ  HORIZONTAL OR VERTICAL IRREGULARITIES  SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  12.73 PSF, 13.41	
SEISMIC DESIGN CATEGORY  RESPONSE MODIFICATION FACTOR, R  1.2  OVERSTRENGTH FACTOR, Ω  REDUNDANCY FACTOR, ρ  HORIZONTAL OR VERTICAL IRREGULARITIES  SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  12.73 PSF, 13.41	
RESPONSE MODIFICATION FACTOR, R  OVERSTRENGTH FACTOR, $\Omega$ REDUNDANCY FACTOR, $\rho$ HORIZONTAL OR VERTICAL IRREGULARITIES  SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  12.73 PSF, 13.41	
OVERSTRENGTH FACTOR, $\Omega$ 1.2  REDUNDANCY FACTOR, $\rho$ 1.0  HORIZONTAL OR VERTICAL IRREGULARITIES NON SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE) 1.10  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE) 12.73 PSF, 13.41	
REDUNDANCY FACTOR, ρ  HORIZONTAL OR VERTICAL IRREGULARITIES  SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  12.73 PSF, 13.41	
HORIZONTAL OR VERTICAL IRREGULARITIES  SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  12.73 PSF, 13.41	
SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  1.10  1.2.73 PSF, 13.41	
ALLOWABLE SOIL BEARING FOR FOUNDATIONS  VARIES - SEE FOUN	· ·
ALLOWABLE SOIL BEARING FOR FOUNDATIONS VARIES - SEE FOUN	
	NDATION CHARTS
FLOOD DESIGN - DESIGN IS ASSUMED TO NOT BE IN FLOOD HAZARD AREA	
IF PROJECT IS LOCATED IN A FLOOD ZONE OTHERTHAN ZONE X, A LETTER STAMPED & SIGNED FROM A SOILS ENGINEER IS REQUIRED TO VALIDATE THE	

ALL DEFLECTIONS SHOWN ALSO INCLUDE THE P-DE	LTA ROTATION PER IR PC-7	DEFLECT	IONS ARE FOR (1) STI	RUCTURE
		SOIL	CLASSES PER CBC TABLE 18	06A.2
MAXIMUM DRIFT $\delta_{max}$ SIDE COLUMNS		Soil Class 5	Soil Class 4	Soil Class 3
20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.40	2.55	2.65
30' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.25	2.35	2.45
40' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT) MINIMUM SEPARATION $(\delta_m = C_d \ \delta_{max})$ $C_d = 1.25$	(INCHES)	2.20	2.25	2.20
20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	3.00	3.19	3.31
30' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.81	2.94	3.06
40' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.75	2.81	2.75
MAXIMUM DRIFT $\delta_{max}$ CORNER COLUMNS		Soil Class 5	Soil Class 4	Soil Class 3
20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.20	2.30	2.40
30' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.30	2.45	2.50
40' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT) MINIMUM SEPARATION $(\delta_m = C_d \ \delta_{max})$ $C_d = 1.25$	(INCHES)	2.40	2.55	2.65
20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.75	2.88	3.00
30' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.88	3.06	3.13
40' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	3.00	3.19	3.31
MAXIMUM DRIFT $\delta_{max}$ END COLUMNS		Soil Class 5	Soil Class 4	Soil Class 3
20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	1.60	1.70	1.75
30' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.00	2.45	2.25
40' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT) MINIMUM SEPARATION $(\delta_m = C_d \ \delta_{max})$ $C_d = 1.25$	(INCHES)	2.50	2.30	2.80
20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.00	2.13	2.19
30' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.50	3.06	2.81
40' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	3.13	2.88	3.50

STRUCTURAL SEPARATION

ARCHITEC TURAL REQUIREMENTS			
DESC RIPTION	DESIGN VAULES		
TYPE OF CONSTRUCTION	II-B		
OCCUPANCY CLASSIFICATION	A-3		
NUMBER OF STORIES	1		
FIRE SPRINKLER SYSTEM	NOT BY ICON/WEIGHT NOT INCLUDED IN DESIGN		

#### RELATED BUILDING CODES AND STANDARDS

#### TITLE 24 CODES:

2019 CALIFORNIA ADMINISTRATIVE CODE (CAC).. ..(PART 1, TITLE 24, CCR) 2019 CALIFORNIA BUILDING CODE (CBC), VOLUMES 1, AND 2.(PART 2, TITLE 24,

2019 CALIFORNIA ELECTRICAL CODE. .(PART 3, TITLE 24, CCR) 2019 CALIFORNIA MECHANICAL CODE (CMC). .(PART 4, TITLE 24, CCR) (PART 5, TITLE 24, CCR) 2019 CALIFORNIA PLUMBING CODE (CPC).. 2019 CALIFORNIA ENERGY CODE. (PART 6, TITLE 24, CCR) 2019 CALIFORNIA FIRE CODE (CFC) .(PART 9, TITLE 24, CCR` 2019 CALIFORNIA GREEN BUILDING STANDARDS CODE.....(PART 11, TITLE 24, CCR) 2019 CALIFORNIA REFERENCE STANDARDS CODE.. ..(PART 12, TITLE 24, CCR)

REFERENCE CODE SECTIONS FOR APPLICABLE STANDARDS: 2019 CBC, CHAPTER 35

#### 2019 CFC, CHAPTER 80 SCOPE OF WORK NARRATIVE

THESE DRAWINGS ILLUSTRATE THE FABRICATION AND INSTALLATION REQUIREMENTS FOR A FREE-STANDING PREFABRIC ATED STEEL SHADE STRUCTURE. THE ENTIRE STRUCTURAL SYSTEM IS COMPRISED OF HOLLOW STRUCTURAL STEEL MEMBERS SUPPORTED BY CONCRETE FOUNDATIONS. THE FLEXIBILITY INCLUDED HEREIN

ALLOWS THE STRUCTURE TO COMPLY WITH A WIDE VARIETY OF PROJECT SITES AND LOADING REQUIREMENTS.

- 1. GENERAL NOTES AND TYPICAL DETAILS SHALL APPLY TO ALL PARTS OF THE JOB EXCEPT WHERE THEY MAY CONFLICT WITH DETAILS AND NOTES ON OTHER SHEETS. WHERE CONDITIONS ARE NOT SPECIFICALLY INDICATED BUT ARE OF SIMILAR CHARACTER TO DETAILS SHOWN, SIMILAR DETAILS OF CONSTRUCTION SHALL BE USED SUBJECT TO REVIEW BY THE STRUCTURAL ENGINEER FOR THIS PROJECT
- 2. WORK SHALL CONFORM TO THE REQUIREMENTS, AS AMENDED TO DATE, OF THE LATEST ADOPTED EDITION OF THE CBC, C.A.C. TITLE 24, AND ALL OTHER LOCAL, STATE AND FEDERAL REGULATIONS
- 3. OMISSIONS OR CONFLICTS BETWEEN THE VARIOUS ELEMENTS OF THE WORKING DRAWINGS AND/OR SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER FOR THIS PROJECT PRIOR TO PROCEEDING
- 4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING THE WORK OF ALL TRADES AND SHALL CHECK ALL DIMENSIONS, ALL DISCREPANCIES SHALL BE CALLED TO THE ATTENTION OF THE STRUCTURAL ENGINEER FOR THIS PROJECT AND BE RESOLVED BEFORE PROCEEDING WITH THE WORK.
- 5. THESE CONSTRUCTION DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE AND DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND SHALL BE SOLELY RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES, INCLUDING, BUT NOT LIMITED TO, BRACING, TEMPORARY SUPPORTS, AND SHORING. OBSERVATION VISIT TO THE SITE BY FIELD REPRESENTATIVES OF THE ARCHITECT/ENGINEER SHALL NOT INCLUDE INSPECTIONS OF THE PROTECTIVE MEASURES OR THE CONSTRUCTION PROCEDURES. ANY SUPPORT SERVICES PERFORMED BY THE ARCHITECT/ENGINEER DURING THE CONSTRUCTION SHALL BE DISTINGUISHED FROM CONSTRUCTION AND DETAILED INSPECTION SERVICES WHICH ARE FURNISHED BY OTHERS. THESE SUPPORT SERVICES PERFORMED BY THE ARCHITECT/ENGINEER, WHETHER OF MATERIAL OR WORK, ARE FOR THE PURPOSE OF ASSISTING IN QUALITY
- CONTROL AND IN ACHIEVING CONFORMANCE WITH CONTRACT DOCUMENTS, BUT DO NOT GUARANTEE CONSTRUCTION. 6. ASTM DESIGNATIONS AND ALL STANDARDS REFER TO THE LATEST AMENDMENTS.
- 7. CONFORM TO APPLICABLE CAL/OSHA CONSTRUCTION SAFETY REGULATIONS FOR ALL WORK PERFORMED DURING CONSTRUCTION. JOB SITE SAFETY IS STRICTLY THE RESPONSIBILITY OF THE CONTRACTOR AND NOT THE ARCHITECT/ENGINEER OR OWNER.
- 8. THE ENGINEER AND THEIR CONSULTANTS SHALL HAVE NO RESPONSIBILITY FOR THE DISCOVERY, HANDLING, REMOVAL OR DISPOSAL OF HAZARDOUS MATERIALS AT THE PROJECT SITE, INCLUDING BUT NOT LIMITED TO
- ASBESTOS, ASBESTOS PRODUCTS, POLYCHLORINATED BIPHENYL (PCB) OR OTHER TOXIC SUBSTANCES. 9. SHOULD ANY CONDITIONS DEVELOP NOT COVERED BY THE CONTRACT DOCUMENTS, OR IF A CHANGE IN THE SCOPE OF WORK IS PROPOSED, A CONSTRUCTION CHANGE DOCUMENT DETAILING AND SPECIFYING THE REQUIRED
- CHANGE(S) SHALL BE SUBMITTED TO AND APPROVED BY DSA BEFORE PROCEEDING WITH THE WORK. 10. THE SCHOOL DISTRICT INSPECTOR ON RECORD SHALL INSPECT AND APPROVE THE ERECTED FRAME PRIOR TO ROOF
- 11. SEE REQUIREMENTS FOR LOCATION IN ANY FIRE HAZARD SEVERITY ZONE FOR WILDLAND URBAN INTERFACE AREAS (WUI) AS SPECIFIED IN THE APPLICABLE VERSION OF THE CALIFORNIA BUILDING CODE. PROVIDE PROTECTION AND DETAILS OF ALL AREAS COMPLYING WITH THE WUI REQUIREMENTS.
- 12. LOCATING THIS STRUCTURE CLOSER THAN 20 FEET TO OTHER STRUCTURES MAY AFFECT THE ALLOWABLE AREA
- FOR THE EXISTING CONSTRUCTION PER THE APPLICABLE VERSION OF THE CALIFORNIA BUILDING CODE. 13. VIEWS AND DETAILS ARE NOT DRAWN TO SCALE (UNLESS NOTED OTHERWISE). DO NOT SCALE THESE DRAWINGS.

#### STRUCTURAL AND MISCELLANEOUS STEEL:

- 1. ALL STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE AMERICAN INSTITUE OF STEEL CONSTRUCTION (AISC) SPECIFICATION MANUAL REFERENCED BY THE LATEST EDITION OF THE CALIFORNIA BUILDING CODE.
- 2. PIPE SECTIONS SHALL CONFORM TO ASTM A53,  $F_y = 35$  KSI, GRADE B OR A501 UNLESS NOTED OTHERWISE. 3. STRUCTURAL TUBING (HSS SHAPES) SHALL CONFORM TO ASTM A-500, GRADE B (OR C), Fy = 46 KSI (MIN).
- 4. IF MATERIAL AVAILABILITY IS LIMITED, MEMBER THICKNESS CAN BE INCREASED BEYOND WHAT IS SHOWN IN THESE DRAWINGS (MAXIMUM INCREASE OF 1/8").
- 5. ALL CHANNELS, ANGLES, AND MISC. STEEL SHALL CONFORM TO ASTM A-36, Fy =36 KSI.
- 6. ALL PLATE STEEL SHALL CONFORM TO ASTM A-572, Fy= 50 KSI.
- 7. ALL COLD FORM STEEL SHALL CONFORM TO ASTM A-653, CS = TYPE B, Fy = 50 KSI.
- 8. STRUCTURAL STEEL AND DECK SHALL BE IDENTIFIED FOR CONFORMITY PER CBC 2202A.1.
- 9. ALL ROOF DECKS SHALL HAVE KYNAR 500 METAL COATING. 10.ALL ROOF DECKS SHALL CONFORM TO ASTM A-792, Fy = 50 KSI.

#### INSTRUCTIONS FOR ARCHITECTS SUBMITTING THESE PRE-CHECKED DRAWING TO DSA: BEFORE SUBMITTING THESE PRE-CHECKED DRAWINGS FOR YOUR PROJECT, FOLLOW THE

STEP 1: SELECT FRAME DIMENSIONS FOR YOUR PROJECT -GABLE STRUCTURES UP TO 20' WIDE USE THE "RG 20" BASE FRAME -GABLE STRUCTURES UP TO 30' WIDE USE THE "RG 30" BASE FRAME

STEPS BELOW TO PROPERLY DEFINE THE APPROVED OPTIONS:

-GABLE STRUCTURES UP TO 40' WIDE USE THE "RG 40" BASE FRAME -MAXIMUM WIDTH IS 40' (SEE "ARCHITECTURAL VIEWS" SHEET FOR REFERENCE) -THE 24', 44', 64', 84' AND 104' LENGTHS ARE SUGGESTED BECAUSE THEY ARE THE MOST COMMON

(20' BAYS ARE THE MOST ECONOMICAL) -FRAME LENGTHS ASSUME 2' OVERHANGS (UNO BY ARCHITECT - 2' MAX DIMENSION)

STEP 2: SELECT ROOF DECK FOR YOUR PROJECT -"M" REPRESENTS McELROY METAL "MULTI-RIB" ROOF PANEL

-"G" REPRESENTS McELROY METAL "MEGA-RIB" ROOF PANEL -"S" REPRESENTS MCELROY METAL "MEDALLION-LOK" 16" STANDING SEAM ROOF PANEL

STEP 3: IDENTIFY THE Ss ACCELERATION (q) FOR YOUR PROJECT

-Ss VALUE DETERMINES THE REQUIRED SEISMIC DESIGN FORCES -Ss VALUE DEPENDS ON THE PROJECTS GEOGRAPHICAL LOCATION (VALUES RANGE FROM 0.00 TO 3.73)

STEP 4: IDENTIFY THE Ss REGION FOR YOUR PROJECT

-THE REGIONS ARE DEPENDANT ON THE Ss VALUE DETERMINED IN STEP 3 -THE SS REGION DICTATES THE MAXIMUM DEAD LOAD PERMITTED ON THE FRAME (SEE TABLE TO RIGHT)

STEP 5: IDENTIFY THE ROOF DEAD LOAD FOR YOUR PROJECT -THE ROOF DECK DEAD LOAD WILL ALWAYS BE INCLUDED -THE COLLATERAL LOAD REPRESENTS ADDITIONAL LOAD THAT CAN BE SUPPORTED BY THE FRAME -BE SURE THE TOTAL ROOF DEAD LOAD FOR YOUR PROJECT IS LESS THAN OR EQUAL TO THE MAX DEAD LOAD SHOWN IN STEP 4 FOR YOUR SE VALUE

-Sds value used in calculation is the capped Sds (see design criteria) STEP 6: IDENTIFY THE FOUNDATION REQUIREMENTS FOR YOUR PROJECT -IDENTIFY SOIL CLASS FOR PROJECT SITE PER SITE SPECIFIC SOIL CONDITIONS -USE THIS TO SELECT CORRECT FOUNDATION SIZE ON FOUNDATION SHEET

STEP 7: SELECT MISCELLANEOUS OPTIONS FOR YOUR PROJECT -MAXIMUM CLEAR HEIGHT IS 12'-0"; (SEE "ARCHITECTURAL VIEWS" SHEET FOR REFERENCE) -MARK UP PC DRAWINGS WITH SIZE AND LOCATION OF CUTOUTS BEFORE SUBMITTING TO DSA

STEP 8: SELECT APPLICABLE SHEET INDEX FOR YOUR PROJECT -REFERENCE THE BASE FRAME (STEP 1) AND THE ROOF PANEL TYPE (STEP 2)

-IDENTIFY THE APPLICABLE SHEÈT INDEX

CONSTRUCTION.

STEP 9: INCLUDE APPLICABLE SHEETS WITH YOUR DSA SUBMITTAL -INCLUDE 'MISC DESIGN OPTIONS' SHEET FOR PROJECTS WITHOUT ELECTRICAL CUTOUTS OR GUTTERS

#### NOTICE OF DISCLAIMER FOR STRUCTURAL ENGINEERING RESPONSIBILITY

- 1. PER TITLE 24, PART 1, SECTION 4-316(e) OF THE CALIFORNIA CODE OF REGULATIONS, THIS NOTICE SHALL
- BE GIVEN TO DSA PRIOR TO THE APPROVAL OF PLANS AND SPECIFICATIONS. 2. FOR THE SITE SPECIFIC PROJECT, J. R. MILLER & ASSOCIATES IS NOT THE DESIGN PROFESSIONAL IN
- GENERAL RESPONSIBLE CHARGE. 3. FOR THE SITE SPECIFIC PROJECT, J.R. MILLER & ASSOCIATES' RESPONSIBILITY IS LIMITED TO THE PREPARATION OF THE PLANS AND SPECIFICATIONS FOR THE SHELTERS OF THIS PC ONLY.
- 4. STRUCTURAL OBSERVATION OF CONSTRUCTION IS SPECIFICALLY EXCLUDED FROM J.R. MILLER & ASSOCIATES' RESPONSIBILITY FOR THE SITE SPECIFIC PROJECT. 5. ALL CONSTRUCTION ACTIVITIES RELATED TO STRUCTURAL ENGINEERING SHALL BE DELEGATED TO A QUALIFIED

ENGINEER BY THE DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE CHARGE. THESE ACTIVITIES INCLUDE.

BUT ARE NOT LIMITED TO. APPROVAL OF INSPECTOR QUALIFICATIONS. STRUCTURAL OBSERVATION OF CONSTRUCTION, REVIEW OF INSPECTION REPORTS, AND SIGNING OFF OF THE VERIFIED REPORT FOR COMPLETED WORK. 6. J.R. MILLER & ASSOCIATES WILL BE RESPONSIBLE FOR RESPONDING TO QUESTIONS PERTAINING TO THE PLANS

AND SPECIFICATIONS FOR THE SHELTERS OF THIS PC WHICH ARISE DURING PLAN REVIEW AND

- 1. ALL WELDING SHALL COMPLY WITH AWS D1.1 SPECIFICATIONS AND SHALL BE DONE BY AWS QUALIFIED WELDERS CERTIFIED FOR THE TYPE OF WELDING TO BE PERFORMED AS REQUIRED BY DSA.
- 2. ALL WELDING SHALL BE DONE BY GAS METAL ARC PROCESS WITH E70XX ELECTRODES. FLUX CORE ARC WELD SHALL CONFORM TO CHARPY NOTCH TOUGHNESS RATING OF 20 ft-16 @ ( 0° F).
- 3. ALL WELDING SHALL BE DONE IN THE SHOP WITH REQUIRED INSPECTION, PRE-APPROVED BY DSA, TO ENSURE
- PROPER MATERIAL ID AND WELDING. 4. WELD FILLER METAL MANUFACTURER SHALL PROVIDE WRITTEN CERTIFICATION OF COMPLIANCE WITH CODE AND SPECIFIC ATIONS.

- 1. ALL BOLTS SHOWN ON THESE DRAWINGS ARE ASTM F3125 GRADE A325 HIGH STRENGTH BOLTS (UNO), WITH THE NUTS CONFORMING TO ASTM A-563
- 2. HIGH STRENGTH BOLTS SHALL BE VERIFIED AND INSPECTED PER CBC 1705A2.1
- 3. BEFORE ERECTING THE FRAME, VERIFY ALL BOLTS AND NUTS ARE CLEAN OF DEBRIS AND BURRS INCLUDING THE HARDWARE ALREADY FASTENED INSIDE THE MEMBERS. CHASING SOME OF THE BOLTS AND NUTS MAY BE
- 4. HARDENED STEEL WASHERS SHALL CONFORM TO ASTM F-436.
- 5. THE BOLTING INSTALLATION REQUIREMENTS OUTLINED BELOW ARE CRITICAL TO THE STRUCTURE'S DESIGN AND PERFORMANCE. THE INSTALLER IS REQUIRED TO COORDINATE THIS PHASE OF CONSTRUCTION WITH THE SPECIAL BOLTING INSPECTOR AND THE INSPECTOR OF RECORD PRIOR TO THE ERECTION OF THE FRAME. ALL BOLTS SHALL BE INSTALLED AND INSPECTED PER THE APPLICABLE VERSION OF AISC'S "SPECIFICATION FOR STRUCTURAL JOINTS
- USING HIGH-STRENGTH BOLTS", CBC 1705A.2.1; AISC 341-16 J7; AISC 360-16 N5.6. A)PRETENSIONED JOINTS MUST BE INSTALLED AND INSPECTED TO MEET ONE OF THE FOLLOWING REQUIREMENTS:
  - 1. TURN-OF-NUT PRETENSIONING
  - 2. CALIBRATED WRENCH PRETENSIONING 3. DIRECT-TENSION-INDICATOR PRETENSIONING (CONTRACTOR RESPONSIBLE FOR PURCHASE OF

- 1. ALLOWABLE SOIL PRESSURES ASSUME CLASS 5 SOIL CLASSIFICATION PER CBC TABLE 1806A, UNLESS NOTED
- 2. PER CBC SECTION 1803A.2, GEOTECHNICAL REPORTS ARE NOT REQUIRED FOR ONE-STORY LIGHT-STEEL FRAME BUILDINGS OF TYPE II CONSTRUCTION AND 4,000 SQUARE FOOT OR LESS IN FLOOR AREA AND NOT LOCATED WITHIN EARTHQUAKE FAULT ZONESOR SIESMIC HAZARD ZONES AS SHOWN ON THE MOST RECENT MAPS PUBLISHED BY THE CGS. ALLOWABLE FOUNDATION AND LATERAL SOIL PRESSURE VALUES MAY BE DETERMINED FROM TABLE 1806A.2.
- 3. FILL AND BACKFILL SHALL BE COMPACTED TO 95% OF MAX. DENSITY IN ACCORDANCE WITH ASTM TEST METHOD D-1557 OR AS RECOMMENDED BY THE GEO-TECH ENGINEER. FLOODING NOT PERMITTED.
- 4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SHORING, ETC. NECESSARY TO SUPPORT CUT AND/OR FILL BANKS DURING EXCAVATION, AND FORMING AND PLACEMENT OF CONCRETE.
- 5. MINIMUM SETBACK FROM TOE OF SLOPE ON AN ASCENDING SLOPE SHALL BE 15 FEET AND MINIMUM SETBACK FROM TOE OF SLOPE ON A DESCENDING SLOPE SHALL BE 40 FEET
- 6. PER CBC SECTION 1803A.6, GEOHAZARD REPORTS ARE NOT REQUIRED FOR ONE-STORY LIGHT-STEEL FRAME BUILDINGS OF TYPE II CONSTRUCTION AND 4,000 SQUARE FOOT OR LESS IN FLOOR AREA AND NOT LOCATED WITHIN EARTHQUAKE FAULT ZONESOR SIESMIC HAZARD ZONES AS SHOWN ON THE MOST RECENT MAPS PUBLISHED BY THE CGS.
- 7. GEOHAZRD REPORTS ARE TO COMPLY WITH DSA IR A-4 PER IR-7 SECTION 1.8
- 8. SITE SPECIFIC GEOTECHNICAL REPORT IS REQUIRED AT THE TIME OF SITE APPLICATION IS USING OTHER THAN
- 9. LATERAL BEARING HAS BEEN INCREASED PER CBC 1806A.3.4 & HAS BEEN DESIGNED FOR P-DELTA EFFECTS

#### 1. MIX DESIGN REQUIREMENTS: (NORMAL WEIGHT CONCRETE)

STRENGTH Pc (28 DAYS)	W/C RATIO (NON—AIR ENTRAINED)	W/C RATIO (AIR ENTRAINED)	SLUMP (±1")	UNIT WEIGHT (NORMAL WEIGHT)
4500 PSI	0.44	0.35	3"	150 PCF

SCHOOL DISTRICT:

- ENTRAINMENT FOR THESE CATEGORIES SHALL BE AS FOLLOWS: F0-0, F1-4.5, F2-6 3. AGGREGATES SHALL CONFORM TO THE ASTM C-33 WITH PROVEN SHRINKAGE CHARACTERISTICS OF LESS THAN 0.005.
- MAX AGGREGATE SIZE = 1". 4. CEMENT SHALL CONFORM TO ASTM C-150 (TYPE V) UNLESS NOTED OTHERWISE ON THE DRAWINGS.
- 5. CONCRETE SHALL BE MAINTAINED IN A MOIST CONDITION FOR A MINIMUM OF FIVE DAYS AFTER PLACEMENT.
- ALTERNATE METHODS WILL BE APPROVED IF SATISFACTORY PERFORMANCE CAN BE ASSURED. 6. CONCRETE SHALL NOT FREE FALL MORE THAN FIVE FEET.
- 7. CONCRETE DURABILITY SHALL BE PER CBC 1904A.1 & ACI 318-14 CHAPTER 19.
- 8. CONCRETE SHALL BE TESTED PER CBC 1903A, TABLE 1705A.3. AND ACI 318-14 SECTION 26.12.

#### STEP 10: IDENTIFY PROJECT NAME AND SCHOOL DISTRICT

CONSTRUCTION NOTES

TESTS AND INSPECTIONS FOR THE PROJECT.

SHALL COMPLY WITH ALL LOCAL ORDINANCES

PROJECT NAME:

			FRAME	DIMENSION	 S	
-			SUGO	GESTED		OTHER
STEP	FRAME WIDTH	[] 20'	<b>X</b> 30'	[] 40'		[] (40' MAX)
	FRAME LENGTH	[] 44'	<b>1</b> 64'	[]84'	[] 104'	[] (NO MAX)

2		ROOF PANEL	
STEP	ROOF PANEL TYPE	<b>⋈</b> м [] G [] S	
TEP 3	PRO	JECT SITE — Ss ACCELERATION (g)	
ST	<u>0.642</u>		

		Ss REGION		
			Ss REGIONS	MAX DEAD LOAD
4		X	0 < Ss <= 2.14	5 PSF
STEP			2.14 < Ss <= 2.50	5 PSF
S	DESC RIPTION		2.50 < Ss <= 2.75	5 PSF
			2.75 < Ss <= 3.00	4 PSF
			Ss > 3.73 MAX	3 PSF

		TOTAL ROOF DEAD LOA	AD
		DEAD LOAD	EXAMPLES
F 5	ROOF DECK	<u>1.1</u> PSF	M=1.1PSF; G=1.2PSF;S=1.3PSF (SEE STEP 2)
STE	COLLATERAL	<u>0</u> PSF	LIGHTING, ETC
	TOTAL	<u>1.1</u> PSF	ADD ROOF DECK AND COLLATERAL LOADS (MAX 5 PSF)

1. A DSA-CERTIFIED CLASS 3 PROJECT INSPECTOR IS REQUIRED FOR THIS PROJECT.

BEFORE PROCEEDING WITH THE WORK, (SECTION 4-317(c), PART 1, TITLE 24, CCR)

2. CHANGES TO THE APPROVED DRAWINGS AND SPECIFICATIONS SHALL BE MADE BY ADDENDA OR CONSTRUCTION CHANGE

CONTINUOUS INSPECTION OF WORK, THE DUTIES OF THE INSPECTOR ARE DEFINED IN SECTION 4-342, PART 1, TITLE 24, CCR.

RECONSTRUCTION IS TO BE IN ACCORDANCE WITH TITLE 24, CCR. SHOULD ANY EXISTING CONDITIONS SUCH AS DETERIORATION

FINISHED WORK WILL NOT COMPLY WITH TITLE 24, CCR, A CONSTRUCTION CHANGE DOCUMENT (CCD), OR A SEPARATE SET OF PLANS AND SPECIFICATIONS, DETAILING AND SPECIFYING THE REQUIRED WORK SHALL BE SUBMITTED TO AND APPROVED BY DSA

OR NON-COMPLYING CONSTRUCTION BE DISCOVERED WHICH IS NOT COVERED BY THE CONTRACT DOCUMENTS WHEREIN THE

6. GRADING PLANS, DRAINAGE IMPROVEMENTS, ROAD AND ACCESS REQUIREMENTS AND ENVIRONMENTAL HEALTH CONSIDERATIONS

4. A DSA ACCEPTED TESTING LABORATORY DIRECTLY EMPLOYED BY THE DISTRICT (OWNER) SHALL CONDUCT ALL THE REQUIRED

5. THE INTENT OF THESE DRAWINGS AND SPECIFICATIONS ARE THAT ALL THE WORK OF THE ALTERATION, REHABILITATION OR

 $\,$  3. A "DSA CERTIFIED" PROJECT INSPECTOR EMPLOYED BY THE DISTRICT (OWNER) AND APPROVED BY DSA SHALL PROVIDE

DOCUMENT (CCD) APPROVED BY DSA, AS REQUIRED BY SECTION 4-338, PART 1, TITLE 24, CCR.

#### REINFORCING STEEL:

- 1. REINFORCING STEEL SHALL BE DEFORMED STEEL CONFORMING TO THE REQUIREMENTS OF ASTM A-615,
  - GR 60: (#4 BARS AND LARGER)
- DETAILING, FABRICATION, AND ERECTION OF REINFORCING BARS SHALL CONFORM TO THE ACL "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCING CONCRETE STRUCTURES."
- 3. MIN. COVER FOR CAST-IN-PLACE CONCRETE SHALL BE AS FOLLOWS:
- A. CAST AGAINST EARTH ...... B. CAST AGAINST FORM BELOW GRADE .....2"
- C. FORMED SLABS (#11 BAR & SMALLER)......3/4"
- D. SLABS ON GRADE (FROM TOP OF SLAB)......1" 4. BARS SHALL BE CLEAN OF RUST, GREASE OR OTHER MATERIAL LIKELY TO IMPAIR BOND.
- BENDS SHALL BE MADE COLD.
- REINFORCING SHALL BE LAP SPLICED PER ACI 318-14 SECTION 25.5. 6. PRIOR TO PLACING OF CONCRETE, REINFORCING STEEL AND EMBEDDED ITEMS SHALL BE WELL SECURED IN POSITION.
- 7. WELDING OF REINFORCING IS NOT ALLOWED. 8. REINFORCING STEEL SHALL BE INSPECTED PER CBC 1705A.3.

#### POWDER-COAT FINISH SYSTEM:

GR 40: (#3 BARS)

- ALL BUILDINGS THAT HAVE A POWDER-COATED FINISH SHALL MEET THE FOLLOWING SPECIFICATIONS:
- 1. THE STEEL FRAME SHALL BE SHOT-BLASTED TO A NEAR WHITE CONDITION PER SSPC-10 SPECIFICATIONS. 2. THE STEEL SHALL BE WASHED IN A ZINC PHOSPHATE IN AN MINIMUM EIGHT STAGE ELECTRO DEPOSITION
- 3. IMMEDIATELY FOLLOWING PRE-TREATMENT THE STEEL SHALL BE TOTALLY IMMERSED IN A LIQUID EPOXY PRIMER(E-COAT) AND COATED TO A UNIFORM THICKNESS OF A MINIMUM OF 0.7 TO 0.9 MILS. THE E-COATING SHALL
- PROVIDE A MINIMUM OF 1000 HOURS OF SALT SPRAY CORROSION PROTECTION TO THE STEEL.
- 4. THE STEEL SHALL THEN HAVE A TGIC POLYESTER COLOR COAT APPLIED OVER THE E-COATED SURFACE. 5. THE COLOR COAT SHALL THEN HAVE A CLEAR TGIC COATING APPLIED TO SEAL IN THE COLOR COAT AND RESIST
- ULTRAVIOLET LIGHT, TO HELP PREVENT FADING. 6. THE FINISH THICKNESS OF THESE THREE APPLICATIONS SHALL BE A MINIMUM OF 8 TO 12 MILS

MINIMUM

MISC ELLANEOUS

7. ALL CARBON STEEL MEMBERS (COLUMNS, BEAMS, PLATES, ETC.) NOT POWDER-COATED SHALL BE PAINTED WITH PRIME COAT PER THE "AISC CODE OF STANDARD PRACTICE" AND THE "AISC SPECIFICATION SECTION M3"(UNLESS NOTED

OTHERWISE).	
ADDDEVIATIONS	

ABBREVI	ATIONS:		
ACI	AMERICAN CONCRETE INSTITUTE	MPH	MILES PER HOUR
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION	М	MULTI-RIB ROOF PANEL (MCELROY)
ASM	ASSEMBLY (INTERNAL REFERENCE)	NTS	NOT TO SCALE
ASTM	AMERICAN SOCIETY FOR TESTING AND MAT'LS	NO	NUMBER
AWS	AMERICAN WELDING SOCIETY	ОС	ON CENTER
CBC	CALIFORNIA BUILDING CODE	OSHA	OCCUPATIONAL HEALTH AND SAFETY ADMIN
CJP	COMPLETE JOINT PENETRATION	PCF	POUNDS PER CUBIC FOOT
CLR	CLEAR	PJ	PRETENSIONED JOINT
DEG	DEGREE	PLCS	PLACES
DIA	DIAMETER	PLT	PLATE
DIM	DIMENSION	PSF	POUNDS PER SQUARE FOOT
DSA	DIVISION OF THE STATE ARCHITECT	PSI	POUNDS PER SQUARE INCH
EQ	EQUAL	QTY	QUANTITY
FT	FEET	REF	REFERENCE
GA	GAGE	SQ	SQUARE
IN	INCHES	SS	STANDING SEAM ROOF PANEL (MCELROY)
KSI	KIPS PER SQUARE INCH	TYP	TYPIC AL
мах	MAXIMUM	UNO	UNLESS NOTED OTHERWISE

U.S. GEOLOGIC AL SURVEY

DIV. OF THE STATE ARCHITECT APP: 04-120013 PC REVIEWED FOR SS 🗹 FLS 🗹 ACS 🗹 CG 🗌 DATE: 08/06/2021

DRAWN BY

DATE

REV

REV DATE

**ARCHITECTS ENGINEERS** 

2700 SATURN STIBREA, CA 92821

. 714.524.1870 | F. 714.524.1875

**ANGEL** 

4/2/202

FOUNDATION REQUIREMENTS SOIL CLASS 5 (BEARING)-1500 PSF 📈 | SOIL CLASS 4 (BEARING)-2000 PSF [] SOIL CLASS 3 (BEARING)-3000 PSF [ SOIL CLASS 5 (LATERAL BEARING)-100 PSF SOIL CLASS 4 (LATERAL BEARING)-150 PSF SOIL CLASS 3 (LATERAL BEARING)-200 PSF

MISC ELLANEOUS DESIGN OPTIONS CLEAR HEIGHT []8' 🔀 10' []12' (12' MAX) ELECTRICAL CUTOUTS **⋈** YES [ ] NO GUTTERS 🔀 YES [ ] NO

	SHEET INDEX												
	BASE FRAME			RG 20			RG 30				RG 40		
	ROOF PANEL TYPE		М	G	S		М	G	S		М	G	S
	SELECT ONE		[]	[]	[]		[]	[]	[X]		[]	[]	[]
	GENERAL NOTES		LS1.0	LS1.0	LS1.0		LS1.0	LS1.0	LS1.0		LS1.0	LS1.0	LS1.0
	DSA 103 EXAMPLE		LS1.1	LS1.1	LS1.1		LS1.1	LS1.1	LS1.1		LS1.1	LS1.1	LS1.1
ω _	FOUNDATION PLAN		LS2.0	LS2.0	LS2.0		LS3.0	LS3.0	LS3.0		LS4.0	LS4.0	LS4.0
STEP	FRAMING PLAN		LS2.1	LS2.1	LS2.1		LS3.1	LS3.1	LS3.1		LS4.1	LS4.1	LS4.1
	FRAME CONNECTION DETAILS		LS2.1	LS2.1	LS2.1		LS3.1	LS3.1	LS3.1		LS4.2	LS4.2	LS4.2
	ROOFING LAYOUT & DETAILS		LS2.2	LS2.3	LS2.4		LS3.2	LS3.3	LS3.4		LS4.3	LS4.4	LS4.5
	MISC DESIGN OPTIONS		LS5.0	LS5.0	LS5.0		LS5.0	LS5.0	LS5.0		LS5.0	LS5.0	LS5.0

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616.396.0919

800.748.0985

616.396.0944 FX

ISTINCTIVE STEEL SHELTERS

PRE-CHECK (PC) DOCUMENT Code: 2019 CBC

PRINTED ON:

# A separate project application for construction is required.

SA 103-19: LISTING OF	STRUCTURAL TESTS & SPECIAL IN			SIS & SPECIAL IN	SPECTIO	VS (SOILS), 2019 CBC	DSA 103-19: LISTI Application Number:	School Name:		School District:	DSA 10 Application			
pplication Number: 4-000000	School Name: ICON Shelter Systems	School District: PC Submittal	Application Number: 04-000000 DSA File Number:	School Name: ICON Shelter Systems Increment Number:			School District: PC Submittal Date Created:	04-000000 DSA File Number:	ICON Shelter Systems Increment Number:		PC Submittal  Date Created:	04-00000 DSA File		
A File Number:	Increment Number:	Date Created: 2021-07-14 05:50:33					2021-07-14 05:50:33				2021-07-14 05:50:33			
												5. Te		
	2010	ono.	Geotechnical Reports:	: Project has a geotechnic	cal report, or CDs in	dicate soils	special inspection is required by GE	C. Compaction test	ting.	Test LO	* Under the supervision of the geotechnical engineer.  (Refer to specific items identified in the Appendix for exemptions where	□ a.		
<b>2019 CBC</b> IMPORTANT: This form is only a summary list of structural tests and some of the special inspections required for the project.			Geotechnical Reports: Project has a geotechnical report, or CDs indicate soils special inspection is required by GE  1. GENERAL: Table 1705A.6								soils testing may be conducted under the supervision of a geotechnical engineer or LOR's engineering manager. In such cases, the LOR's form DS. 291 shall satisfy the soil test reporting requirements for the exempt items	A		
Generally, the structural tests and special inspections noted on this form are those that will be performed by the Geotechnical Engineer of Record, Laboratory of Record, or Special Inspector. The actual complete test and inspection program must be performed as detailed on the DSA approved documents. The appendix at the bottom of this form identifies work NOT subject to DSA requirements for special inspection or structural testing. The project inspector is responsible for providing inspection of all facets of construction, including but			Test or Special Inspe	ection	Type Pe By	formed Co	de References and Notes				271 shall satisfy the soil test reporting requirements for the exempericins	□ <b>b</b> .		
			☑ a. Verify that:       Periodic       GE*       * By geotechnical engineer or his or her qualified representative.         • Site has been prepared properly prior to placement of       GE*       * By geotechnical engineer or his or her qualified representative.				4. CAST-IN-PLACE	E DEEP FOUNDATIONS (PIERS):	Table 1705A.8		□ c.			
not limited to, special insp	ections not listed on this form such as struc	tural wood framing, high-load wood diaphragms, cold-formed steel etc., per Title 24, Part 2, Chapter 17A (2019 CBC).	controlled fill and/or excavations for foundations.  • Foundation excavations are extended to proper depth				Test or Special Ins	Test or Special Inspection Type Performed Code References and Notes						
	-	is document are from the CBC, or California Building Code.	and have reached pro  • Materials below foot design bearing capaci	tings are adequate to achieve the				<ul> <li>☑ a. Inspect drilling of and accurate record</li> </ul>	operations and maintain complete	Continuous GI	E*  * By geotechnical engineer or his or her qualified representative.  (See Appendix for exemptions.)	e.		
TO COLUMNS	ica section and table references round in the	is document are from the CDC, of Camornia building Code.		·				□ <b>b.</b> Verify pier locati	ions, diameters, plumbness, bell cable), lengths and embedment into	Continuous GI		6.		
1. TYPE		2. PERFORMED BY	2. SOIL COMPACTION  Test or Special Inspe		Table 1705A,6  Type Pe	formed Co	de References and Notes		ible); record concrete or grout		(See Appendix for exemptions.)	□ a.		
ntinuous – Indicates that a cor	ntinuous special inspection is	GE – Indicates that the special inspection shall be performed by a registered geotechnical engineer or his or her authorized representative.	□ a. Perform classification	ion and testing of fill materials.	By Test	LOR* *U	nder the supervision of the geotechnical engineer.	C. Confirm adequat	te end strata bearing capacity.	Continuous GI	* By geotechnical engineer or his or her qualified representative. (See Appendix for exemptions.)			
uired		LOR – Indicates that the test or special inspection shall be performed by a testing		er materials, densities and		GE* * E	y geotechnical engineer or his or her qualified representative. (Refer to ecific items identified in the Appendix for exemptions where soils SI and	d. Concrete piers.		Provide tests and insp	ections per CONCRETE section below.	b.		
iodic – Indicates that a period	ic special inspection is required	laboratory accepted in the DSA Laboratory Evaluation and Acceptance (LEA) Program. See CAC Section 4-335.	during placement of f	es, placement and compaction fill.		tes	ting may be conducted under the supervision of a geotechnical gineer or LOR's engineering manager. In such cases, the LOR's form DSA					□ c.		
		PI – Indicates that the special inspection may be performed by a project inspector when specifically approved by DSA.				29 ite	shall satisfy the soil SI and test reporting requirements for the exempt ms.)							
t – Indicates that a test is requ	ired	SI – Indicates that the special inspection shall be performed by an appropriately						DGS DSA 103-19 (Revised 07/	/16/2020)			DGS DSA 1		
		qualified/approved special inspector.						DIVISION OF THE STATE ARCH	HITECT	DEPARTMENT OF GENE Page 3 of		NIA DIVISION O		
OSA 103-19 (Revised 07/16/2020)			DGS DSA 103-19 (Revised 07/16/	,	DEPARTMENT OF G	ENERAL SERVIO	ES STATE OF CALIFORNIA							
ON OF THE STATE ARCHITECT	DEPARTMENT OF G Page 1				Page :									
					ESTS & SPECIAL I	NSPECTIO	NS (Concrete), 2019 CBC		TING OF STRUCTURAL TEST -14 Sections 26.12 & 26.13	S & SPECIAL INS	PECTIONS (Concrete), 2019 CBC	<b>DSA 10</b> Table 170		
DSA 103-19: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (SOILS), 2019 CBC  Application Number: School Name: School District: 14-000000 ICON Shelter Systems PC Submittal			Table 1705A.3; ACI 318-7 Application Number: 04-000000	Table 1705A.3; ACI 318-14 Sections 26.12 & 26.13 Application Number: School Name: School District:				Application Number: 04-00000 DSA File Number:	School Name: ICON Shelter Systems Increment Number:		School District: PC Submittal Date Created:	Application 04-000000 DSA File N		
ile Number:	Increment Number:	Date Created: 2021-07-14 05:50:33	DSA File Number:	Increment Number:			<b>Date Created:</b> 2021-07-14 05:50:33	DSA FILE NUMBER:	mcrement number:		2021-07-14 05:50:33	_		
7. CAST-IN-PLACE CONCRE			17. STRUCTURAL  Material Verification and	L STEEL, COLD-FORMED STEEL AM	ND ALUMINUM USED FO	R STRUCTUR	AL PURPO	☑ b. Test high-stren	ngth bolts, nuts and washers.	Test L	OR Table 1705A.2.1 Item 1c, 2213A.1; RCSC 2014 Section 7.2; DSA IR 17-8.			
Test or Special Inspection	By	rformed Code References and Notes	Test or Special Ins		Type F	erformed	ode References and Notes	Inspection of High-Stre				Tes		
rial Verification and Testing a. Verify use of required des		SI Table 1705 A.3 Item 5, 1910 A.1.		ation of all materials and: ndicate material properties that cor	Periodic		able 1705A.2.1 Item 3a–3c. 2202A.1; AISI S100-16 Section A3.1 & A3.2, ISI S240-15 Section A3 & A5, AISI S220-15 Sections A4 & A6. * By special		'snug tight") connections.	Periodic	SI Table 1705A.2.1 Item 2a, 1705A.2.6, 2204A.2; AISC 360-16 J3.1, J3.2, M2.5 & N5.6; RCSC 2014 Section 9.1; DSA IR 17-9.	fille		
b. Identifiy, sample, and test	t reinforcing steel. Test	LOR 1910A.2; ACI 318-14 Section 26.6.1.2; DSA IR 17-10. (See Appendix for exemptions.)	with requirements  • Material sizes, typ		inpi		is pector or qualified technician when performed off-site.	d. Pretensioned a	and slip-critical connections.	*	SI Table 1705A.2.1 Items 2b & 2c, 1705A.2.6, 2204A.2; AISC 360-16 J3.1, J3.2, M2.5 & N5.6; RCSC 2014 Sections 9.2 & 9.3; DSA IR 17-9. * "Continuous" or "Periodic" depends on the tightening method used.	dec		
<b>c.</b> During concrete placeme for strength tests, perform s	nt, fabricate specimens Test  Jump and air content	LOR Table 1705 A.3 Item 6; ACI 318-14 Sections 26.5 & 26.12.	requirements.   b. Test unidentified	ed materials	Test	LOR 2	202A.1.							
tests, and determine the terconcrete.			c. Examine seam w	velds of HSS shapes	Periodic	SI	SA IR 17-3.	19. WELDI		D1.2 for Aluminum; A	<b>05A.2.1 Items 4 &amp; 5</b> ; AWS D1.1 and AWS D1.8 for structural steel; AWS AWS D1.3 for cold-formed steel; AWS D1.4 for reinforcing steel; DSA IR 17-	□ d. V		
<b>d</b> . Test concrete (f'c).	Test	LOR 1905A.1.15; ACI 318-14 Section 26.12.	<u> </u>	ıment steel fabrication per DSA-app	proved Periodic		ot applicable to cold-formed steel light-frame construction, except for usses (1705 A.2.4).	Verification of Material	ls, Equipment, Welders, etc.:	3 (See Appendix for e	exemptions.)	e.lr		
e. Batch plant inspection:	See Notes	SI Default of 'Continuous' per 1705A.3.3. If approved by DSA, batch plant inspection may be reduced to 'Periodic' subject to requirements in	18, HIGH-STRENG				asses (TroomET).	Test or Special Ir	nspection	Type Perfo	ormed Code References and Notes	23.		
		Section 1705A.3.3.1, or eliminated per 1705A.3.3.2. (See Appendix for exemptions.)		nd Testing of High-Strength Bolts	s, Nuts and Washers:				er material identification markings per listed on the DSA-approved documents		SI DSA IR 17-3.	Tes		
f. Welding of reinforcing ste	el. Provide special i	nspection per STEEL, Category 19.1(d) & (e) and/or 19.2(g) & (h) below.	Test or Special Ins	· 	,, [	y	ode References and Notes		er material manufacturer's certificate of	Periodic	SI DSA IR 17-3.	— a. A		
			certificates of com	ation markings and manufacturer's npliance conform to ASTM standard SA-approved documents.			able 1705A.2.1 Items 1a & 1b, 2202A.1; AISC 360-16 Section A3.3, J3.1, and N3.2; RCSC 2014 Section 1.5 & 2.1; DSA IR 17-8 & DSA IR 17-9.	c. Verify WPS, wel	lder qualifications and equipment.	Periodic	SI DSA IR 17-3.	□ <b>b</b> . T		
											1			
SA 103-19 (Revised 07/16/2020)			DGS DSA 103-19 (Revised 07/16/2020)  DIVISION OF THE STATE ARCHITECT DEPARTMENT OF GENERAL SERVICES STATE OF CALIFORNIA					DGS DSA 103-19 (Revised 07/16/2020)  DIVISION OF THE STATE ARCHITECT DEPARTMENT OF GENERAL SERVICES STATE OF CALIFORNIA						
N OF THE STATE ARCHITECT	DEPARTMENT OF ( Page	EENERAL SERVICES STATE OF CALIFORN 5 of 11	IIA DIVISION OF THE STATE ANCI	AIIECI		e 6 of 11	ICES STATE OF CALIFORNIA			Page 7 o	f11			
		SPECTIONS (Steel and Aluminum), 2019 CBC	DSA 103-19: LIST Application Number:	TING OF STRUCTURAL T School Name:	ESTS & SPECIAL	NSPECTI	DNS(SIGNATURE), 2019 CBC School District:	DSA 103-19: LIST Application Number: 04-000000	OF REQUIRED VERIFIED REI School Name: ICON Shelter Systems	PORTS, CBC 2019	School District: PC Submittal	-		
.1, Table 1705A.2.1; AISC 3 tion Number: 00	803-16, AISC 341-16, AISC 358-16, AISC 360-16 School Name: ICON Shelter Systems	AISI S100-16  School District: PC Submittal	04-000000 DSA File Number:	ICON Shelter System Increment Number			PC Submittal  Date Created: 2021-07-14 05:50:33	DSA File Number:	Increment Number:		Date Created: 2021-07-14 05:50:33	_		
Number:	Increment Number:	Date Created: 2021-07-14 05:50:33												
			Name of Architect or Engineer in general responsible charge:					Soils Testing and Inspection: Geotechnical Verified Report Form DSA 293  ———————————————————————————————————						
23. ANCHOR BOLTS AND ANCHOR RODS:				r (When structural design has been deleg	gated):				2. Structural Testing and Inspection: Laboratory Verified Report Form DSA 291					
est or Special Inspection		rformed   Code References and Notes	_					<sup>3.</sup> DSA 292			independently contracting SI, Special Inspection Verified Report Form	_		
, Anchor Bolts and Anchor R	dods Test	LOR Sample and test anchor bolts and anchor rods not readily identifiable poprocedures noted in DSA IR 17-11.	er Signature of Architect or Stru	actural Engineer:	Date:			4. High-Strength Bo Report Form DS		y Verified Report Form	DSA 291, or, for independently contracting SI, Special Inspection Verified	_		
o. Threaded rod not used for	foundation anchorage. Test	LOR Sample and test threaded rods not readily identifiable per procedures noted in DSA IR 17-11.	Note: Tr. C. W. C. C.	SA alactronic mark	ication eterno and the second	Dcv	ands against using secured electronic and distributed in							
			NOTE: To facilitate DS	A electronic mark-ups and identifi	ication stamp application	, DSA recomn	ends against using secured electronic or digital signatures.  DSA STAMP							
							DOMOTRIVIE							
												-		
			DGS DSA 103 10 (Partical 03	7/16/2020)				DGC DCA 100 10 /D 1 15-	/16/2020)					
A 103-19 (Revised 07/16/2020) N OF THE STATE ARCHITECT	DEPARTMENT OF (	SENERAL SERVICES STATE OF CALIFO	DGS DSA 103-19 (Revised 07  DIVISION OF THE STATE ARC		DEPARTMENT O	F GENERAL SER ge 10 of 11	/ICES STATE OF CALIFORNIA	DGS DSA 103-19 (Revised 07/ DIVISION OF THE STATE ARCH		DEPARTMENT OF GENER Page 11 of		A.		

Application Number: 04-000000 ICON Shelter Systems Increment Number: DSA File Number: Date Created: 5. RETAINING WALLS: Type Performed Code References and Notes Test or Special Inspection Continuous GE\* 1705A.6.1. \* By geotechnical engineer or his or her qualified representative. (See Section 2 above). a. Placement, compaction and inspection of backfill. □ b. Placement of soil reinforcement and/or drainage Continuous | GE\* | \* By geotechnical engineer or his or her qualified representative Continuous

# By geotechnical engineer or his or her qualified representative See DSA IR 16-3. c. Segmental retaining walls; inspect placement of units, dowels, connectors, etc. d. Concrete retaining walls. Provide tests and inspections per CONCRETE section below. e. Masonry retaining walls. Provide tests and inspections per MASONRY section below. 6. OTHER SOIL Test or Special Inspection Type Performed Code References and Notes a. Soil Improvements Test GE\* Submit a comprehensive report documenting final soil improvements constructed, construction observation and the results of the confirmation testing and analysis to CGS for final acceptance. \* By geotechnical engineer or his or her qualified representative b. Inspection of Soil Improvements Continuous GE\* \* By geotechnical engineer or his or her qualified representative DGS DSA 103-19 (Revised 07/16/2020) INIA DIVISION OF THE STATE ARCHITECT DEPARTMENT OF GENERAL SERVICES STATE OF CALIFORNIA Page 4 of 11 DSA 103-19: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (Concrete), 2019 CBC Table 1705A.3; ACI 318-14 Sections 26.12 & 26.13
Application Number: School Name: PC Submittal ICON Shelter Systems Date Created: 2021-07-14 05:50:33 DSA File Number: Increment Number: 19.1 SHOP WELDING: Type Performed Code References and Notes Test or Special Inspection a. Inspect groove welds, multi-pass fillet welds, single pass | Continuous fillet welds > 5/16", plug and slot welds. applicable); DSA IR 17-3. ✓ b. Inspect single-pass fillet welds ≤ 5/16", floor and roof
 Periodic
 SI
 1705A.2.2, Table 1705A.2.1 Items 5a.5 & 5a.6; AISC 360-16 (and AISC) 341-16 as applicable); DSA IR 17-3. deck welds. c. Inspect welding of stairs and railing systems. Periodic SI 1705A.2.1; AISC 360-16 (and AISC 341-16 as applicable); AWS D1.1 & D1.3; d. Verification of reinforcing steel weldability Periodic SI 1705A.3.1; AWS D1.4; DSA IR 17-3. Verify carbon equivalent reported on other than ASTM A706. Continuous SI Table 1705A.2.1 Item 5b, 1705A.3.1, Table 1705A.3 Item 2, 1903A.8; AWS D1.4; DSA IR 17-3. e. Inspect welding of reinforcing steel. 23. ANCHOR BOLTS AND ANCHOR RODS: Type Performed Code References and Notes Test or Special Inspection ☑ a. Anchor Bolts and Anchor Rods LOR Sample and test anchor bolts and anchor rods not readily identifiable per procedures noted in DSA IR 17-11. LOR Sample and test threaded rods not readily identifiable per procedures noted in DSA IR 17-11. **b.** Threaded rod not used for foundation anchorage.

DEPARTMENT OF GENERAL SERVICES

STATE OF CALIFORNIA

DSA 103-19: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (SOILS), 2019 CBC

DIV. OF THE STATE ARCHITECT APP: 04-120013 PC REVIEWED FOR SS 🗹 FLS 🗹 ACS 🗹 CG 🗌 DATE: 08/06/2021

RH/DSA-PC

ANGEL

4/2/2021

ARCHITECTS ENGINEERS

2700 SATURN ST I BREA, CA 92821

T. 714.524.1870 | F. 714.524.1875 WWW.JRMA.COM

ICON STD

DRAWN BY

DATE

REV

REV DATE

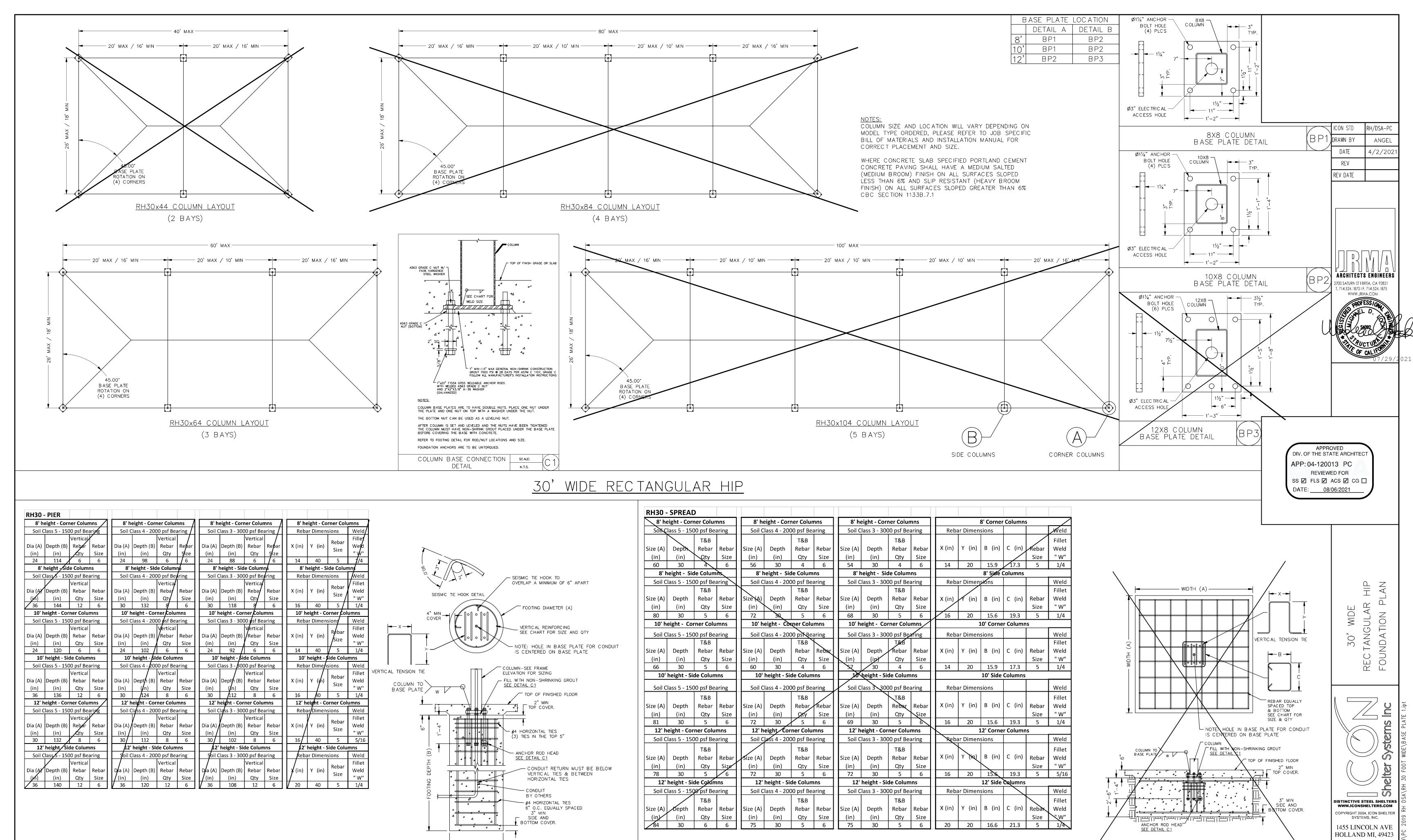
103 DSA

DISTINCTIVE STEEL SHELTERS COPYRIGHT 2004, ICON SHELTER 1455 LINCOLN AVE

HOLLAND MI, 49423

616.396.0919 800.748.0985 616.396.0944 FX

PRE-CHECK (PC) DOCUMENT Code: 2019 CBC A separate project application for construction is required.



FOOTING DIAMETER (A)

SEE DETAILS BP1. BP2 OR BP3 FOR ANCHOR BOLT PATTERNS

BP1 & BP2 ARE (4) BOLT PATTERN WHILE B3 IS A (6) BOLT

PRE-CHECK (PC) DOCUMENT
Code: 2019 CBC
A separate project application for construction is required.

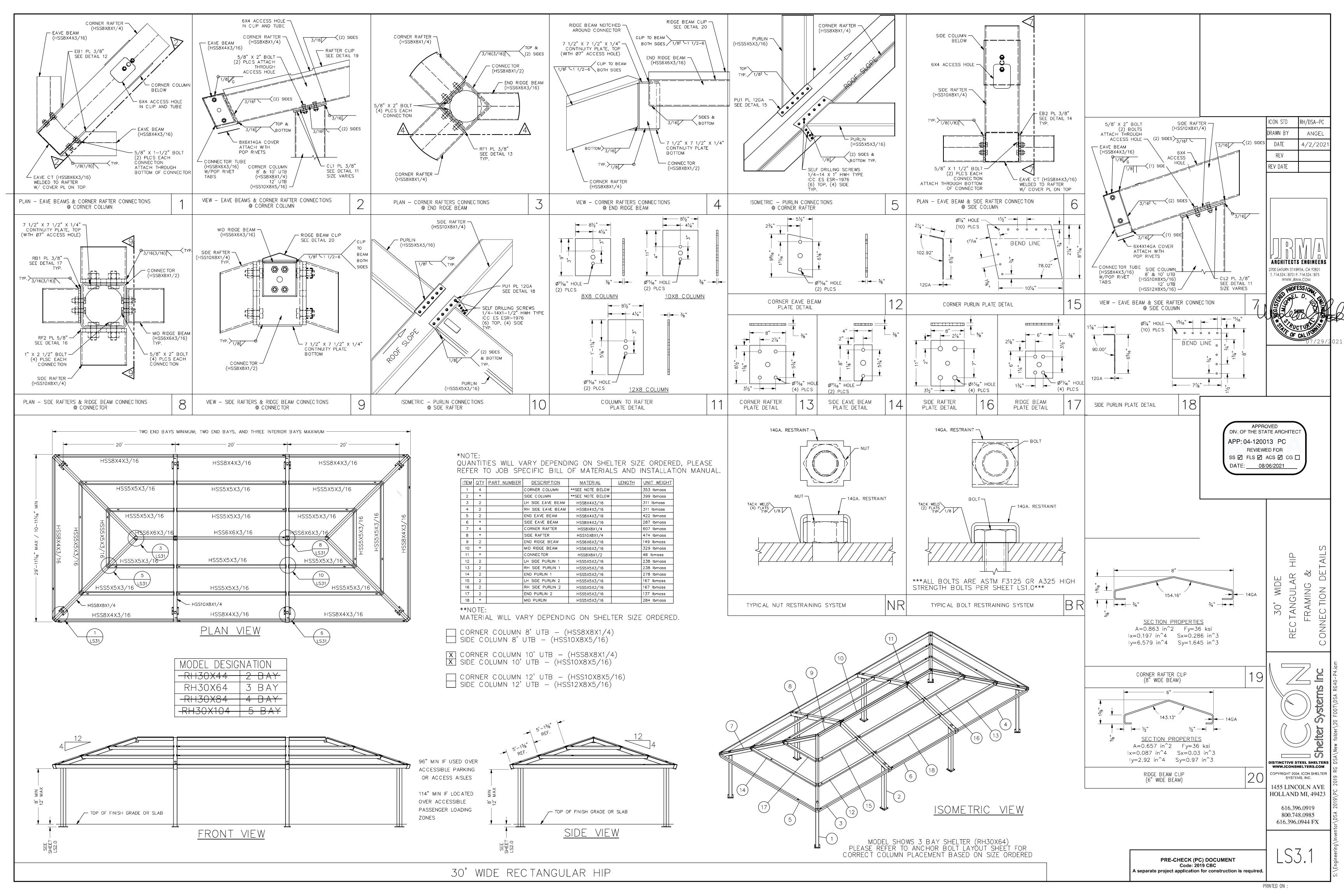
DETAILS BP1. BP2 OR BP3 FOR ANCHOR BOLT PAT**N**ERNS

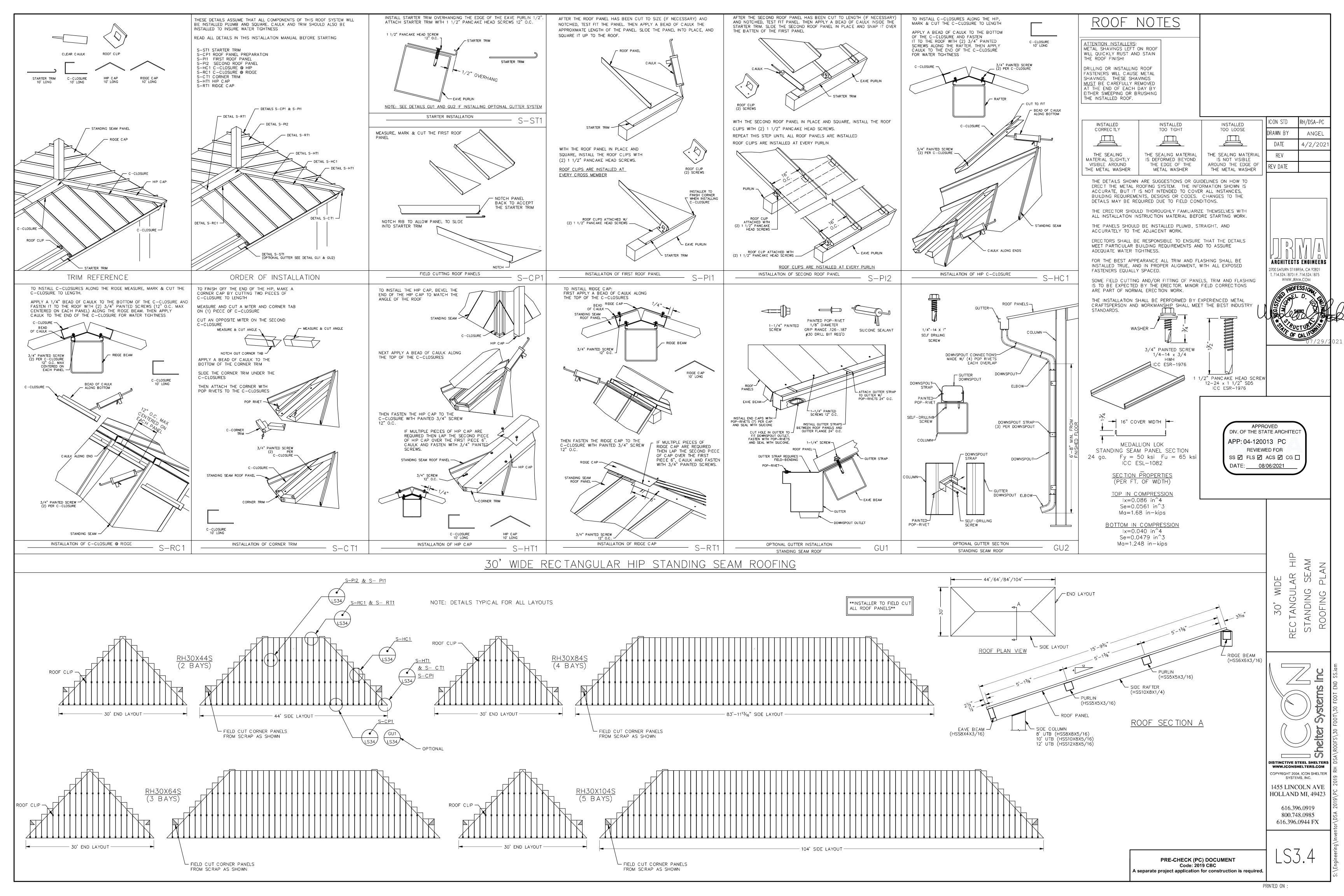
 $t ar{eta}$ P1 & BP2 are (4) bolt pattern while b3 is a (6) b $\delta$ L^

PRINTED ON :

616.396.0919 800.748.0985

616.396.0944 FX



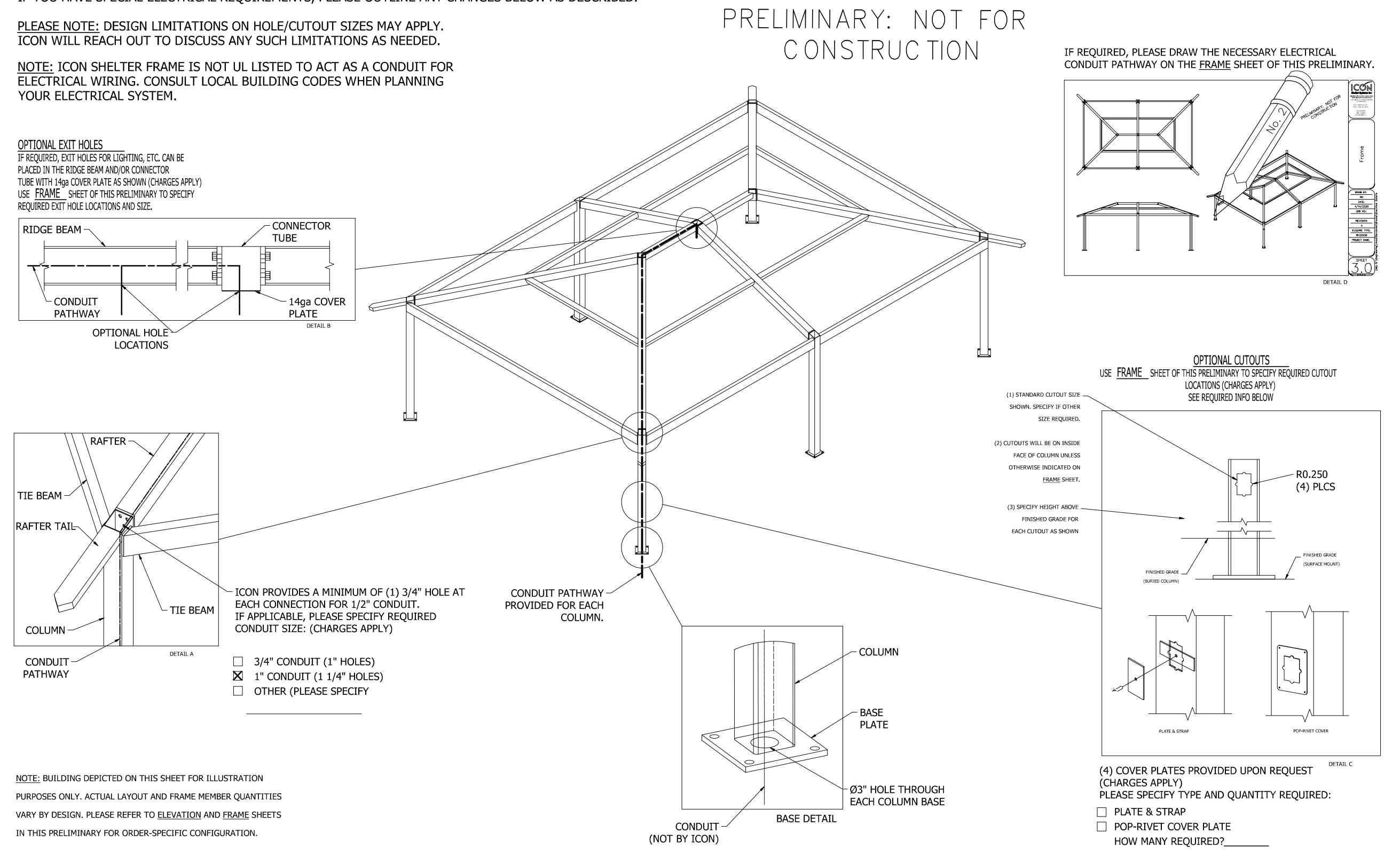


### ELECTRICAL INFORMATION - RECTANGULAR HIP

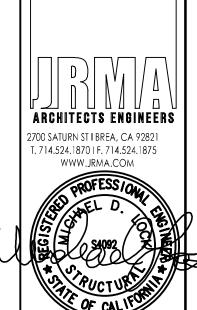
ICON'S STANDARD ELECTRICAL IS DESIGNED TO ACCOMMODATE Ø1/2" CONDUIT WITH A Ø3" INLET HOLE ON THE BOTTOM OF EACH COLUMN. THE CONDUIT PATHWAY RUNS THROUGH THE COLUMN, RAFTER, AND RIDGE BEAM THROUGH ALL BOLTED CONNECTIONS AS SHOWN. IF YOU HAVE SPECIAL ELECTRICAL REQUIREMENTS, PLEASE OUTLINE ANY CHANGES BELOW AS DESCRIBED.

2. ELECTRICAL EXIT HOLES (DETAIL B) 3. ELECTRICAL ACCESS & COVER PLATES (DETAIL C) 4. ELECTRICAL CONDUIT PATHWAY (DETAIL D)

1. CONDUIT HOLE SIZE (DETAIL A)

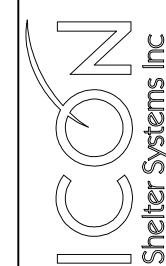


REV DATE



APPROVED DIV. OF THE STATE ARCHITEC APP: 04-120013 PC SS 🗹 FLS 🗹 ACS 🗹 CG 🗌

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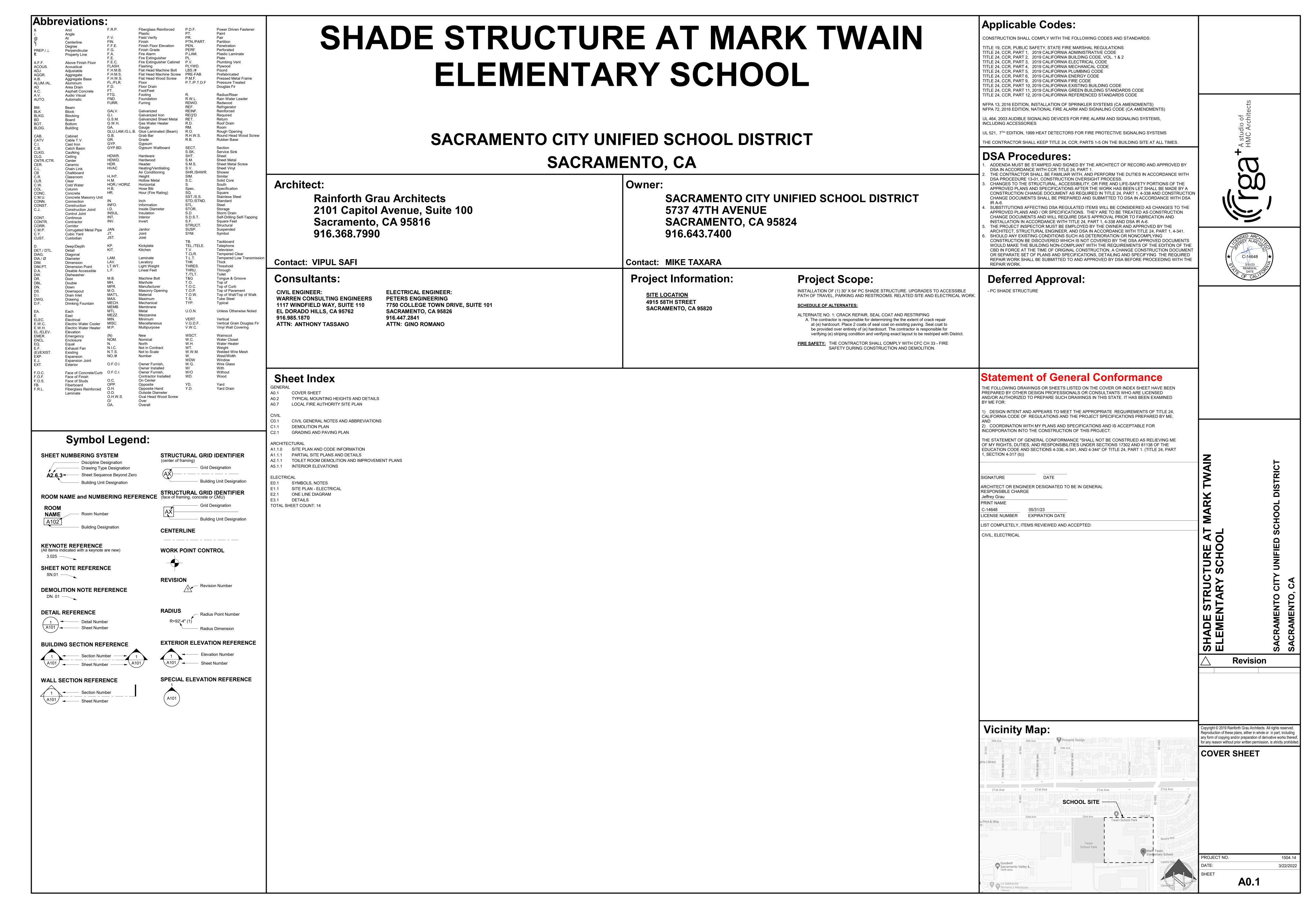


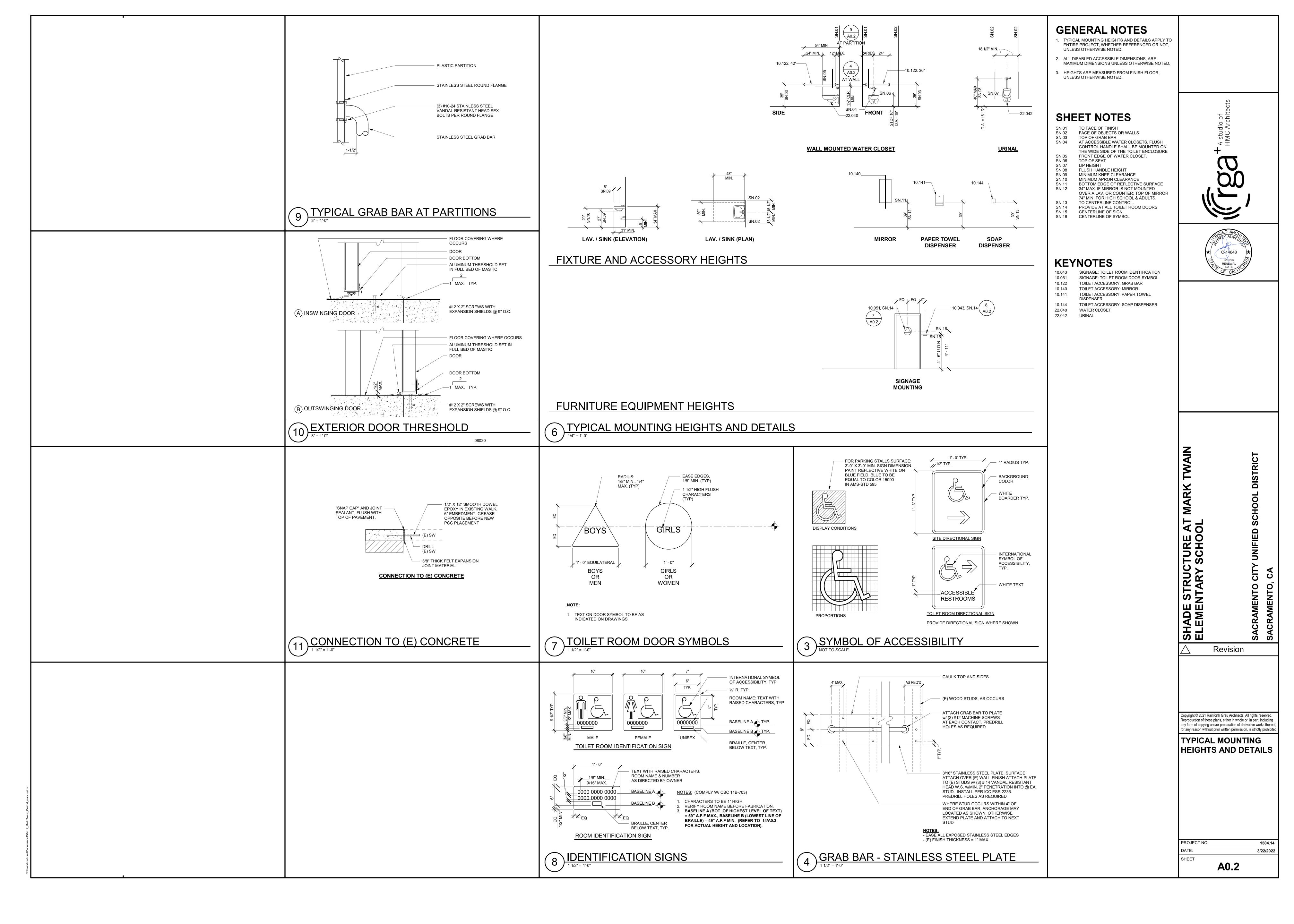
COPYRIGHT 2004, ICON SHELTER 1455 LINCOLN AVE HOLLAND MI, 49423 616.396.0919

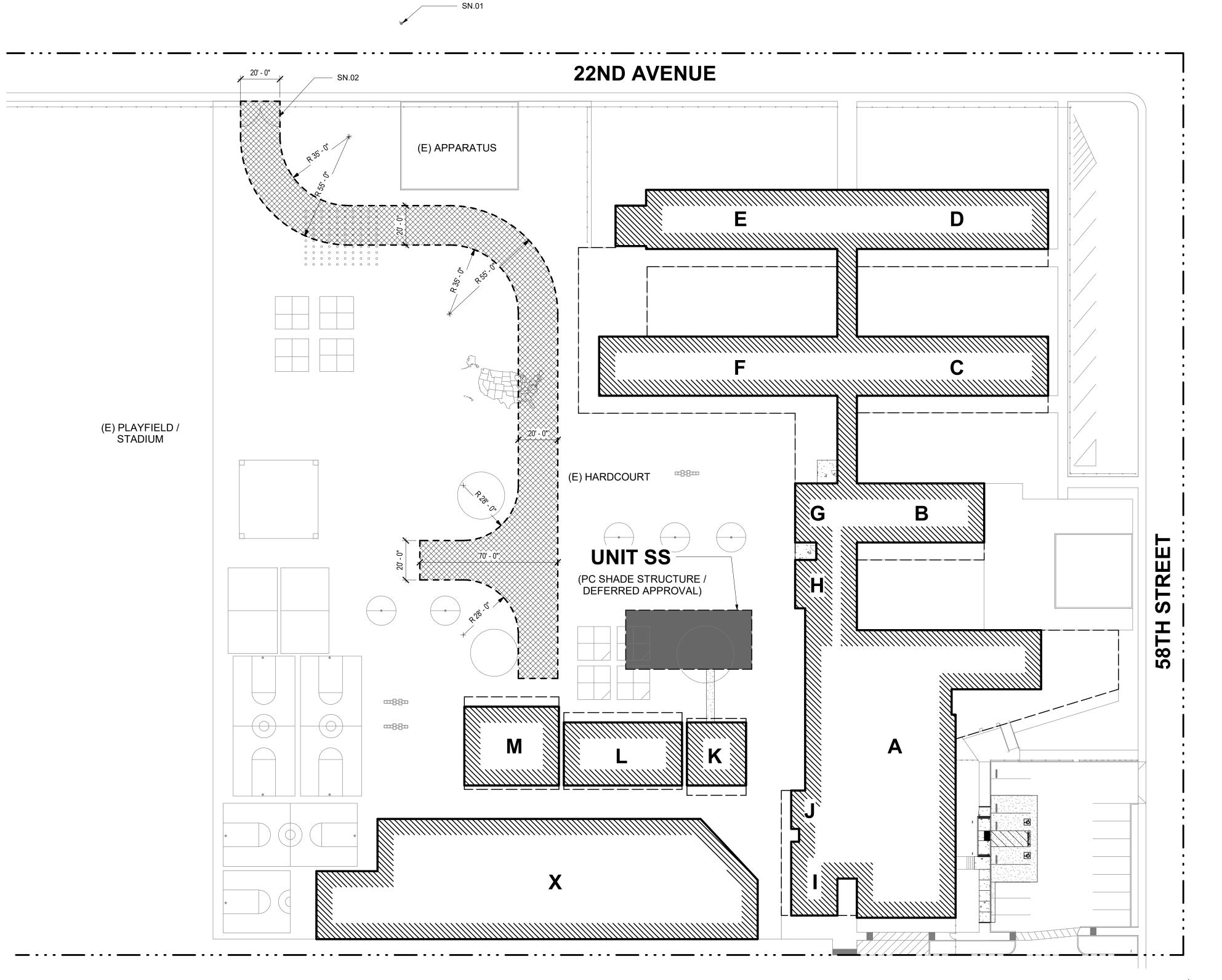
> 800.748.0985 616.396.0944 FX

PRE-CHECK (PC) DOCUMENT Code: 2019 CBC

A separate project application for construction is required.







1 LOCAL FIRE AUTHORITY SITE PLAN

**DSA-810** FIRE & LIFE SAFETY SITE CONDITIONS SUBMITTAL **LEGEND** PROJECT INFORMAITION - - - - PROPERTY LINE School District: SACRAMENTO UNIFED SCHOOL DISTRICT Project name / school: MARK TWAIN SHADE STRUCTURE — UNIT DESIGNATION 4914 58TH STREET, SACRAMENTO, CA 95820 SHADE STRUCTURE ALTERNATE ACCEPTED FIRE & LIFE SAFTEY INFORMATION UNIT DESIGNATION Has a fire hydrant flow test been preformed within the past 12 months? EXISTING BUILDINGS (If yes, provide a copy of the test data) Was the fire hydrant water flow test performed as part of this LFA review? Yes CONCRETE WALK / PAVING 3. Is the project located within a designated fire hazard serverity zone as established by Cal-Fire? (If yes, indicate fire hazard zone classification below) ASPHALT CONCRETE PAVING Refer to the following for fire hazard zone locations: www.fire.ca.gov/fire\_prevention/fire\_prevention\_wildland \_zones\_maps (E) EMERGENCY ACCESS LANE Wildland Interface Area (WIFA) (If any designations are checked, project design must meet the requirements of CBC Chapter 7A) (E) CHAIN LINK FENCE **CONDITION MEANS AND METHODS RESOLUTION** ALTERNATE ACCEPTED (E) FIRE HYDRANT (NTS) 4. Emergency vehicle access roadways do not meet CFC requirements 4a. **Acceptable Alternative:** Emergency vehicle and personel access SHEET NOTES as proposed by the architect is acceptable for providing fire suppression and protection of life and property SN.01 (E) FIRE HYDRANT Fire Hydrants: Number and spacing does not meet CFC requirements SN.02 (E) PR. 10' - 0" WIDE GATES WITH KNOX LOCK BOX 5a. **Acceptable Alternative:** Number of fire hydrants and spacing as proposed by the architect is acceptable for fire suppression and protection of life and property. 6. **Fire Hydrants:** Water flow and pressure are less than CFC minimum. 6a. **Acceptable Alternative:** The available flow and pressure is acceptable for providing fire suppression and protection of life and property. Location of fire department connection(s) serving fire sprinkler system **BUILDING DESIGNATIONS** or standpipe system does not meet CFC requirements. 7a. **Acceptable Alternative:** The location of fire department connection serving the fire sprinkler system and/or standpipe system is acceptable UNIT A - ADMINISTRATION AND for providing fire suppression and protection of life and property. **School District Acceptance of Acceptable Design Alternates MULTI-PURPOSE** By signing this form, the school district acknowledges and accepts the proposed design as an alternative to California Building Code (CBC) and California Fire Code (CFC) minimum requirements as indicated by one of more of the conditions indicated at items 4a, 5a, 6a, or 7a, for providing fire and life safety protection of life and property. UNIT B - CLASSROOMS Accepted by: UNIT C - CLASSROOMS Signature: Date: UNIT D - CLASSROOMS LOCAL FIRE AUTHORITY (LFA) INFORMATION UNIT E - CLASSROOMS LFA Agency Name: UNIT F - CLASSROOMS LFA Review Official: UNIT G - TOILET ROOMS Work Phone: UNIT H - MECH/ELECTRICAL Work Email: \_ Date:\_\_\_ LFA Reviewer's Signature: UNITS - TEACHER FACILITIES UNIT K - CLASSROOMS UNIT L - CLASSROOMS UNIT M - CLASSROOMS UNIT X - BUILDING BELONGS TO SEPARATE SITE

SHADE STRUCTURE ELEMENTARY SCHO Revision

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Reproduction of these plans, either in whole or in part, including any form of copying and/or preparation of derivative works thereof, for any reason without prior written permission, is strictly prohibited. LOCAL FIRE AUTHORITY SITE PLAN

SEE OTHER SHEETS FOR CONSTRUCTION

THIS PLAN INCLUDES INFORMATION FOR LOCAL FIRE AUTHORITY APPROVAL ONLY. REFER TO OTHER SHEETS FOR SITE CONSTRUCTION DETAILS.

PROJECT NO. 3/22/2022 SHEET A0.7

= STORM DRAIN MANHOLE = STORM DRAIN CLEANOUT = DROP INLET = AREA DRAIN

= RAIN WATER LEADER = DOWNSPOUT SANITARY SEWER LINE

(SIZE & DIRECTION OF FLOW) \_\_\_\_\_12~SS \_ = SANITARY SEWER LINE (RECORD INFORMATION) \_\_\_\_\_\_\_\_\_\_\_\_\_\_\_\_ = SANITARY SEWER LINE (UNDERGROUND LOCATING) = SANITARY SEWER MANHOLE

= SANITARY SEWER CLEANOU = WATER LINE (SIZE INDICATED) - -W - -W = WATER LINE (RECORD INFORMATION)

-W- - W = WATER LINE (UNDERGROUND LOCATING)= WATER MANHOLE = WATER VALVE

= WATER METER

= WATER BOX = IRRIGATION CONTROL VALVE

= FIRE HYDRANT = BACKFLOW PREVENTER = SPRINKLER = HOSE BIBB

-OH-E- = OVERHEAD ELECTRIC LINE ---E = UNDERGROUND ELECTRIC LINE ---E---= UNDERGROUND ELECTRIC LINE

— — E — — = UNDERGROUND ELECTRIC LINE (UNDERGROUND LOCATING)

= ELECTRIC MANHOLE = UTILITY POLE (WITH GUY WIRE)

= ELECTRIC METER = ELECTRIC BOX = STREET LIGHTING BOX

 $\square$   $\square$   $\square$   $\square$   $\square$  = LIGHT STANDARD □ □ □ □ □ = SIGNAL LIGHT = FLOOD LIGHT = ELECTRICAL OUTLET

---G--- = GAS LINE (RECORD INFORMATION)--G--= GAS LINE (UNDERGROUND LOCATING)

= GAS MANHOLE = GAS VALVE = GAS METER

---- G ---- = GAS LINE (SIZE INDICATED)

--- T --- = TELEPHONE LINE---T---= TELEPHONE LINE (RECORD INFORMATION) -- T -- = TELEPHONE LINE (UNDERGROUND LOCATING)

= STORM DRAIN BOX = TRAFFIC SIGNAL BOX

TBM LIST

NUMBER DESCRIPTION NORTHING EASTING ELEV CPS CHISELED "+" 8109.58 10070.07 28.46 CPS CHISELED "+" 8193.02 10027.62 28.73 8235.09 9833.67 31.24 11 CPS MAG NAIL 8284.54 9606.66 30.00 12 CPS MAG NAIL 13 CPS MAG NAIL 8296.89 9831.13 31.52 18 CPS CHISELED "+" 8496.69 9596.85 29.52 19 CPS MAG NAIL 7913.15 10003.43 29.11 27 CPS CHISELED "+" 7903.10 9892.93 30.44 29 CPS CHISELED "+" 7898.54 9688.11 30.36 40 CPS CHISELED "+" 8208.18 9853.02 31.82 41 CPS CHISELED "+" 8308.27 9869.43 31.74 42 CPS CHISELED "+" 8306.75 9749.90 31.59 43 CPS CHISELED "+" 8381.71 9745.70 31.66 44 CPS CHISELED "+" 8437.79 9549.21 29.76 45 CPS CHISELED "+" 8210.96 9551.62 29.24 46 CPS CHISELED "+" 8076.17 9556.66 29.20 48 CPS CHISELED "+" 8009.88 9603.58 29.51 49 CPS CHISELED "+" 7995.03 10007.46 29.50

50 CPS CHISELED "+" 7988.32 9880.66 29.84

### CIVIL ABBREVIATIONS AND LEGEND

CATV

COMM

CONC.

CONST.

CO

CR

DWG

**ESMT** 

**HDPE** 

SCH

STD

W/

W/O

GATE VALVE

HEADER BOARD

PIPE INVERT ELEVATION

PORTLAND CEMENT CONCRETE

JOINT UTILITY POLE

HIGH DENSITY POLYETHYLENE PIPE

HOSE BIBB

HIGH POINT

LINEAL FEET

**MOWSTRIP** 

OVERHEAD

LEFT

LIP OF GUTTER

NOT TO SCALE

PLANTER DRAIN

PROPERTY LINE

RIGHT OF WAY

STORM DRAIN

SANITARY SEWER

SCHEDULE

**STANDARD** 

SIDEWALK

UTILITY

WATER

WITHOUT

WATER VALVE

WITH

TELEPHONE

TOP OF CURB

TRENCH DRAIN

TELEPHONE POLE

TOP OF SEAT WALL

VITRIFIED CLAY PIPE

UNDERGROUND

POWER POLE

POST INDICATOR VALVE

PUBLIC UTILITY EASEMENT

STORM DRAIN MANHOLE

SANITARY SEWER MANHOLE

TRENCH DRAIN CATCH BASIN

TOP OF RAMP ELEVATION

TOP OF WALK ELEVATION

UNLESS OTHERWISE NOTED

TOP OF RETAINING WALL

SUBGRADE ELEVATION

REINFORCED CONCRETE PIPE

MANHOLE RIM ELEVATION (SOLID COVER)

REDUCED PRESSURE BACKFLOW PREVENTER

POLYVINYL CHLORIDE

<u>LEGEND</u> **ABBREVIATIONS** NOTE: NOT ALL SYMBOLS MAY NOTE: NOT ALL ABBREVIATIONS BE USED ON THESE PLANS. MAY BE USED ON THESE PLANS. PROPOSED GRADING & DRAINAGE SYMBOLS: AGGREGATE BASE ASPHALTIC CONCRETE 8" SD STORM DRAIN LINE AREA DRAIN (SIZE AND FLOW SHOWN) ASSESSOR'S PARCEL NUMBER AIR RELEASE VALVE STORM DRAIN MANHOLE AGGREGATE SUB-BASE BLOW-OFF VALVE (SDMH) **BUTTERFLY VALVE** BACK OF WALK ——— CATCH BASIN (CB) **CENTERLINE** CATCH BASIN ——— DROP INLET (DI) CLASS CORRUGATED METAL PIPE —— AREA DRAIN (AD) CABLE TELEVISION **CLEANOUT** PLANTER DRAIN (PD) OR COMMUNICATION FLOOR DRAIN (FD) CONCRETE CONSTRUCT STORM DRAIN CLEANOUT CURB RETURN CONCRETE SURFACE ELEVATION DOUBLE CHECK VALVE DOUBLE DETECTOR CHECK VALVE FINISHED FLOOR ELEVATION DECOMPOSED GRANITE DROP INLET BUILDING PAD ELEVATION PAD = 99.33DIAMETER DUCTILE IRON PIPE CONCRETE SIDEWALK DRAWING DOWNSPOUT GRADED DIRECTION FOR ELECTRIC DRAINAGE FLOW EDGE OF PAVEMENT EASEMENT  $\longrightarrow$ ---- SWALE **EXISTING** FIRE SERVICE LINE FIRE DEPARTMENT CONNECTION FLOWLINE TREE TO BE REMOVED SANITARY SEWER FORCE MAIN FINISHED FLOOR ELEVATION RETAINING WALL FIRE HYDRANT GRATE ELEVATION GRADE ELEVATION

PROPOSED SANITARY SEWER SYMBOLS: 6" SS SANITARY SEWER LINE (SIZE AND FLOW SHOWN) SANITARY SEWER MANHOLE (SSMH) SEWER CLEANOUT FLUSHER BRANCH

### PROPOSED WATER SYMBOLS:

8" RW RECLAIMED WATER LINE & SIZE 8" IRR IRRIGATION SERVICE LINE & SIZE 8" NP NON POTABLE WATER LINE & SIZE 8" SP FIRE SPRINKLER SERVICE LINE & SIZE <del>───</del> GATE VALVE ———M——— WATER METER

→ → → FH FIRE HYDRANT ASSEMBLY FIRE DEPARTMENT CONNECTION DETECTOR CHECK VALVE DOUBLE DETECTOR CHECK VALVE REDUCED PRESSURE BACKFLOW PREVENTER BUTTERFLY VALVE AIR RELEASE VALVE + SIZE BLOW-OFF VALVE + SIZE POST INDICATOR VALVE

### **DEMOLITION GENERAL NOTES**

- IN THE EVENT THAT ANY UNUSUAL CONDITIONS NOT COVERED BY THE GEOTECHNICAL INVESTIGATION REPORT OR ARE ENCOUNTERED DURING GRADING OPERATIONS THE GEOTECHNICAL ENGINEER AND THE ARCHITECT SHALL BE IMMEDIATELY NOTIFIED FOR DIRECTIONS.
- 2. NO BURNING OR BLASTING SHALL BE PERMITTED.
- ADDITIONAL DEMOLITION INFORMATION MAY BE SHOWN ON THE GRADING, DRAINAGE, AND UTILITY PLANS, AND THOSE PLANS PREPARED BY OTHER DISCIPLINES FOR THIS PROJECT.
- 4. ALL DEMOLISHED ITEMS SHALL BE DISPOSED OF OFFSITE AT A SUITABLE, LEGAL, DUMP SITE OR OTHER FACILITY.
- 5. ALL DISPOSED OF MATERIALS SHALL BE RECYCLED IF POSSIBLE
- 6. THE TYPES, LOCATIONS, SIZES AND/OR DEPTHS OF EXISTING UNDERGROUND UTILITIES AS SHOWN IN THESE PLANS WERE OBTAINED FROM SOURCES OF VARYING RELIABILITY. THE CONTRACTOR IS CAUTIONED THAT ONLY ACTUAL EXCAVATION WILL REVEAL THE TYPES, EXTENT, SIZES, LOCATIONS, AND DEPTHS OF SUCH UNDERGROUND UTILITIES. A REASONABLE EFFORT HAS BEEN MADE TO LOCATE AND DELINEATE ALL KNOWN UNDERGROUND UTILITIES. HOWEVER, WARREN CONSULTING ENGINEERS CAN ASSUME NO RESPONSIBILITY FOR THE COMPLETENESS OR ACCURACY OF ITS DELINEATION OF SUCH UNDERGROUND UTILITIES, NOR FOR THE EXISTENCE OF OTHER BURIED OBJECTS OR UTILITIES WHICH MAY BE ENCOUNTERED BUT WHICH ARE NOT SHOWN ON THESE DRAWINGS. THE CONTRACTOR OR ANY SUBCONTRACTOR FOR THIS CONTRACT SHALL NOTIFY THE DISTRICT TWO (2) WORKING DAYS IN ADVANCE OF PERFORMING ANY EXCAVATION WORK IN ORDER TO VERIFY TO THE GREATEST EXTENT POSSIBLE THE EXISTING UTILITY LINES, CONFLICTS AND PROPOSED UTILITY CONNECTION POINTS.
- 7. THE SCHOOL DISTRICT SHALL HAVE SALVAGE RIGHTS TO ANY DEMOLISHED ITEMS SHOWN HEREON. THE CONTRACTOR SHALL GIVE THE DISTRICT NOTICE 7 DAYS PRIOR TO THE START OF DEMOLITION. THE DISTRICT SHALL MOVE ANY RETAINED ITEMS OUT OF THE CONTRACTORS WORK AREA, UNLESS ANOTHER ARRANGEMENT IS MADE WITH THE CONTRACTOR. ANY REMAINING ITEMS BECOME THE PROPERTY OF THE CONTRACTOR AND SHALL BE REMOVED FROM THE SITE. ANY ITEMS NOT SHOWN FOR REMOVAL SHALL REMAIN AND SHALL BE PROTECTED FROM DAMAGE DURING CONSTRUCTION TO A REASONABLE
- 8. EXISTING UTILITY STRUCTURES IN AREAS OF NEW PAVING SHALL BE REMOVED AND REPLACED WITH NEW BOX/COVER AT NEW GRADE UNLESS SPECIFICALLY NOTED OTHERWISE.
- 9. ITEMS OUTSIDE THE LIMITS OF DEMOLITION SHALL REMAIN AND BE PROTECTED FROM DAMAGE DURING CONSTRUCTION.
- 10. EXISTING UTILITY STRUCTURES AND PIPING NOT SHOWN ON DEMOLITION PLAN TO BE REMOVED SHALL REMAIN AND BE PROTECTED.

### UTILITY VERIFICATION NOTE

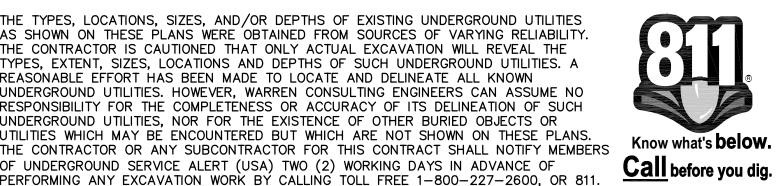
PRIOR TO THE START OF CONSTRUCTION, VERIFY AND POTHOLE ALL UTILITY POINTS OF CONNECTION FOR LOCATION, DEPTH, AND SIZE. IF CONFLICT IS FOUND. CONTACT THE ENGINEER IMMEDIATELY FOR

### IRRIGATION DEMOLITION NOTE

WITHIN LANDSCAPE AREAS TO BE DEMOLISHED THERE MAY BE EXISTING IRRIGATION LINES NOT SHOWN ON THIS PLAN. CONTRACTOR SHALL REMOVE LATERAL LINES AND HEADS ENCOUNTERED. MAIN LINES AND CONTROL WIRES MAY ONLY BE REMOVED PROVIDED THAT ROUTING IS KNOWN AND REMOVAL WILL NOT DEACTIVATE AN IRRIGATION SYSTEMS INTENDED TO REMAIN. IF CONFLICT IS FOUND, CONTACT THE ENGINEER FOR DIRECTION.

### **GENERAL NOTES:**

 THE TYPES, LOCATIONS, SIZES, AND/OR DEPTHS OF EXISTING UNDERGROUND UTILITIES AS SHOWN ON THESE PLANS WERE OBTAINED FROM SOURCES OF VARYING RELIABILITY THE CONTRACTOR IS CAUTIONED THAT ONLY ACTUAL EXCAVATION WILL REVEAL THE TYPES, EXTENT, SIZES, LOCATIONS AND DEPTHS OF SUCH UNDERGROUND UTILITIES. A REASONABLE EFFORT HAS BEEN MADE TO LOCATE AND DELINEATE ALL KNOWN UNDERGROUND UTILITIES. HOWEVER. WARREN CONSULTING ENGINEERS CAN ASSUME NO RESPONSIBILITY FOR THE COMPLETENESS OR ACCURACY OF ITS DELINEATION OF SUCH UNDERGROUND UTILITIES, NOR FOR THE EXISTENCE OF OTHER BURIED OBJECTS OR UTILITIES WHICH MAY BE ENCOUNTERED BUT WHICH ARE NOT SHOWN ON THESE PLANS. THE CONTRACTOR OR ANY SUBCONTRACTOR FOR THIS CONTRACT SHALL NOTIFY MEMBERS OF UNDERGROUND SERVICE ALERT (USA) TWO (2) WORKING DAYS IN ADVANCE OF



- . WARREN CONSULTING ENGINEERS, INC. (WCE) ASSUMES NO RESPONSIBILITY FOR ERRORS IN PHYSICAL LOCATION OF IMPROVEMENTS, HORIZONTAL OR VERTICAL, IF STAKED BY OTHERS. IN ADDITION, ANY SUCH ERRORS IN PHYSICAL LOCATION MAY AFFECT THE INTENDED DESIGN OF SUCH IMPROVEMENTS AND WCE CANNOT BE HELD RESPONSIBLE FOR SUCH CONDITIONS WHICH ARE A RESULT OF ERRORS IN SURVEYING, OR IMPROPER CONSTRUCTION.
- 3. IF SUBSURFACE CULTURAL RESOURCES, REMAINS, AND/OR ARTIFACTS ARE UNCOVERED DURING PROJECT CONSTRUCTION, ALL WORK IN THE VICINITY SHALL BE STOPPED UNTIL SUCH ITEMS CAN BE ASSESSED BY AN APPROPRIATE MEMBER OF THE COUNTY ENVIRONMENTAL IMPACT SECTION STAFF.
- 4. CONTRACTOR AGREES THAT HE/SHE SHALL ASSUME SOLE AND COMPLETE RESPONSIBILITY FOR JOB SITE CONDITIONS DURING THE COURSE OF CONSTRUCTION OF THIS PROJECT, INCLUDING SAFETY OF ALL PERSONS AND PROPERTY: THAT THIS REQUIREMENT SHALL APPLY CONTINUOUSLY AND SHALL NOT BE LIMITED TO NORMAL WORKING HOURS: AND THAT THE CONTRACTOR SHALL DEFEND, INDEMNIFY AND HOLD THE OWNER AND ENGINEER HARMLESS FROM ANY AND ALL LIABILITY, REAL OR ALLEGED, IN CONNECTION WITH THE PERFORMANCE OF WORK ON THIS PROJECT, EXCEPTING FOR LIABILITY ARISING FROM THE SOLE NEGLIGENCE OF THE OWNER OR ENGINEER.
- 5. THE CONTRACTOR SHALL OBTAIN AN EXCAVATION PERMIT FROM THE STATE OF CALIFORNIA DEPARTMENT OF INDUSTRIAL SAFETY FOR ALL EXCAVATIONS OF 5 FEET OR MORE IN DEPTH.
- 6. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO MAKE ALL NECESSARY PRE-BID AND PRE-CONSTRUCTION SITE INSPECTION, AND/OR OBSERVATIONS ON THE SITE TO PRE-DETERMINE ALL HIS/HER MEANS AND METHODS NECESSARY TO COMPLETE THE IMPROVEMENTS SHOWN ON THESE PLANS AND PER THE PROJECT SPECIFICATIONS. IT IS THE CONTRACTORS RESPONSIBILITY TO DETERMINE, AND INCLUDE IN HIS/HER CONTRACT, ALL MEANS AND METHODS NECESSARY TO PERFORM A COMPLETE AND ACCEPTABLE JOB.
- . WHERE IMPROVEMENTS LIE WITHIN AN EXISTING DEVELOPED AREA, CONTRACTOR SHALL USE CAUTION WHEN ACCESSING THE SITE THROUGH THESE EXISTING IMPROVEMENTS. IT IS THE CONTRACTORS RESPONSIBILITY TO PROTECT ANY SUCH EXISTING IMPROVEMENTS OUTSIDE THE PROJECT BOUNDARY, OR EXISTING IMPROVEMENTS WITHIN THE BOUNDARY WHICH ARE TO REMAIN. PROPER PRECAUTIONS SHALL BE PROVIDED AND MAINTAINED THROUGHOUT CONSTRUCTION. ANY DAMAGE SHALL BE REPAIRED OR REPLACED TO THE SATISFACTION OF THE
- 8. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO KEEP DETAILED RECORDS OF MINOR CHANGES OR ADJUSTMENTS MADE DURING CONSTRUCTION (WHICH WERE NOT FORMALLY ISSUED). UPON PROJECT COMPLETION, THESE RECORDS AND/OR INFORMATION SHALL BE PROVIDED TO THE OWNER AND WARREN CONSULTING ENGINEERS, INC. UNLESS AN OFFICIAL "AS-BUILT" SET OF PLANS IS A REQUIREMENT OF THE CONTRACT. IF AS-BUILT PLANS ARE A REQUIREMENT OF THE CONTRACT, REFER TO SPECIFICATIONS FOR AS-BUILT DELIVERABLE REQUIREMENTS.
- 9. IN VEHICULAR PATHWAYS, EXISTING ASPHALTIC AND/OR CONCRETE SURFACES SHALL BE CUT TO A NEAT AND STRAIGHT LINE, PARALLEL OR PERPENDICULAR TO THE VEHICULAR TRAVELED PATH. THIS IS TYPICALLY THE ROADWAY CENTERLINE, BUT MAY VARY. THAT SAWCUT EDGE SHALL BE PROTECTED FROM DAMAGE DURING CONSTRUCTION SO A CLEAN EDGE REMAINS FOR PATCH BACK.. IF EDGE IS DAMAGED, A NEW SAW CUT WILL BE REQUIRED. THE EXPOSED EDGE SHALL BE "TACKED" WITH EMULSION PRIOR TO PAVING.
- 10. NO BURNING OR BLASTING SHALL BE ALLOWED ONSITE UNLESS SPECIFICALLY ADDRESSED ON PLANS, OR SPECIFICALLY APPROVED AND COORDINATED WITH THE ARCHITECT, ENGINEER, AND LOCAL AGENCY OR OTHER ADMINISTRATIVE AUTHORITY.
- 11. SUBGRADE AND RESULTING FINISHED GRADE SHALL BE CONSTRUCTED SMOOTH AND UNIFORM BETWEEN SPOT ELEVATIONS, CONTOURS OR OTHER STRUCTURE ELEVATIONS SHOWN ON GRADING OR OTHER PLANS. NO MOUNDS, RUTS, DEPRESSIONS OR OTHER GRADING DEFICIENCIES WILL BE ALLOWED UNLESS SPECIFICALLY SHOWN ON PLANS.
- 12. ON NEW WATER SYSTEMS, SERVICE LATERALS SHALL BE MADE USING APPROPRIATE "TEE" AND "WYE" FITTINGS. SADDLE TAPS WILL ONLY BE ALLOWED WHEN MAKING CONNECTIONS TO EXISTING WATER MAINS.
- 13. CURING COMPOUND SHALL BE APPLIED IN A CONTINUOUS SOLID WET FLOWING COAT. ANY "SPOTTY" APPLICATIONS SHALL BE RECOATED IMMEDIATELY. APPLICATION SHALL BE INSPECTED BY PROJECT INSPECTOR DURING APPLICATION.
- 14. EMBEDMENT OF FEATURES IN CONCRETE PAVING, CURBS, OR WALLS, SUCH AS SQUARE OR ROUND TUBING, POSTS, OR COLUMNS, STEEL BOLTED PLATES, OR OTHER STRUCTURES, SHALL REQUIRE ADDITIONAL SCORE OR EXPANSION JOINTS TO PREVENT UNCONTROLLED CRACKING. THOSE ADDITIONAL JOINTS MAY OR MAY NOT BE SPECIFICALLY SHOWN ON PLANS BUT SHALL BE PROVIDED BY THE CONTRACTOR.
- 15. EMBEDMENT OF FEATURES IN CONCRETE PAVING, CURBS, OR WALLS, SUCH AS SQUARE OR ROUND TUBING, POSTS, OR COLUMNS, STEEL BOLTED PLATES, OR OTHER STRUCTURES, SHALL REQUIRE A MINOR ADJUSTMENT OF REBAR WITHIN CONCRETE TO ALLOW FOR SUCH STRUCTURE. THAT REBAR ADJUSTMENT MAY NOT BE SPECIFICALLY SHOWN
- 16. NO MORE THAN 1 GALLON OF WATER PER YARD OF CONCRETE CAN BE ADDED TO THE TRUCK AFTER ARRIVAL TO PROJECT SITE. THE ADDITION OF WATER CAN ONLY BE ADDED UNDER THE SUPERVISION OF THE CONCRETE INSPECTOR OR LABORATORY TECHNICIAN.
- 17. WHEN PUMPING CONCRETE FOR PLACEMENT, ABSOLUTELY NO WATER IS TO BE ADDED TO PUMP HOPPER. ANY WATER ADDED TO HOPPER WILL BE REASON FOR CONCRETE REJECTION AT THE CONTRACTORS EXPENSE.
- 18. ALL CONTRACTION/CONSTRUCTION JOINTS "CJ" SHALL BE 1/4 THE SLAB THICKNESS DEEP, BUT NO LESS THAN 1" FOR CONTROLLING OF CRACKING. CONTRACTOR SHALL EXERCISE CAUTION WHEN FINAL TROWELING OF CONCRETE SO AS NOT TO FILL IN THESE JOINTS WITH CONCRETE CREAM. ANY CRACKS OUTSIDE OF JOINTS WHICH WERE CONSTRUCTED LESS THAN 1" DEEP, SHALL BE CAUSE FOR CONCRETE SLAB(S) TO BE REMOVED AND REPLACE AT
- 19. ANY SCREED BOARDS SET WITHIN CONCRETE SLABS SHALL BE AN "OVERHEAD SCREED" SO THERE IS NO INTERFERENCE WITH THE PLACEMENT AND ALIGNMENT OF SLAB REINFORCING.
- 20. 3-1/2" FELT JOINTS WILL NOT BE ACCEPTED. PROVIDE A FULL 4" FELT JOINT FOR 4" SLAB CONSTRUCTION, AND A 6" FELT JOINT FOR A 6" SLAB SLAB CONSTRUCTION.
- 21. SHOULD ANY SHRINKAGE CRACKS OCCUR OUTSIDE OF EITHER THE EXPANSION JOINTS OR CRACK CONTROL JOINTS, THEN THE CONCRETE SLAB SHALL BE SAWCUT AT THE NEAREST JOINTS ON EACH SIDE OF THE CRACK AND THE CONCRETE SECTION SHALL BE, REMOVED AND REPLACED. NEW CONCRETE SHALL BE DOWELED INTO EXISTING CONCRETE PER DRAWING DETAIL.
- 22. ALL AREAS DISTURBED BY GRADING OPERATIONS WHETHER SHOWN ON THE DRAWINGS OR NOT SHALL BE HYDRO SEEDED UNLESS OTHERWISE NOTED. HYDRO SEEDING SHALL CONFORM TO LOCAL CITY/COUNTY STANDARDS.
- 23. REPAIR OR PATCHING OF GALVANIZED METALS, SUCH AS AFTER WELDING GALVANIZED COMPONENTS, SHALL BE MADE USING A ZINC COMPOSITION "HOT STICK" APPLICATION PER ASTM A 780-01. GALVANIZING PAINTS WILL NOT BE ALLOWED.

### **GENERAL PAVING SURFACE NOTES:**

- 1. PROVIDE EQUIVALENT OF MEDIUM BROOM FINISH AT SLOPES UP TO 5.99%, TYPICAL. PROVIDE EQUIVALENT OF HEAVY BROOM FINISH AT SLOPES 6% AND GREATER. REFER TO SPECIFICATIONS.
- 2. ALL NEW PEDESTRIAN WALKWAYS (NON-RAMP) SHALL BE SLOPED NO GREATER THAN 2.0%, AND NO LESS THAN 0.75% IN ANY DIRECTION, UNLESS SPECIFICALLY LABELED OTHERWISE. ALL CONCRETE SHALL MEET THE FOLLOWING SLOPE REQUIREMENTS:
- NO GREATER THAN 5% SLOPE IN THE DIRECTION OF TRAVEL. - NO GREATER THAN 2% SLOPE CROSSING THE DIRECTION OF TRAVEL.
- NO GREATER THAN 2% SLOPE IN ANY DIRECTION IN COURTYARD OR PLAZA AREAS.

### CIVIL SHEET INDEX

- CO.1 CIVIL GENERAL NOTES AND ABBREVIATIONS
- C1.1 DEMOLITION PLAN
- C2.1 GRADING AND PAVING PLAN



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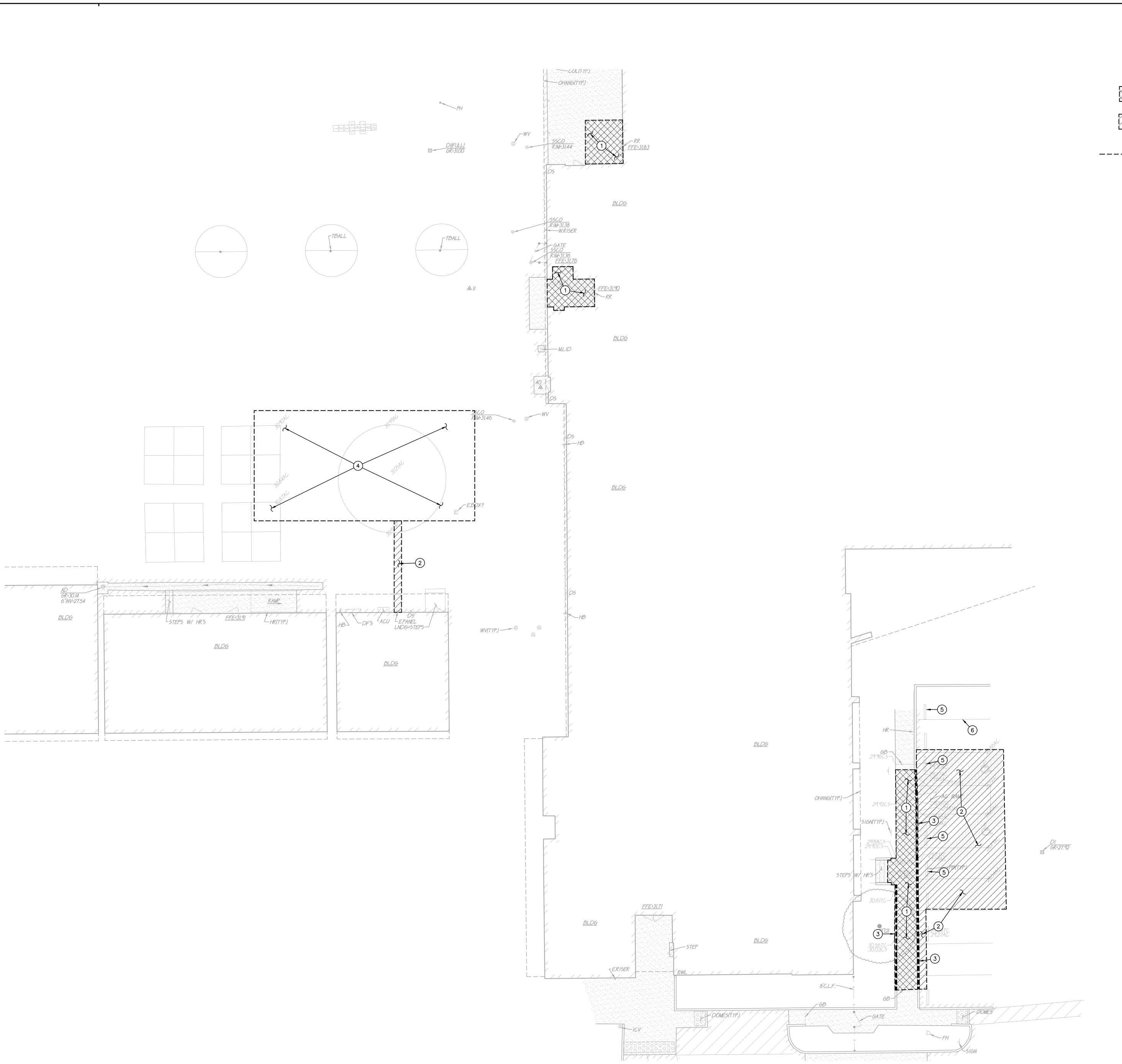
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**CIVIL GENERAL NOTES AND ABBREVIATIONS** 

PROJECT NO. 3/21/2022



DEMOLITION NOTES

SAWCUT, REMOVE AND DISPOSE OF EXISTING CONCRETE PAVING AND ASSOCIATED AGGREGATE BASE. SAWCUT SHALL BE A NEAT STRAIGHT LINE, MAINTAIN CLEAN, STRAIGHT CUT EDGE UNTIL NEW PAVING IS PLACED.

2. SAWCUT, REMOVE AND DISPOSE OF EXISTING ASPHALT PAVING AND ASSOCIATED AGGREGATE BASE. SAWCUT SHALL BE A NEAT STRAIGHT LINE, MAINTAIN CLEAN, STRAIGHT CUT EDGE UNTIL NEW PAVING IS PLACED.

---- 3. REMOVE AND DISPOSE OF EXISTING CONCRETE CURB.

- 4. REMOVE ASPHALT PAVING AS REQUIRED AT PROPOSED SHADE STRUCTURE FOOTING LOCATIONS TO ALLOW FOR INSTALLATION.
- REMOVE AND SALVAGE EXISTING PARKING BUMPER FOR REINSTALLATION.
- 6. BLACK OUT EXISTING STRIPING.

TOO + A studio of HMC Archited



WARREN CONSULTING ENGINEER
1117 WINDFIELD WAY, SUITE
EL DORADO HILLS, CA 95762 | (916)

ELEMENTARY SCHOOL

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DEMOLITION PLAN

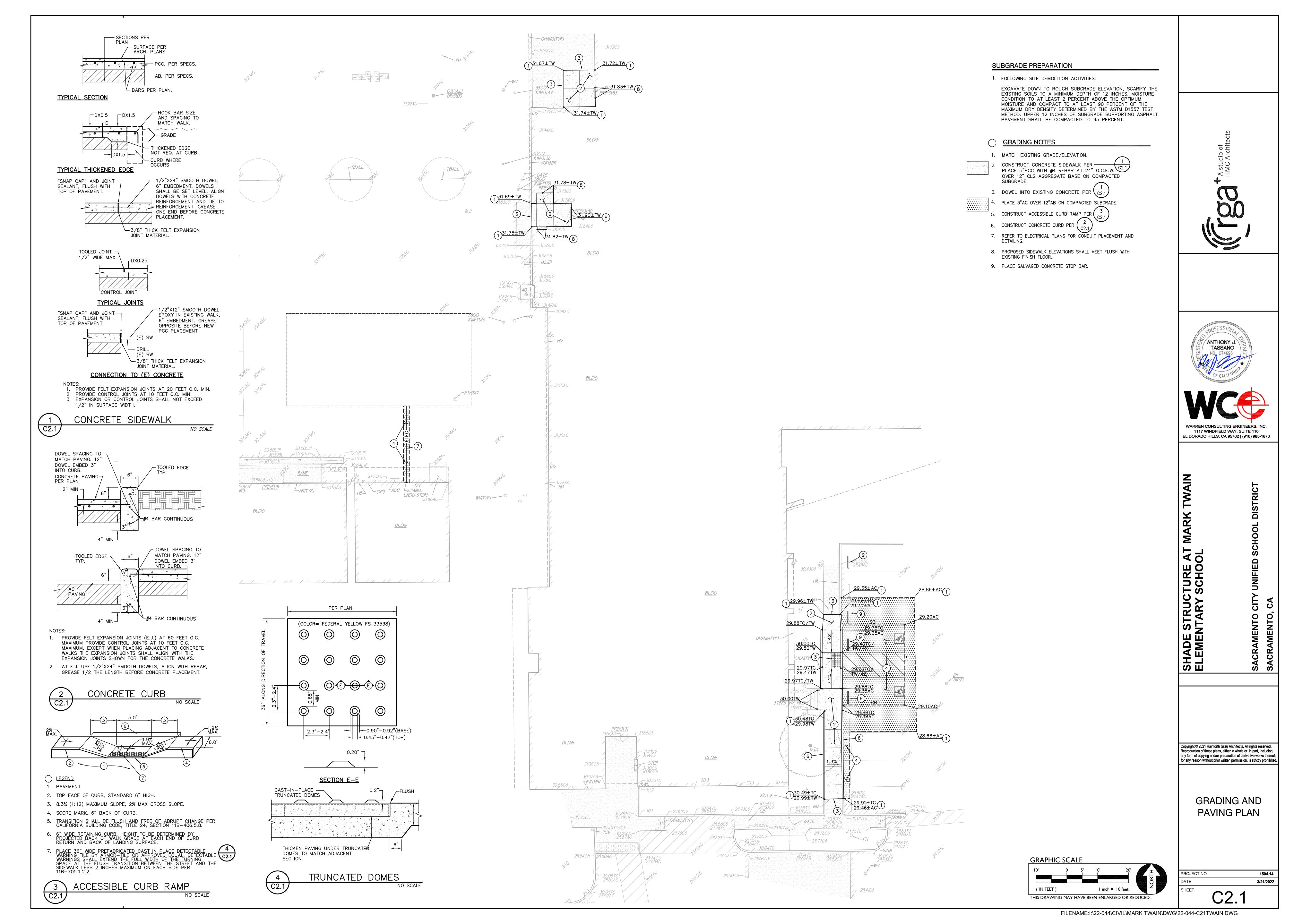
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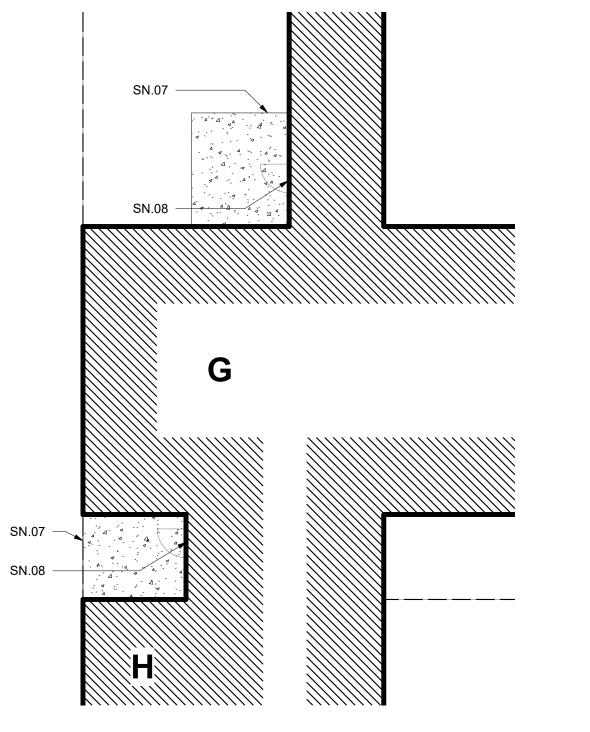
GRAPHIC SCALE

10' 0 5' 10' 20'
PROJECT NO.
DATE:

SHEET

THIS DRAWING MAY HAVE BEEN ENLARGED OR REDUCED.







(E) DRINKING FOUNTAIN
NOT TO SCALE



(E) APPARATUS

(E) HARDCOURT

**UNIT SS** 

(PC SHADE STRUCTURE / DEFERRED APPROVAL)



22ND AVENUE

IMPROVEMENT (

\_\_\_\_\_\_\_

SN.05

SN.06

		PROPOSED SHAD	E STRUCTURE	
UNIT	DESCRIPTION	CONSTRUCTION TYPE	OCCUPANCY	ALLOWABLE AREA
SS1-SS2	SHADE	II-B OR	A-3	9,500 SF MAX
33. 332	STRUCTURE	V-B	A-3	6,000 SF MAX

	<u>E</u>	XISTING BUILDING DESIGN	<u>ATIONS</u>	
UNIT	DESCRIPTION	DSA APPLICATION #	AREA (SF)	NOTES
А	MULTI-PURPOSE, ADMIN., CLASSROOMS	6936, 15440	10,779	
В	CLASSROOMS	6936	1,930	
С	CLASSROOMS	6936	2,890	
D	CLASSROOMS	6936	2,890	
E	CLASSROOMS	6936	2,890	
F	CLASSROOMS	6936	3,610	
G	TOILET ROOMS	6936, THIS APPLICATION	630	
Н	MECH./ ELECTRICAL	6936	557	
I-J	TEACHER FACILITIES	11292, THIS APPLICATION	2,020	
K	RELOCATABLE CLASSROOMS	53491	960	
L	RELOCATABLE CLASSROOMS	<u>-</u>	1,920	
М	RELOCATABLE CLASSROOMS	-	1,920	

EXISTING PATH OF TRAVEL (POT): ARCHITECT STATEMENT

DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE IN CHARGE STATEMENT: THE POT IDENTIFIED IN THESE CONSTRUCTION DOCUMENTS IS COMPLIANT WITH THE CURRENT APPLICABLE CALIFORNIA BUILDING CODE ACCESSIBILITY PROVISIONS FOR PATH OF TRAVEL REQUIREMENTS FOR ALTERATIONS, ADDITIONS AND STRUCTURAL REPAIRS. AS PART OF THE DESIGN OF THIS PROJECT, THE POT WAS EXAMINED AND ANY ELEMENTS, COMPONENTS OR PORTIONS OF THE POT THAT WERE DETERMINED TO BE NON-COMPLIANT

1) HAVE BEEN IDENTIFIED AND
2) THE CORRECTIVE WORK NECESSARY TO BRING THEM INTO COMPLIANCE HAS BEEN INCLUDED WITHIN THE SCOPE OF THIS PROJECT'S WORK THROUGH DETAILS, DRAWINGS, AND SPECIFICATIONS INCORPORATED INTO THESE CONSTRUCTION DOCUMENTS.

ANY NONCOMPLIANT ELEMENTS, COMPONENTS OR PORTIONS OF THE POT THAT WILL NOT BE CORRECTED BY THIS PROJECT BASED ON VALUATION THRESHOLD LIMITATIONS OR A FINDING OF UNREASONABLE HARDSHIP ARE SO INDICATED IN THESE CONSTRUCTION DOCUMENTS.

DURING CONSTRUCTION, IF POT ITEMS WITHIN THE SCOPE OF THE PROJECT REPRESENTED AS CODE COMPLIANT ARE FOUND TO BE NON-CONFORMING BEYOND REASONABLE CONSTRUCTION TOLERANCES, THEY SHALL BE BROUGHT TO COMPLIANCE WITH THE CBC AS PART OF THIS PROJECT BY MEANS OF A CONSTRUCTION CHANGE DOCUMENT.

### ACCESSIBLE PARKING STALL CALCULATION

ACCESSIBLE PARKING STALLS

REQUIRED ACCESSIBLE STALLS:

REQUIRED VAN ACCESSIBLE STALLS:

ACCESSIBLE STALLS PROVIDED:

TOTAL PARKING STALL COUNT:

16 STALLS (TABLE 11B-208.2) 1 (1-25 TOTAL STALLS) 1 (1-6 ACCESSIBLE STALLS) 1 STANDARD & 1 VAN

### LEGEND

— • • • PROPERTY LINE

UNIT DESIGNATION
PC SHADE STRUCTURE / DEFERRED APPROVAL

UNIT DESIGNATION

EXISTING BUILDINGS

CONCRETE WALK / PAVING

CONTROL JOINT

ASPHALT CONCRETE PAVING

UNIT VERTICAL TO 2-UNITS HORIZONTAL.

SITE WALKWAYS SHALL PROVIDE A BARRIER-FREE P.O.T.
ABRUPT CHANGES IN LEVEL ALONG ANY P.O.T. ARE
ALLOWED UP TO 1/2". ONLY ABRUPT CHANGES IN ELEVATION

UP TO 1/4" ARE ALLOWED TO HAVE A VERTICAL TRANSITION.
ABRUPT CHANGES IN ELEVATION BETWEEN 1/4" AND 1/2"
SHALL BE BEVELED WITH A SLOPE NO GREATER THAN 1-

WALKWAYS SHALL BE FREE OF GRATINGS WHEREVER POSSIBLE. GRATING WHICH OCCUR WITHIN THE P.O.T. SHALL HAVE OPENINGS WHICH DO NOT EXCEED 1/2" IN THE DIRECTION OF TRAVEL PER CBC SECTION 11B-302.3. AN ABRUPT DROP-OFF CHANGE IN ELEVATION AT THE EDGE OF ANY WALK INTO AN ADJACENT PLANTER SHALL NOT EXCEED 4".

SLOPES IN THE DIRECTION OF THE P.O.T. GREATER THAN 1-UNIT VERTICAL TO 20-UNITS HORIZONTAL SHALL BE CONSIDERED A RAMP AND WILL REQUIRE HANDRAILS ON BOTH SIDES PER CBC SECTION 11B-505 SLOPES IN THE DIRECTION OF THE P.O.T. ALONG WALKWAYS SHALL NOT

EXCEED 5%. CROSS SLOPES IN THE P.O.T. ALONG WALKWAYS SHALL NOT EXCEED 2%.
ALL WALKWAYS WITHIN THE P.O.T. SHALL BE A MINIMUM OF 48" IN WIDTH. SURFACES WITH A SLOPE OF 5% OR LESS SHALL BE AT LEAST AS SLIP-RESISTANT AS THAT PROVIDED BY A LIGHT BROOM FINISH. SURFACES WITH A SLOPE OF MORE THAN 5% SHALL BE AT LEAST AS SLIP-RESISTANT AS THAT PROVIDED BY A MEDIUM BROOM

OBJECTS PROTRUDING INTO THE P.O.T. SHALL NOT REDUCE THE CLEAR WIDTH OR MANEUVERING SPACE WITHIN THE P.O.T. PER CBC SECTION 11B-307.

PASSING SPACES (11B-403.5.3) OF 60" X 60" MIN. ARE LOCATED NOT MORE THAN 200' APART. WALKS WITH CONTINUOUS GRADIENTS SHALL HAVE 60" IN LENGTH LEVEL RESTING AREAS (11B-403.7) NOT MORE THAN 400' APART. P.O.T. SHALL BE MAINTAINED FREE OF OVERHANGING OBSTRUCTIONS TO 80" MIN (11B-307.4) AND FREE OF PROTRUDING OBJECTS (11B-307) GREATER THAN 4" PROJECTION FROM WALL ABOVE 27" AND LESS THAN 80". OBJECTS PROTRUDING INTO THE P.O.T SHALL NOT REDUCE THE CLEAR WIDTH OR MANEUVERING SPACE REQUIRED FOR ACCESSIBLE ROUTES (11B-307.5).

### SHEET NOTES

SN.01 (E) PARKING LOT ENTRANCE SIGN REVIEWED
AND VERIFIED PER THIS APPLICATION.
SN.02 ACCESSIBLE PARKING STALLS PER THIS APPLICATION
SN.03 (E) ACCESSIBLE STAFF TOILET ROOM UPGRADED
PER THIS APPLICATION

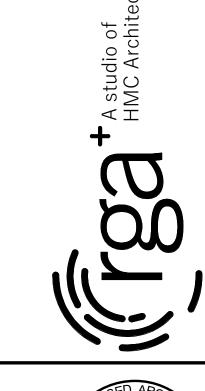
SN.04 (E) ACCESSIBLE GIRL'S TOILET ROOM UPGRADED PER THIS APPLICATION
SN.05 (E) ACCESSIBLE BOY'S TOILET ROOM UPGRADED PER THIS APPLICATION

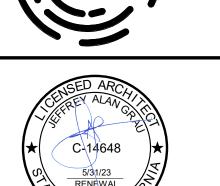
SN.06 (E) ACCESSIBLE DRINKING FOUNTAIN REVIEWED AND VERIFIED PER THIS APPLICATION. SEE 2/A1.1.0
SN.07 INSTALL NEW CONCRETE WITH 2% MAX. SLOPE IN ALL DIRECTIONS. EDGES TO HAVE A FLUSH TRANSITION TO (E) SLAB. SEE

SN.08 REMOVE (E) DOOR THRESHOLD. INSTALL NEW DOOR THRESHOLD PER

10

A0.2





E STRUCTURE AT MARK TV ENTARY SCHOOL

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Revision

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SITE PLAN AND CODE INFORMATION

PROJECT NO.

DATE:

SHEET

A1.1.0

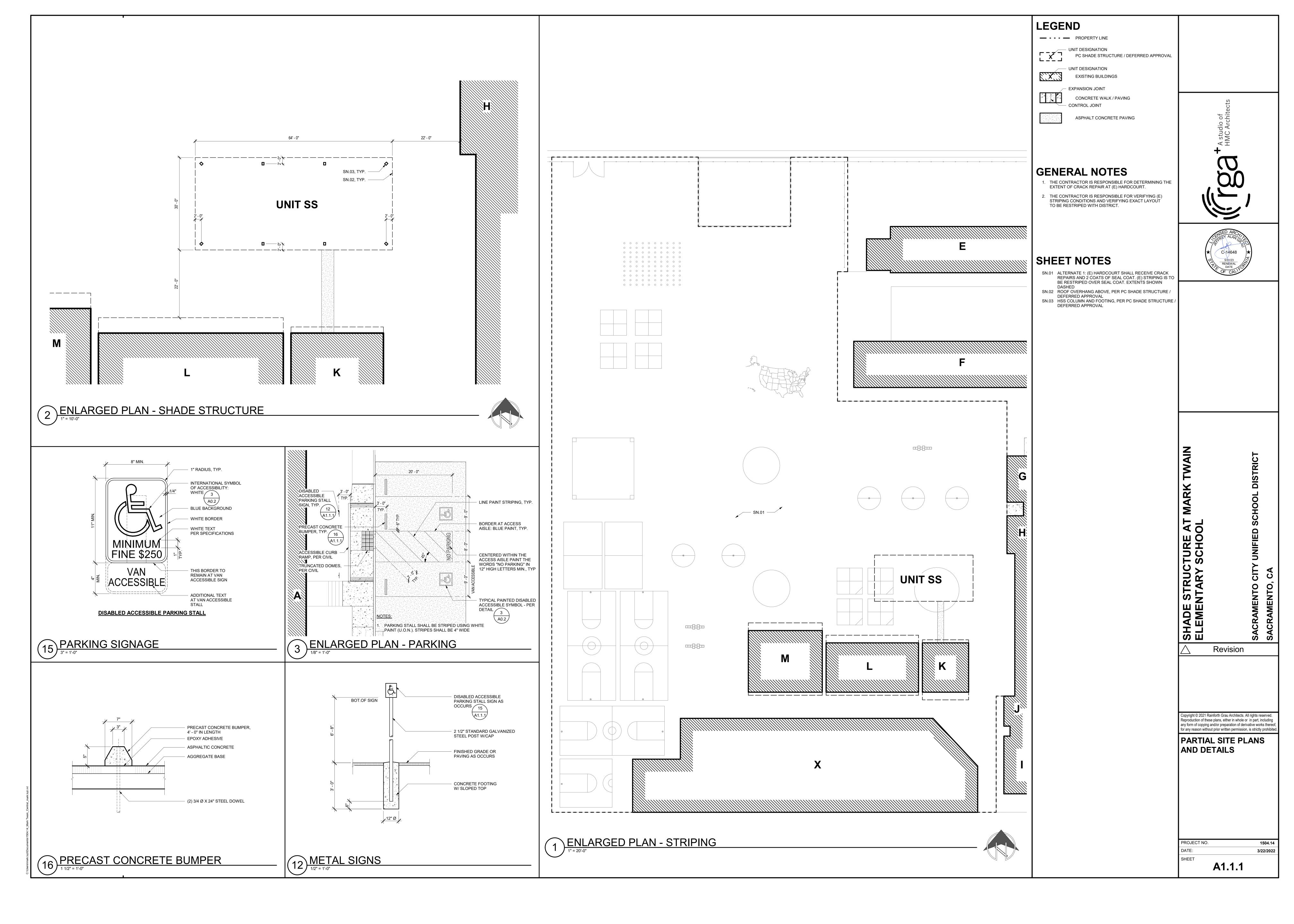


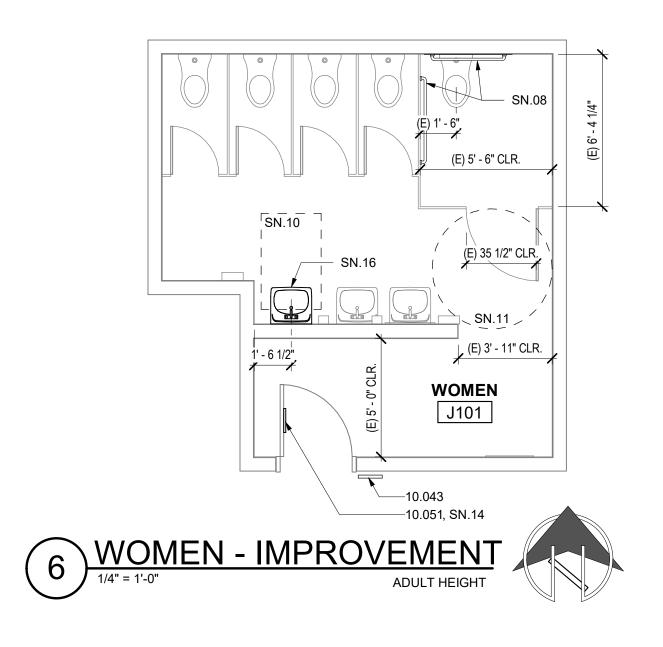
(E) PLAYFIELD / STADIUM

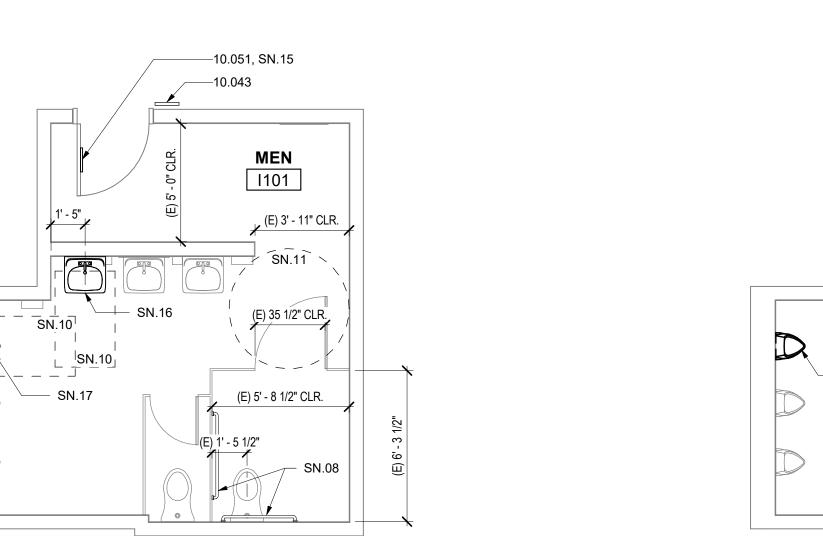


**PARKING LOT A** 

(UPGRADED PER THIS APPLICATION)





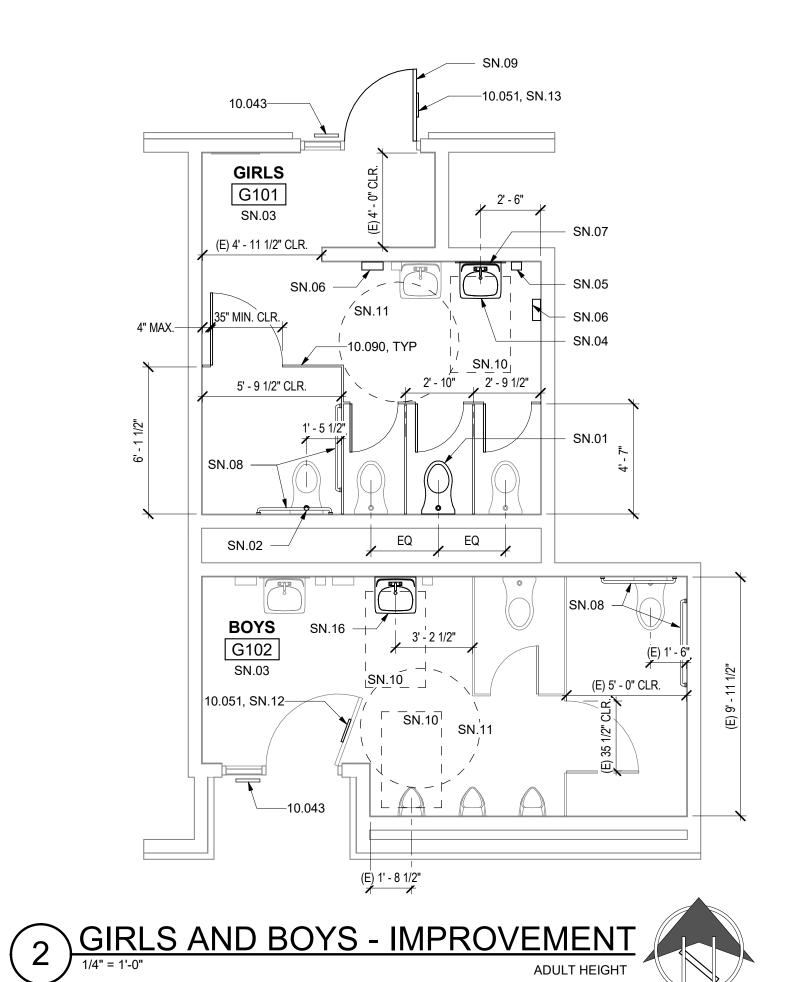




5 WOMEN - DEMOLITION

MEN

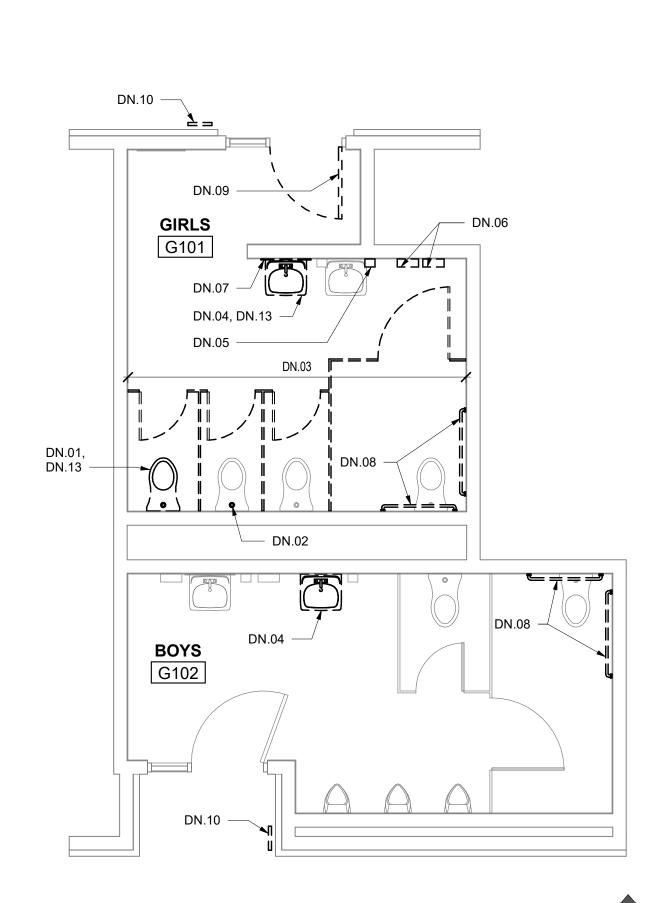
I101



4 MEN - IMPROVEMENT

1/4" = 1'-0"

ADULT HEIGHT





### **LEGEND**

INTERIOR ELEV.

CONSECUTIVE NUMBERING CONVENTION FOR INTERIOR **ELEVATIONS AND ROOM** FINISHES.

### **GENERAL NOTES**

CONTRACTOR UNDER THIS CONTRACT.

- FOR MOUNTING HEIGHTS, LOCATIONS, AND DETAILS, INCLUDING THOSE FOR DISABLED ACCESSIBITY, REFER TO SHEET A0.2
- PROTECT ALL ADJACENT SURFACES, ITEMS AND FINISHES NOT NOTED TO BE DEMOLISHED. EQUIPMENT/FIXTURES NOTED AS "SALVAGED FOR REINSTALLATION" WILL BE REMOVED AND STORED BY THE

CONTRACTOR PRIOR TO START OF DEMOLITION. THESE EQUIPMENT/FIXTURES SHALL BE REINSTALLED BY THE

- REMOVE ALL ITEMS SCHEDULED TO BE REMOVED, INCLUDING MOUNTING HARDWARE.
- DEMO AND REPAIR WALL FINISH AS NECESSARY TO PERFORM FIXTURE AND EQUIPMENT WORK AS NOTED. ADJACENT FINISHES TO BE VERIFIED BY CONTRACTOR.

### **DEMOLITION NOTES**

- DN.01 REMOVE (E) WALL-MOUNTED WATER CLOSET AND SALVAGE FOR REINSTALLATION DN.02 REMOVE (E) FLUSH VALVE AT (E) WATER CLOSET
- DN.03 REMOVE (E) TOILET PARTITIONS AND (E) TOILET PARTITION DN.04 REMOVE (E) LAVATORY AND SALVAGE FOR REINSTALLATION DN.05 REMOVE (E) SOAP DISPENSER AND SALVAGE FOR
- REINSTALLÁTION DN.06 REMOVE (E) PAPER TOWEL DISPENSER AND SALVAGE FOR
- REINSTALLATION DN.07 REMOVE (E) MIRROR AND SALVAGE FOR REINSTALLATION
- DN.08 REMOVE (E) GRAB BARS AND SALVAGE FOR REINSTALLATION DN.09 REMOVE (E) DOOR AND SALVAGE FOR REINSTALLATION DN.10 REMOVE (E) TOILET ROOM I.D. SIGN
- DN.11 REMOVE (E) TOILET ROOM DOOR SYMBOL DN.12 REMOVE (E) WALL-MOUNTED URINAL AND SALVAGE FOR
- REINSTALLATION DN.13 ABANDON AND CAP IN PLACE (E) PLUMBING, WHERE NOTED

### SHEET NOTES

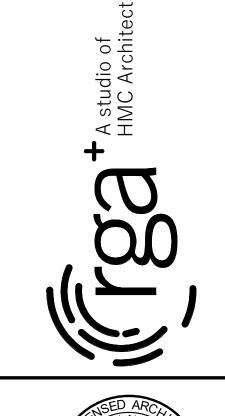
- SN.01 REINSTALL (E) SALVAGED WALL-MOUNTED WATER CLOSET TO COMPLY WITH A0.2. PROVIDE NEW WATER CARRIER. PROVIDE CONNECTION TO WATER LINE, WASTE LINE AND
- SN.02 PROVIDE NEW FLUSH VALVE AT (E) WALL-MOUNTED WATER CLOSET TO COMPLY WITH A0.2 SN.03 WRAP ALL EXPOSED PIPES WITH INSULATION SN.04 REINSTALL (E) SALVAGED LAVATORY TO COMPLY WITH
- A0.2. PROVIDE NEW WATER CARRIER. PROVIDE CONNECTION TO WATER LINE, WASTE LINE AND VENT. SN.05 REINSTALL (E) SALVAGED SOAP DISPENSER TO COMPLY
- WITH A0.2 SN.06 REINSTALL (E) SALVAGED PAPER TOWEL DISPENSER TO COMPLY WITH A0.2
- SN.07 REINSTALL (E) SALVAGED MIRROR TO COMPLY WITH A0.2 SN.08 REINSTALL (E) SALVAGED GRAB BARS TO COMPLY WITH A0.2
- SN.09 REINSTALL (E) SALAVAGED DOOR. CHANGE THE DIRECTION OF THE DOÒR SWING AS SHOWN
- SN.10 30" X 48" CLEAR SPACE SN.11 60" DIA. TURNING CIRCLE
- SN.12 SIGN TO READ "BOYS" SN.13 SIGN TO READ "GIRLS"
- SN.14 SIGN TO READ "WOMEN" SN.15 SIGN TO READ "MEN"
- SN.16 REINSTALL (E) SALVAGED LAVATORY TO COMPLY WITH A0.2. ADJUST (E) WATER CARRIER AS REQUIRED FOR RECONNECTION TO LAVATORY. RECONNECT TO (E) WATER LINE, WASTE LINE AND VENT. SN.17 REINSTALL (E) SALVAGED WALL-MOUNTED URINAL TO COMPLY WITH A0.2. ADJUST (E) WATER CARRIER AS

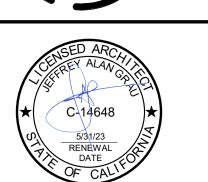
TO (E) WATER LINE, WASTE LINE AND VENT.

REQUIRED FOR RECONNECTION TO LAVATORY. RECONNECT

### **KEYNOTES**

10.043 SIGNAGE: TOILET ROOM IDENTIFICATION 10.051 SIGNAGE: TOILET ROOM DOOR SYMBOL 10.090 COMPOSITE TOILET COMPARTMENT





TURE SHADE

Revision

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TOILET ROOM **DEMOLITION AND** IMPROVEMENT PLANS

UNITS G, I & J A2.1.1



### LEGEND

4.5

INTERIOR
2

CONSECUTIVE NUMBERING
CONVENTION FOR INTERIOR
ELEVATIONS AND ROOM
FINISHES.

### **GENERAL NOTES**

- FOR MOUNTING HEIGHTS, LOCATIONS, AND DETAILS, INCLUDING THOSE FOR DISABLED ACCESSIBITY, REFER TO SHEET A0.2
- 2. PROTECT ALL ADJACENT SURFACES, ITEMS AND FINISHES NOT NOTED TO BE DEMOLISHED.
- 3. EQUIPMENT/FIXTURES NOTED AS "SALVAGED FOR REINSTALLATION" WILL BE REMOVED AND STORED BY THE CONTRACTOR PRIOR TO START OF DEMOLITION. THESE EQUIPMENT/FIXTURES SHALL BE REINSTALLED BY THE CONTRACTOR UNDER THIS CONTRACT.
- 4. REMOVE ALL ITEMS SCHEDULED TO BE REMOVED, INCLUDING MOUNTING HARDWARE.
- 5. DEMO AND REPAIR WALL FINISH AS NECESSARY TO PERFORM FIXTURE AND EQUIPMENT WORK AS NOTED. ADJACENT FINISHES TO BE VERIFIED BY CONTRACTOR.



### SHEET NOTES

- SN.01 REINSTALL (E) SALVAGED WALL-MOUNTED WATER CLOSET TO COMPLY WITH A0.2. PROVIDE NEW WATER CARRIER. PROVIDE CONNECTION TO WATER LINE, WASTE LINE AND VENT.

  SN.02 PROVIDE NEW FLUSH VALVE AT (E) WALL-MOUNTED WATER
- CLOSET TO COMPLY WITH A0.2 SN.03 NOT USED SN.04 REINSTALL (E) SALVAGED LAVATORY TO COMPLY WITH
- A0.2. PROVIDE NEW WATER CARRIER. PROVIDE CONNECTION TO WATER LINE, WASTE LINE AND VENT.

  SN.05 REINSTALL (E) SALVAGED SOAP DISPENSER TO COMPLY
- SN.06 REINSTALL (E) SALVAGED PAPER TOWEL DISPENSER TO COMPLY WITH A0.2
- SN.07 REINSTALL (E) SALVAGED MIRROR TO COMPLY WITH A0.2 SN.08 REINSTALL (E) SALVAGED GRAB BARS TO COMPLY WITH A0.2
- SN.09 NOT USED ` ´ SN.10 NOT USED

WITH A0.2

- SN.11 NOT USED SN.12 NOT USED SN.13 NOT USED
- SN.13 NOTUSED SN.14 NOTUSED
- SN.15 NOT USED
  SN.16 REINSTALL (E) SALVAGED LAVATORY TO COMPLY WITH
  A0.2. ADJUST (E) WATER CARRIER AS REQUIRED FOR
- RECONNECTION TO LAVATORY. RECONNECT TO (E) WATER LINE, WASTE LINE AND VENT.

  SN.17 REINSTALL (E) SALVAGED WALL-MOUNTED URINAL TO COMPLY WITH AD 2. AD JUST (E) WATER CARRIER AS
- SN.17 REINSTALL (E) SALVAGED WALL-MOUNTED URINAL TO COMPLY WITH A0.2. ADJUST (E) WATER CARRIER AS REQUIRED FOR RECONNECTION TO LAVATORY. RECONNECT TO (E) WATER LINE, WASTE LINE AND VENT.

### KEYNOTES

10.090 COMPOSITE TOILET COMPARTMENT

## SHADE STRUCTURE AT MARK TWA ELEMENTARY SCHOOL

Revision

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INTERIOR ELEVATIONS

UNITS G, I & J

ROJECT NO.

A5.1.1

### ABBREVIATION LIST AMPERE ALTERNATING CURRENT AIR CONDITIONING ARC ENERGY REDUCTION AMP FRAME ABOVE FINISHED FLOOR AMPERES INTERRUPTING CAPACITY AMP TRIP SETTING AMERICAN WIRE GAUGE BARE COPPER BELOW FINISHED CEILING BREAKER BUILDING **BOOSTER POWER SUPPLY** CONDUIT CIRCUIT BREAKER CONTRACTOR FURNISHED. CONTRACTOR INSTALLED CIRCUIT CEILING CONDUIT ONLY, WITH PULL LINE CONT CONTINUOUS METALLIC COLD WATER PIPE DEMOLISH DIRECT CURRENT DISCONNECT DISTRIBUTION PANEL EXISTING EACH WITH **EVENING LIGHT** ELECTRIC EMERGENCY ELECTRICAL METALLIC TUBING END OF LINE DEVICE **EQUIPMENT** EXISTING RELOCATED ELECTRICAL WATER COOLER ELECTRIC WATER HEATER FIRE ALARM CONTROL PANEL FAEP FIRE ALARM EXTENDER PANEL FATC FIRE ALARM TERMINAL CABINET FURNISHED BY OTHERS **FLUOR** FLUORESCENT GROUND FAULT CIRCUIT INTERRUPT GENERAL LIGHTING ZONE METALLIC GAS PIPE GYPSUM HIGH INTENSITY DISCHARGE HORSE POWER HEIGHT HERTZ INTERMEDIATE METALLIC CONDUIT SHORT CIRCUIT CURRENT (RMS SYMMETRICAL) ISOLATED J-B0X JUNCTION BOX THOUSAND CIRCULAR MILLS KCMIL KILO VOLT AMP KILOWATT LIGHTING CONTROL PANEL LOW VOLTAGE THOUSAND CIRCULAR MILLS MECHANICAL MAIN DISTRIBUTION PANEL METAL HALIDE MISCELLANEOUS MAIN LUGS ONLY MPOE MAIN POINT OF ENTRY MAIN SWITCHBOARD NOT IN CONTRACT NOT IN ELECTRICAL SECTION OF THESE PLANS & SPECS. NIGHT LIGHT NUMBER NOT TO SCALE ON CENTER OFCI OWNER FURNISHED, CONTRTRACTOR INSTALLED OFOI OWNER FURNISHED, OWNER INSTALLED PULL BOX PROVISION FOR FUTURE BREAKER W/ PFB MOUNTING HARDWARE PRIMARY DAYLIT ZONE PROVISION FOR FUTURE CURRENT TRANSFORMER PHASE PLYWOOD PLYWD PANEL PNLPAIR POLYVINYL CHLORIDE CONDUIT RELOCATE / RELOCATED (R) REQ'D REQUIRED ROOM RIGID METAL CONDUIT REMOVE AND REPLACE SECONDARY DAYLIT ZONE SKYLIGHT DAYLIT ZONE SPEC SPECIFICATION SIGNAL TERMINAL CABINET SQUARE SWITCH TELEPHONE TELECOMMUNICATIONS GROUNDING TELECOMMUNICATIONS MAIN GROUNDING BUSBAR TELEPHONE TERMINAL BOARD TYPICAL UNDERGROUND UNLESS OTHERWISE NOTED UON VOLTS WEATHERPROOF WEIGHT WATT WITH TRANSFORMER

### **GENERAL NOTES**

- 1. PLANS ARE NOT FOR CONSTRUCTION UNTIL APPROVED BY THE AUTHORITY HAVING JURISDICTION. THE CONTRACTOR SHALL NOT ORDER ANY MATERIALS OR INSTALL ANY EQUIPMENT, PIPING, ETC. UNTIL PLANS ARE APPROVED BY THE AUTHORITY HAVING JURISDICTION.
- 2. ALL WORK SHALL BE DONE AT SUCH TIME AND IN SUCH MANNER AS PRESCRIBED BY THE SCHOOL'S REPRESENTATIVE.
- 3. PROTECT EXISTING EQUIPMENT AND FURNISHINGS FROM ANY DAMAGE DUE TO DUST, MOISTURE OR CONTACT WITH WORK CREW OR MATERIALS.
- 4. THE SCHOOL SHALL BE NOTIFIED AT LEAST FORTY-EIGHT (48) HOURS IN ADVANCE OF ANY POWER SHUTDOWN OF EXISTING PANELS OR SERVICE. SCHEDULE OF SHUTDOWNS SHALL BE AT CONVENIENCE OF THE SCHOOL. THE SCHOOL MAY, AT THEIR OPTION, HAVE A REPRESENTATIVE PRESENT DURING SHUTDOWN. ALL WORK REQUIRING SHUTDOWNS OF EXISTING PANELS OR SERVICE SHALL BE DONE BETWEEN 12:00 AM MIDNIGHT AND 6:00AM WEEKDAYS OR ON SATURDAY AND SUNDAY. REQUIRED SHUTDOWNS SHALL BE KEPT TO A MINIMUM.
- 5. ADEQUATELY STRAP AND SUPPORT ALL CONDUIT WORK PER CEC. IN GENERAL, SUPPORT ALL CONDUIT WITHIN THREE FEET (3') OF OUTLET BOX, CABINET OR PANEL AND MAXIMUM TEN FEET (10') ON CENTER THEREAFTER.
- 6. CORE BORE SHALL BE 1" DIAMETER LARGER THAN EACH CONDUIT. SPACE CONDUIT HOLES 3" APART. SEAL AROUND CONDUIT WITH NON-SHRINK, NON-METALLIC GROUT.
- 7. ALL CONDUCTORS INSTALLED IN PANELBOARDS SHALL BE TRAINED, LACED, AND INSTALLED WITH PHASE TAPE ON ALL CONDUCTORS. 8. LABEL DEVICES (I.E. RECEPTACLES, ETC.) ON EACH COVER PLATE IDENTIFYING CIRCUIT AND PANEL DEVICE IS CONNECTED TO.
- 9. CLEAN ALL EXTERIOR AND INTERIOR SURFACES OF PANELS AND ALL MATERIAL AND METAL SHAVINGS FROM PANEL AND CABINET INTERIORS. ALL OPENINGS SHALL BE SEALED AND APPLY TOUCH-UP SPRAY PAINT WHERE NEEDED.
- 10. FIELD COORDINATE DEVICE LOCATIONS PRIOR TO ROUGH-IN.
- 11. CONTRACTOR WILL PROVIDE WARNING LABELS NOTING THE POTENTIAL FOR ELECTRIC ARC FLASH HAZARDS PER CEC 110.16. PROVIDE LABELS ON EQUIPMENT SUCH AS SWITCHBOARDS, SWITCHGEAR, PANELBOARDS, INDUSTRIAL CONTROL PANELS, METER SOCKET ENCLOSURES, MOTOR CONTROL CENTERS, MOTOR STARTER / CONTACTOR PANELS, DISCONNECTS, ETC.. PROVIDE WARNING LABELS BY BRADY, MODEL NO. 101517, OR EQUAL, ON ALL
- 12. INSTALLATION SHALL COMPLY WITH CEC 210.4 EACH MULTIWIRE BRANCH CIRCUIT SHALL BE PROVIDED WITH A MEANS THAT WILL SIMULTANEOUSLY DISCONNECT ALL UNGROUNDED CONDUCTORS AT THE POINT WHERE THE BRANCH CIRCUIT ORIGINATES. THEREFORE ANY CIRCUIT SHARING A COMMON NEUTRAL SHALL BE CAPABLE OF SIMULTANEOUS DISCONNECT OR DEDICATED NEUTRALS SHALL BE INSTALLED.
- 13. SUPPORT ENCLOSURES, BOXES AND CONDUIT INSTALLATIONS PER CEC 314.23 (A) THROUGH (H).
- 14. SEAL CONDUIT OPENINGS THROUGH WALLS AND CEILINGS. INSTALL ESCUTCHEON PLATES AT BUILDING INTERIOR. WHERE EQUIPMENT IS INSTALLED ON THE EXTERIOR WALL, STUB CONDUITS THROUGH WALL AND SEAL CONDUIT OPENINGS, THEN INSTALL EXTERIOR EQUIPMENT. ALSO, SEAL AROUND THE PERIMETER EDGE OF THE EQUIPMENT ENCLOSURE BETWEEN THE ENCLOSURE AND BUILDING.
- 15. CONDUITS INSTALLED ON ROOF AND BUILDING EXTERIOR SHALL BE RIGID GALV. STEEL (HEAVY WALL) WITH THREADED FITTINGS. CONDUIT AND WALL TO BE PAINTED OUT TO MATCH EXTERIOR FINISH.
- 16. SPLICES AND TERMINALS SHALL BE COMPRESSION TYPE OF SEAMLESS PURE COPPER, TIN PLATED, LONG BARREL (TERMINALS WITH TWO-HOLE PAD AND INSPECTION WINDOW WITH NEMA DRILLING), AS MANUFACTURED BY BURNDY TYPE YS, YAZ-2N OR EQUAL. CLEAN ALL SURFACES AND INSTALL WITH OXIDE INHIBITING COMPOUND, BURNDY PENETROX-E OR EQUAL. APPLY COMPOUND BETWEEN BUS AND LUG PAD AND BETWEEN CONDUCTOR AND LUG BARREL. INSTALL COMPRESSION CONNECTORS WITH 360° CIRCUMFERENTIAL COMPRESSION DYE, BURNDY HYPRESS OR EQUAL. THE INDENTER OR OTHER TYPE TOOLS WILL NOT BE ACCEPTABLE.
- 17. INSTALL 'MECHANICALLY FASTENED PHENOLIC NAMEPLATE WITH WHITE LETTERING ON BLACK BACKGROUND ON ALL EQUIPMENT, INCLUDING PULL BOXES, WITH DESCRIPTION INDICATED ON DRAWINGS. NAMEPLATES SHALL READ EXACTLY AS DESCRIBED ON THE DRAWINGS. IN GENERAL NAMEPLATE LETTERING SIZE SHALL BE 3/16" HIGH FOR ALL NAMEPLATES SERVING FEEDER AND BRANCH CIRCUIT BREAKERS. ON MAIN SERVICE PANEL. DISTRIBUTION PANELS AND ALL OTHER NAMEPLATES LETTERING SHALL BE 1/4" HIGH.
- 17.1. ALL SWITCHBOARDS, SWITCHGEAR, PANELBOARDS, VFD'S, MOTORS, JUNCTION BOXES, PULL BOXES, DISCONNECT SWITCHES, ETC., SHALL BE MARKED TO INDICATE EACH DEVICE OR EQUIPMENT WHERE THE POWER ORIGINATES PER CEC 408.4, FIELD IDENTIFICATION REQUIRED, (B) SOURCE OF SUPPLY.
- 18. COORDINATE EQUIPMENT LOCATIONS, CONTROL AND POWER WIRING REQUIREMENTS AND CONNECT POINTS WITH ALL APPLICABLE DISCIPLINES.
- 19. PROVIDE AND INSTALL FUSES PER UNIT NAMEPLATE DATA ON THE EQUIPMENT PROVIDED.
- 20. A LAMINATED COPY OF THE FINAL RECORD ONE LINE DIAGRAM SHALL BE PLACED IN ELEC ROOM.
- 21. PROVIDE WIRING DEVICES AND COVER PLATES IN COLOR(S) SELECTED BY ARCHITECT. THE COLOR OF THE WIRING DEVICE AND COVER PLATE SHALL BE THE SAME UNLESS SPECIFICALLY NOTED OTHERWISE.
- 22. RECEPTACLE WEATHERPROOF COVERS SHALL BE LISTED "EXTRA DUTY", LOCAKBLE, METAL, IN-USE TYPE.
- 23. REINSTALL EXISTING ELECTRICAL INSTALLATIONS DISTURBED. CERTAIN EXISTING ELECTRICAL INSTALLATIONS MAY BE LOCATED IN WALLS. CEILINGS OR FLOORS THAT ARE TO BE REMOVED AND ARE ESSENTIAL FOR THE OPERATION OF OTHER REMAINING INSTALLATIONS. WHERE THIS CONDITIONS OCCURS, PROVIDE A NEW EXTENSION OF ORIGINAL CIRCUITS, RACEWAYS, EQUIPMENT AND OUTLETS TO RETAIN SERVICE CONTINUITY. INSTALLATIONS SHALL BE CONCEALED IN FINISHED AREAS.
- 24. FOR ROOF PENETRATIONS, REFER TO ARCHITECTURAL PLANS FOR INSTALLATION REQUIREMENTS.
- 25. FOR WALL PENETRATION INSTALLATIONS, REFER TO ARCHITECTURAL PLANS FOR REQUIREMENTS.
- 26. PROVIDE "LOCK-ON" DEVICE FOR ALL CIRCUIT BREAKERS ON EMERGENCY DEDICATED CIRCUITS.
- 27. DRAWINGS ARE TO BE CONSIDERED DIAGRAMMATIC. CONTRACTOR SHALL ACCEPT RESPONSIBILITY IN FAMILIARIZING THEMSELVES WITH ARCHITECTURAL AND STRUCTURAL CONDITIONS ALONG WITH INHERENT SPACE LIMITATIONS. WITH THAT UNDERSTANDING SHALL PROVIDE ALL ITEMS OF LABOR, MATERIALS AND TOOLS REQUIRED TO PROVIDE A COMPLETE INSTALLATION.
- 28. MAINTAIN A MINIMUM OF 12" SEPARATION BETWEEN ANY CONDUIT AND (E) UTILITY CONDUIT.
- 29. FOR INTERSECTING TRENCHED CONDUIT, MAINTAIN OR EXCEED THE MINIMUM CONDUIT DEPTH REQUIREMENTS.

### MEP COMPONENT ANCHORAGE NOTE

SYSTEM BRACING NOTE

ALL MECHANICAL, PLUMBING AND ELECTRICAL COMPONENTS SHALL BE ANCHORED AND INSTALLED PER THE DETAILS ON THE DSA APPROVED CONSTRUCTION DOCUMENTS. THE FOLLOWING COMPONENTS SHALL BE ANCHORED AND BRACED TO MEET THE FORCE AND DISPLACEMENT REQUIREMENTS PRESCRIBED IN THE 2019 CBC SECTIONS 1617A.1.18 THROUGH 1617A.1.26 AND ASCE 7-16 CHAPTERS 13, 26 AND 30:

- ALL PERMANENT EQUIPMENT AND COMPONENTS. TEMPORARY, MOVEABLE OR MOBILE EQUIPMENT THAT IS PERMANENTLY ATTACHED (E.G. HARD WIRED) TO THE BUILDING UTILITY SERVICES SUCH AS ELECTRICITY, GAS OR WATER. "PERMANENTLY ATTACHED" SHALL INCLUDE ALL ELECTRICAL CONNECTIONS EXCEPT PLUGS FOR 110/20 VOLT RECEPTACLES HAVING A FLEXIBLE CABLE.
- 3. TEMPORARY, MOVEABLE OR MOBILE EQUIPMENT WHICH IS HEAVIER THAN 400 POUNDS OR HAS A CENTER OF MASS LOCATED 4 FEET OR MORE ABOVE THE ADJACENT FLOOR OR ROOF LEVEL THAT DIRECTLY SUPPORTS THE COMPONENT IS REQUIRED TO BE RESTRAINED IN A MANNER APPROVED BY DSA.
- THE FOLLOWING MECHANICAL AND ELECTRICAL COMPONENTS SHALL BE POSITIVELY ATTACHED TO THE STRUCTURE, BUT NEED NOT DEMONSTRATE DESIGN COMPLIANCE WITH THE REFERENCES NOTED ABOVE. THESE COMPONENTS SHALL HAVE FLEXIBLE CONNECTIONS PROVIDED BETWEEN THE COMPONENT AND ASSOCIATED DUCTWORK, PIPING, AND CONDUIT. FLEXIBLE CONNECTIONS MUST ALLOW MOVEMENT IN BOTH TRANSVERSE AND LONGITUDINAL DIRECTIONS:
- A. COMPONENTS WEIGHING LESS THAN 400 POUNDS AND HAVING A CENTER OF MASS LOCATED 4 FEET OR LESS ABOVE THE ADJACENT FLOOR OR ROOF LEVEL THAT DIRECTLY SUPPORTS THE COMPONENT. B. COMPONENTS WEIGHING LESS THAN 20 POUNDS, OR IN THE CASE OF DISTRIBUTED SYSTEMS, LESS THAN 5 POUNDS
- PER FOOT, WHICH ARE SUSPENDED FROM A ROOF OR FLOOR OR HUNG FROM A WALL. THE ANCHORAGE OF ALL MECHANICAL, ELECTRICAL AND PLUMBING COMPONENTS SHALL BE SUBJECT TO THE APPROVAL OF THE DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE CHARGE OR STRUCTURAL ENGINEER DELEGATED

### EQUIPMENT HAVE BEEN ANCHORED IN ACCORDANCE WITH THE ABOVE REQUIREMENTS. PIPING, DUCTWORK, AND ELECTRICAL DISTRIBUTION

RESPONSIBILITY AND ACCEPTANCE BY DSA. THE PROJECT INSPECTOR WILL VERIFY THAT ALL COMPONENTS AND

PIPING, DUCTWORK, AND ELECTRICAL DISTRIBUTION SYSTEMS SHALL BE BRACED TO COMPLY WITH THE FORCES AND DISPLACEMENTS PRESCRIBED IN ASCE 7-16 SECTION 13.3 AS DEFINED IN ASCE 7-16 SECTIONS 13.6.5, 13.6.6, 13.6.7, 13.6.8 AND 2019 CBC, SECTIONS 1617A.1.24, 1617A.1.25 AND 1617A.1.26.

THE METHOD OF SHOWING BRACING AND ATTACHMENTS TO THE STRUCTURE FOR THE IDENTIFIED DISTRIBUTION SYSTEM ARE AS NOTED BELOW. WHEN BRACING AND ATTACHMENTS ARE BASED ON A PREAPPROVED INSTALLATION GUIDE (E.G., OSHPD OPM FOR 2013 CBC OR LATER), COPIES OF THE BRACING SYSTEM INSTALLATION GUIDE OR MANUAL SHALL BE AVAILABLE ON THE JOBSITE PRIOR TO THE START OF AND DURING THE HANGING AND BRACING OF THE DISTRIBUTION SYSTEMS. THE STRUCTURAL ENGINEER OF RECORD SHALL VERIFY THE ADEQUACY OF THE STRUCTURE TO SUPPORT THE HANGER AND BRACE LOADS.

MECHANICAL PIPING (MP), MECHANICAL DUCTS (MD), PLUMBING PIPING (PP), ELECTRICAL DISTRIBUTION SYSTEMS (E): MP ☐ MD ☐ PP ☐ E ■ OPTION 1: DETAILED ON THE APPROVED DRAWINGS WITH PROJECT SPECIFIC NOTES AND DETAILS.

MP ☐ MD ☐ PP ☐ E ☐ OPTION 2: SHALL COMPLY WITH THE APPLICABLE OSHPD PRE-APPROVAL (OPM #)

### SYMBOLS LIST

- F' FUSED DISCONNECT SWITCH
- ➡ DUPLEX CONVENIENCE OUTLET
- DOUBLE DUPLEX CONVENIENCE OUTLET
- GROUND FAULT CIRCUIT INTERRUPTER DUPLEX OUTLET GROUND FAULT CIRCUIT INTERRUPTER DOUBLE DUPLEX OUTLET
- SPECIAL OUTLET TO MATCH CAP PROVIDED WITH MACHINE
- FLUSH FLOOR BOX OR "POKE-THRU" UNIT EQUIPPED WITH FLUSH OR PEDESTAL DUPLEX RECEPTACLE AND VOICE/DATA OUTLETS
- AS NOTED, OR REFER TO SCHEDULE ON DRAWINGS.
- PLUGMOLD/WIREMOLD RECEPTACLE SYSTEM
- △ TRANSFORMER JUNCTION BOX, SIZE AS REQUIRED BY CODE
- FLEX CONNECTION TO FIXTURE
- PANELBOARD, RECESSED MOUNTED
- PANELBOARD, SURFACE MOUNTED
- MAIN SWITCHBOARD
- TERMINAL CABINET, RECESSED MOUNTED ☐ TERMINAL CABINET, SURFACE MOUNTED
- → HOMERUN TO PANELBOARD OR RESPECTIVE TERMINAL
- III CONDUIT RUN CONCEALED IN CEILING OR WALL, SEE SYMBOLS LIST NOTES
- ---- CONDUIT RUN UNDERGROUND OR UNDER FLOOR —EM— EMERGENCY SYSTEM CONDUIT AND WIRES
- INSULATED GREEN GROUND CONDUCTOR
- ——>>— INSULATED ISOLATED GROUND CONDUCTOR, GREEN WITH TRACER STRIPE -----O CONDUIT RISER
- - EXISTING EQUIPMENT, LIGHTING, DEVICES, CONDUIT, WIRING, ETC., ARE SHOWN LIGHT. NEW OR RELOCATED EQUIPMENT, LIGHTING, DEVICES, CONDUIT, WIRING, ETC., ARE SHOWN DARK.
- X X EXISTING ELECTRICAL EQUIPMENT TO BE REMOVED
- WIREMOLD SURFACE RACEWAY(S) WITH OUTLETS AS SHOWN OR NOTED, SEE SURFACE RACEWAY SCHEDULE
- (1) 1> SYMBOLS REFERRING TO KEYED NOTES ON SAME SHEET MECHANICAL EQUIPMENT BY OTHERS, CONNECTED BY ELECTRICAL CONTRACTOR
- DETAIL DESIGNATION, "A" SIGNIFIES DETAIL, "E-1" SIGNIFIES SHEET NUMBER

(1)1-1/2"C  $\leftarrow$  INDICATES SIZE OF CONDUIT = ONE AND ONE HALF INCH CONDUIT — NUMBER WITHIN PARENTHESIS INDICATES QUANTITY OF CONDUITS

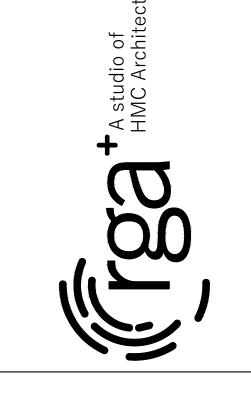
### SYMBOLS LIST NOTES:

- 1. MOUNT SWITCH BOXES AT +48" TO TOP OF BOX UNLESS OTHERWISE NOTED.
- 2. MOUNT OUTLET BOXES AT +15" TO BOTTOM OF BOX UNLESS OTHERWISE NOTED.
- 3. "A" ADJACENT TO OUTLET INDICATES OUTLET BOX TO BE MOUNTED ABOVE COUNTER. COORDINATE WITH COUNTER HEIGHT AND DEPTH PRIOR TO ROUGH IN. MOUNT OUTLET ABOVE COUNTERS AT: 3.1. +46" MAX TO TOP OF BOX WHERE BOX IS INSTALLED OVER BASE CABINET.
- 3.2. +44" MAX TO TOP OF BOX WITH OPEN COUNTERS WITH FORWARD APPROACH.
- 4. OUTLET BOXES SHALL BE: 4.1. WALL MOUNTED -4" SQ.  $\times 2-1/8$ " DEEP MINIMUM 4.2. CEILING MOUNTED -4" SQ. OR 4" OCT.  $\times 2-1/8$ " DEEP MINIMUM
- 5. OUTLET BOXES REQUIRING 1-1/4", 1-1/2" OR 2" CONDUITS SHALL BE 4-11/16" x 3-1/4" DEEP MINIMUM.
- 6. FLUSH MOUNTED OUTLET BOXES SHALL UTILIZE TRIM RINGS. COORDINATE TRIM RING DEPTH WITH WALL FINISH PRIOR TO ROUGH-IN.
- 7. NO CROSSBARS ON CONDUIT RUN INDICATES MINIMUM 1" CONDUIT, TWO #10 CU CONDUCTORS PLUS 1#10 CU GND. CROSSBARS INDICATE NUMBER OF #10 CU CONDUCTORS IN CONDUIT. CONDUCTOR SIZES OTHER THAN #10 NOTED ON DRAWINGS. INCREASE CONDUIT SIZE AS REQUIRED TO ACCOMMODATE C.E.C. WIRE FILL REQUIREMENTS. INCLUDE ADDITIONAL BOND WIRE IN ALL PVC AND FLEXIBLE CONDUIT. LONG CROSSBAR INDICATES NEUTRAL CONDUCTOR, SHORT CROSSBARS INDICATE PHASE CONDUCTORS.
- 8. INCREASE BRANCH CIRCUIT CU CONDUCTOR SIZES AS REQUIRED BY THE 120V BRANCH CIRCUIT VOLT DROP CONDUCTOR LENGTH CHART BELOW. USE CONDUCTOR LENGTHS AS FIELD MEASURED, BASED UPON MEASURED FIELD ROUTING LENGTHS. INCREASE MINIMUM CONDUIT SIZE AS REQUIRED TO ACCOMMODATE A MAXIMUM 40% CONDUCTOR FILL OF THE BRANCH CIRCUIT CONDUCTORS. WHERE NECESSARY, PROVIDE A JUNCTION BOX AT ACCESSIBLE CEILING SPACE TO CONVERT THE LAST 15 FEET OF CONDUCTORS TO #10 AWG TO ACCOMMODATE TERMINATION OF CONDUCTORS AT WIRING DEVICES, LIGHTING FIXTURES, CIRCUIT BREAKER, ETC.
- 9. INSTALL CU GROUND CONDUCTOR IN ALL BRANCH CIRCUITS FOR LIGHT FIXTURES AND POWER DEVICES.

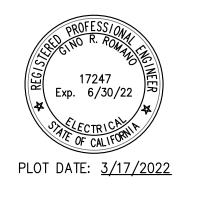
### 120V BRANCH CIRCUIT VOLT DROP CONDUCTOR LENGTH CHART

LOAD IN	LENGTH OF CONDUCTOR							
VOLT		WIRE	SIZE IN (G	AUGE)				
AMPERES	#12	#10	#8	#6	#4			
1200VA	74	121	183	284	434			
1560VA	57	93	141	218	334			
1800VA	49	81	122	189	289			
1920VA	46	76	115	178	271			
2340VA	Х	62	94	146	223			
2880VA	Х	51	76	118	181			
3000VA	Х	48	73	114	174			
3900VA	Х	Χ	56	87	134			
4800VA	Х	Χ	46	71	108			

- 1. THIS CHART IS FOR COPPER CONDUCTORS ONLY.
- THIS CHART ASSUMES AN 80% POWER FACTOR AND STEEL RACEWAYS. 3. 2019 CALIFORNIA ENERGY CODE, 130.5(c) ALLOWS A MAXIMUM COMBINED VOLTAGE DROP OF 5%. THIS CHART ASSUMES A MAXIMUM DROP OF 3% FOR FEEDERS. THIS CHART PROVIDES THE MAXIMUM LENGTH OF CONDUCTORS FOR LESS THAN 2% VOLTAGE DROP ON A BRANCH
- CIRCUIT AT GIVEN VA LOAD. 4. USE WIRE SIZE FROM THIS CHART UNLESS LARGER CONDUCTOR SIZES ARE NOTED ON THE
- 5. FOR VA VALUES NOT SHOWN USE NEXT HIGHEST VALUE FROM THE CHART







### S < 0 ED R H $\vdash \omega$ CIT TRU 0

Revision

S E

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SYMBOLS, NOTES

E0.1

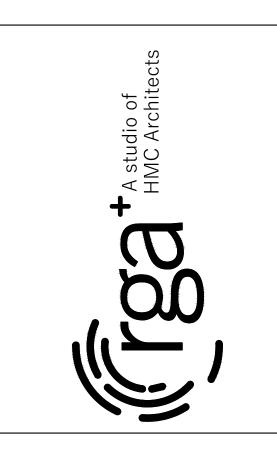
# 22ND AVENUE (E) 'MSB' ——**—** (M) PANEL '22' (2)

- SHEET NOTES:

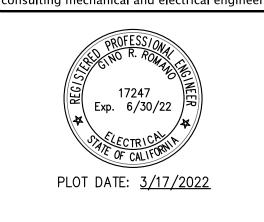
  ALL EXISTING EQUIPMENT, DEVICES, CONDUIT AND WIRING, ETC., SHOWN ON PLANS ARE BASED ON AVAILABLE EXISTING DRAWINGS AND LIMITED SITE SURVEYS, AND SHOWN FOR CLARITY ONLY.

  2. SEE ONE LINE DIAGRAM AND PANEL SCHEDULE ON SHEET <u>E2.1</u> FOR REFERENCE.
- KEYED NOTES:

  1 PROVIDE TRENCH FOR 24 INCH MINIMUM COVER. LOCATE AND PROTECT (E)
  UTILITIES, I.E. IRRIGATION, SEWER, DRAINAGE PIPES, ETC. SAW CUT AND PATCH BACK (E) ASPHALT. PROVIDE SAND TO COVER CONDUIT TO SIX(6) INCHES, THEN ADD TRACER TAPE. COMPLETE BACKFILL TO GRADE WITH NATIVE SOIL. COMPACT IN SIX(6) LIFTS. FINISH TO MATCH EXISTING. SEE DETAIL **3/E3.1**.
- DROP CONDUIT TO BELOW ASPHALT AND PROVIDE CHRISTY N9 PULL BOX WITHIN FIVE(5) FT OF SHADE STRUCTURE. TRENCH TO SHADE LOCATION, INTERCEPTING THE CHRISTY BOX ALONG THE WAY. CHRISTY BOX TO HAVE HOLD DOWN BOLTS AND BE LABELED FOR POWER. PAINT EXPOSED CONDUIT TO MATCH (E) FINISH.
- 3 PROVIDE AT MINIMUM TWO(2) GROUND RODS, EACH 5/8" BY TEN(10) FEET LONG, CU, AT LEAST TEN(10) FEET APART. BOND TO METAL OF SHADE STRUCTURE. SEE DETAIL <u>5/E3.1</u>.
- 4 LOCKABLE, WEATHERPROOF RECEPTACLE TO HAVE A TWO-GANG BACK BOX WITH 1" THREADED PORT. MOUNT RECEPTACLES 36" ABOVE GRADE UNLESS SPECIFIED OTHERWISE. SEE DETAIL 4/E3.1.







SHADE STRUCTURE ELEMENTARY SCHO Revision

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SITE PLAN -ELECTRICAL

E1.1

SITE PLAN - ELECTRICAL

SCALE: 1"-20"

1000	WIDTH: DEPTH: DIRECTORY  EMOVE THIS K.O.	100% AØ 1200	BØ	
AØ BØ DIRECTORY BRKR CKT CKT BRKR  MAIN  " - 3 • 4 "  1000 PHOTO CELL  A LIGHT  B LIGHT  360 RECEPTS - SHADE STRUCT. [5]  SPACE  PFB 13 • 14 60/2 HVAC	DIRECTORY MOVE THIS K.O.		BØ	
MAIN  " - 3 • 4 "  1000  PHOTO CELL  A LIGHT  B LIGHT  B LIGHT  B LIGHT  B LIGHT  CONTROL  20/1 5 • 6 20/1 REC  20/1 7 • 8 20/1 REC  20/1 7 • 8 20/1 REC  20/1 9 • 10 20/1 24 HR TIME  20/1 11 • 12 PFB SPACE  SPACE  SPACE  PFB 13 • 14 60/2 HVAC	EMOVE THIS K.O.		BØ	
""       -       3       •       4       ""         1000       PHOTO CELL A LIGHT       20/1       5       •       6       20/1       REC         B LIGHT       20/1       7       •       8       20/1       REC         B LIGHT       20/1       9       •       10       20/1       24 HR TIME         RECEPTS - SHADE STRUCT. [5]       20/1       11       •       12       PFB       SPACE         SPACE       PFB       13       •       14       60/2       HVAC		1200		
1000 PHOTO CELL A LIGHT 20/1 5 • 6 20/1 REC 20/1 7 • 8 20/1 REC B LIGHT 20/1 9 • 10 20/1 24 HR TIME 360 RECEPTS - SHADE STRUCT. [5] 20/1 11 • 12 PFB SPACE  SPACE PFB 13 • 14 60/2 HVAC	:R	1200		
A LIGHT 20/1 7 • 8 20/1 REC  B LIGHT 20/1 9 • 10 20/1 24 HR TIME  360 RECEPTS - SHADE STRUCT. [5] 20/1 11 • 12 PFB SPACE  SPACE PFB 13 • 14 60/2 HVAC	:R	1200		
B LIGHT  360  RECEPTS - SHADE STRUCT. [5]  SPACE  20/1  9 • 10 20/1 24 HR TIME  20/1  11 • 12 PFB SPACE  PFB 13 • 14 60/2 HVAC	:R			
360         RECEPTS - SHADE STRUCT. [5]         20/1         11         •         12         PFB         SPACE           SPACE         PFB         13         •         14         60/2         HVAC	R		1200	
SPACE         PFB         13         ●         14         60/2         HVAC		1000		
	SPACE			
ODACE DED 45 140 III		4160		
SPACE   PFB   15   •   16   -  "			4160	
NEW LOAD DEMAND READINGS PEAK DEMAND @	) 125% + (N) LOAD	TOTAL D	EMAND	
TOTAL PANEL VA AMPS AMPS @125% AMPS	VA	LOA	∤D	
AØ = 7360 VA 61.3 6.8 8.5 69.8 A	8380 VA	15180	VA	
BØ = 5720 VA 47.7 7.2 9.0 56.7 A	6800 VA	69.8	AMPS	

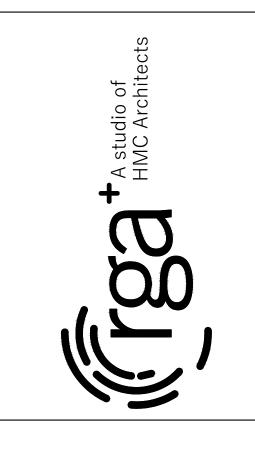
		Vo	oltage	e Drop	Calcu	llatio	ns C	opp	er		
Job Name:	Mark Twair	n Elementa	ry School	- Shade Stru	ıcture					Job #:	22.020
Date:	3/10/2022										
	_				_	•			_		
	VOLTAGE:	120	PHASE:	1		POWER	FACTOR:	80%	CONDUIT:	S	teel
	_		•		•		•		·		
FEEDER	AMPS AT	KVA	VOLTS	DISTANCE	DISTANCE	WIRES/	LOAD/	WIRE	WIRE	VOLTS	PERCENT
NUMBER	LOAD	TOTAL	AT LOAD	FEET	TOTAL	PHASE	WIRE	SIZE	FACTOR	DROP	VOLT DROP
RECEPT-1	3.0	0.4	119.76	40	40	1	3.00	10	1995	0.24	0.20%
RECEPT-2	1.5	0.2	119.59	56	96	1	1.50	10	1995	0.41	0.34%

SHEET NOTES:

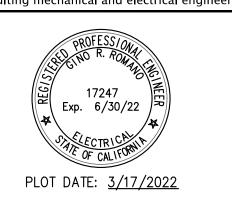
1. ALL EXISTING EQUIPMENT, DEVICES, CONDUIT AND WIRING, ETC., SHOWN ON PLANS ARE BASED ON AVAILABLE EXISTING DRAWINGS AND LIMITED SITE SURVEYS, AND SHOWN FOR CLARITY ONLY.

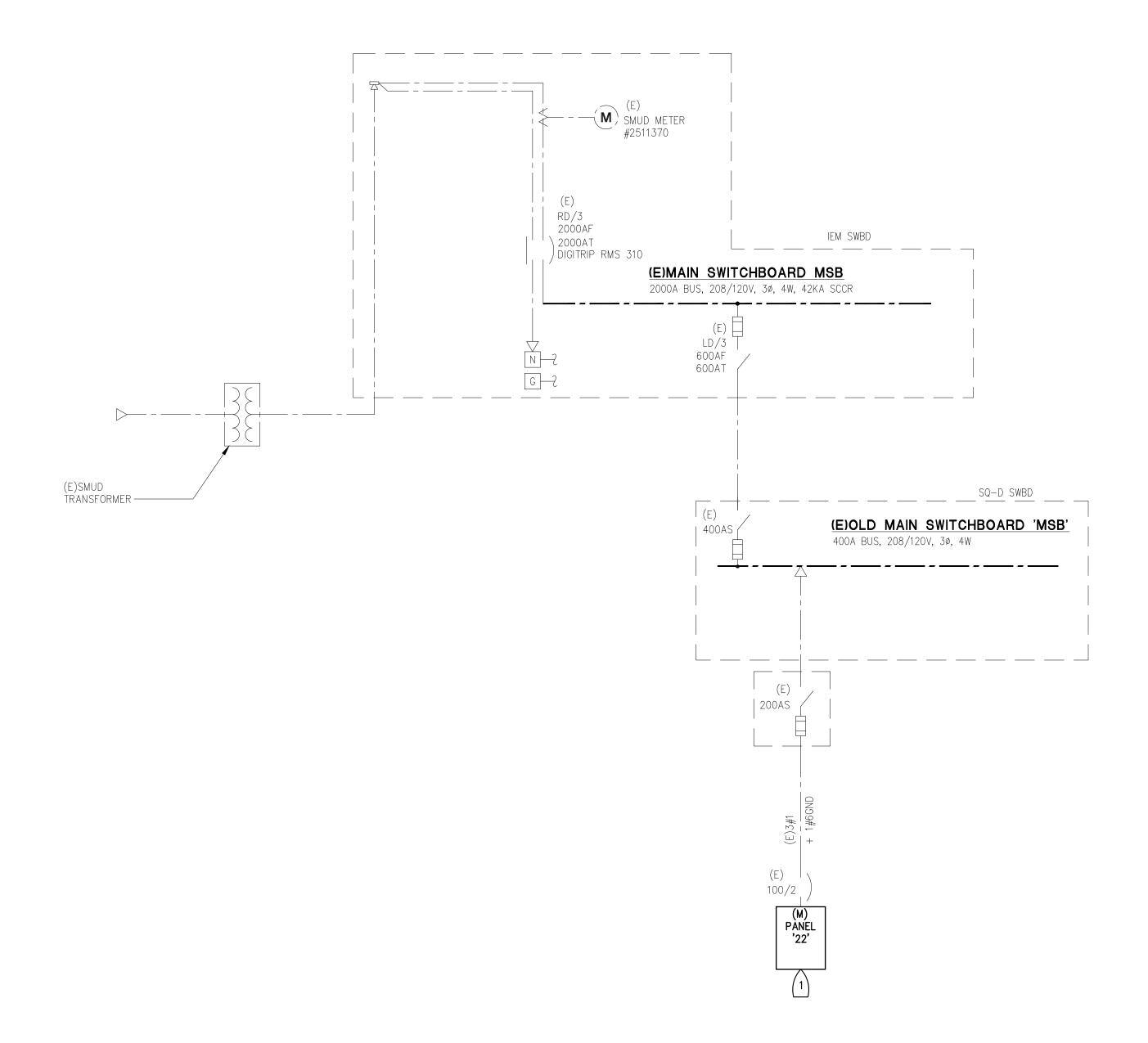
KEYED NOTES:

MODIFIED PANEL SERVES EQUIPMENT BEING ADDED IN THIS PROJECT. SEE PANEL SCHEDULE ON THIS SHEET FOR REFERENCE.









SHADE STRUCTURE ELEMENTARY SCHO Revision

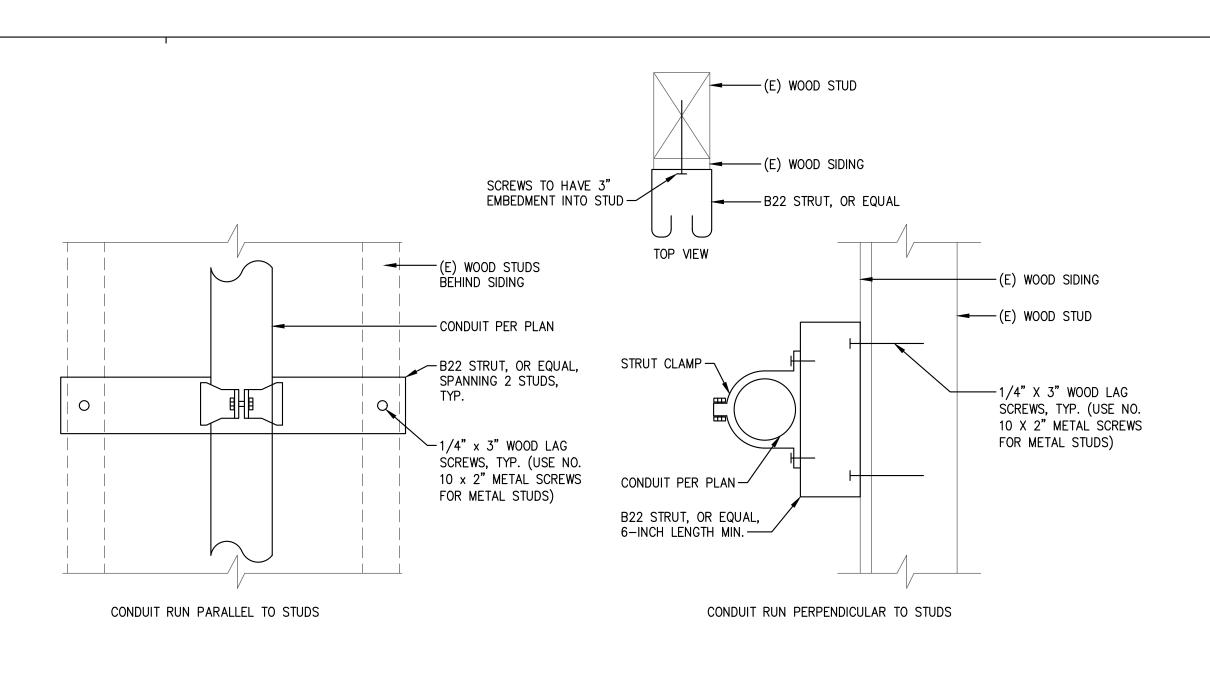
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ONE LINE DIAGRAM

E2.1

1504.14 3/21/2022

ONE LINE DIAGRAM	PROJECT N
SCALE: NONE	DATE:
	SHEET



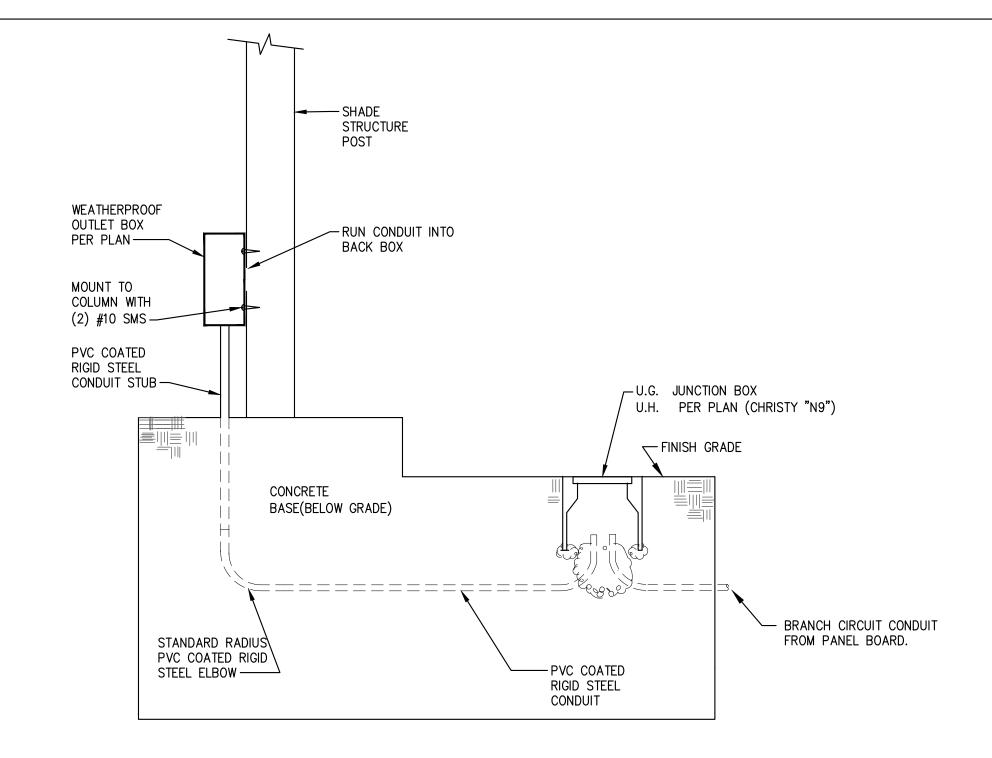
NOTES:

1. CONDUIT SHALL BE SUPPORTED AT INTERVALS NOT EXCEEDING TEN(10)
FEET AND NOT MORE THAN THREE(3) FEET FROM THE OUTLET AND AT
ANY POINT WHERE IT CHANGES DIRECTION.

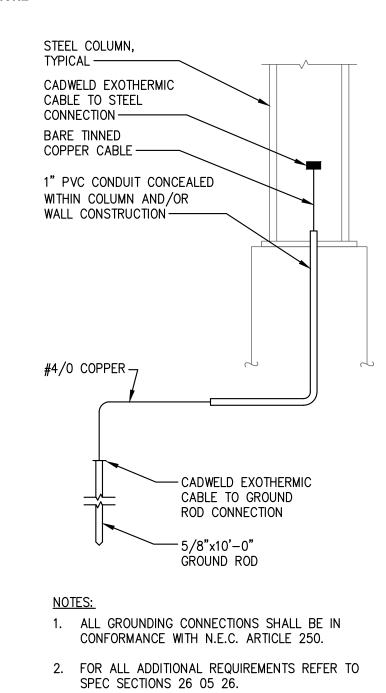
2. PERFORATED STRAP AND PLUMBER'S TAPE SHALL NOT BE PERMITTED.

3. MAXIMUM CONDUIT AND CONDUCTOR WEIGHT IS 1.83LBS PER LINEAR FOOT.

1 CONDUIT MOUNTING DETAIL - STUD WALLS

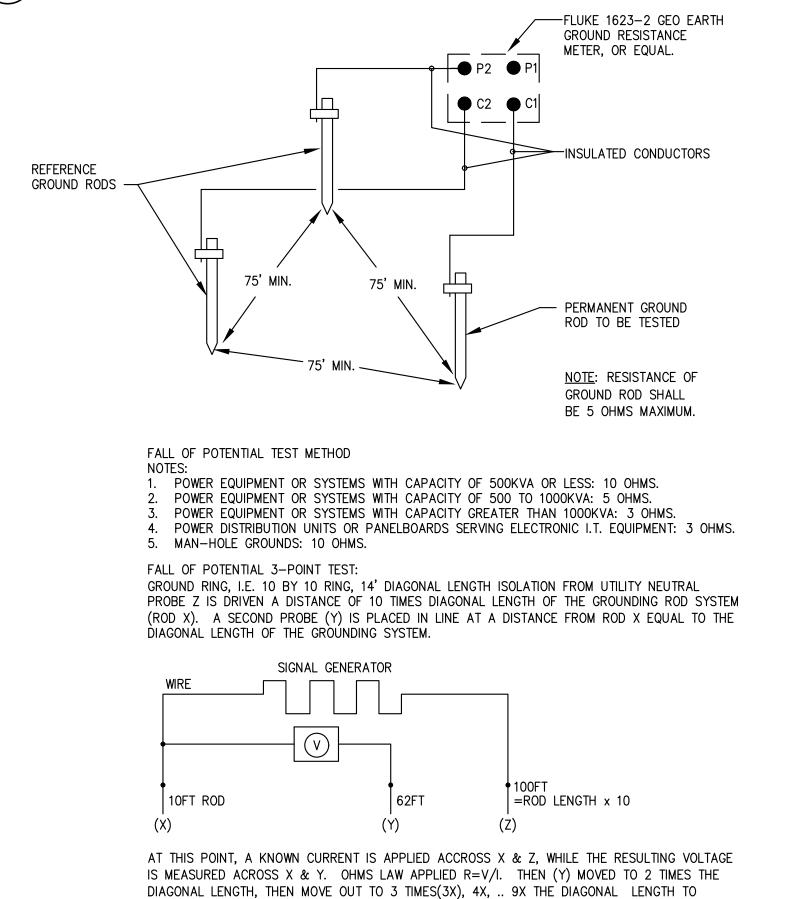


### 4 CONDUIT STUB IN POST DETAIL SCALE: NONE



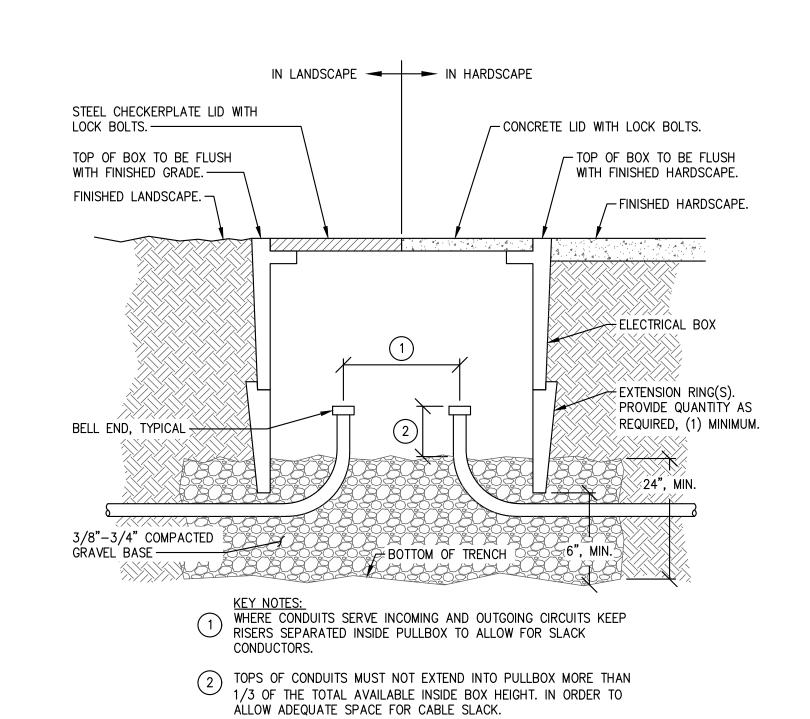
### TYPICAL STEEL COLUMN

### & REBAR GROUNDING DETAIL SCALE: NONE

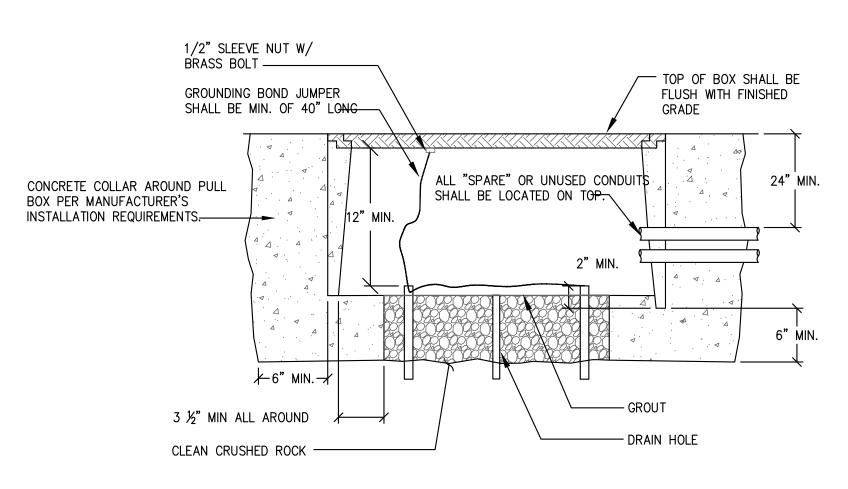


### 6 METHOD OF TESTING GROUND RODS DETAIL SCALE: NONE

COMPLETE THE 3 POINT TEST WITH A TOTAL OF NINE RESISTANCE MEASUREMENTS.



### 1 NON-TRAFFIC RATED PULL BOX SCALE: NONE

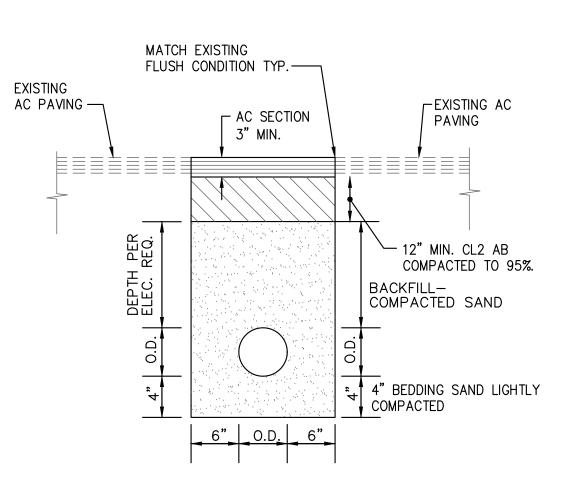


NOTES:

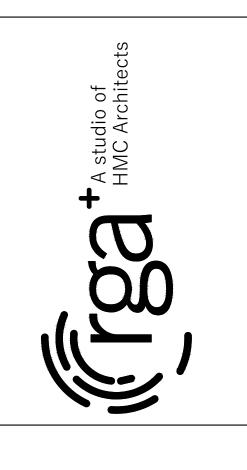
1. HANDHOLES SHALL BE PROVIDE WITH A MINIMUM OF (4) GALVANIZED PULLING PLATES IN BOTTOM OF PULLBOX.

- 2. PULLBOXES SHALL BE PROVIDED WITH CAST IN PLACE VERTICAL CABLE RACKS. ALL CABLES SHALL BE NEATLY BUNDLED, ORGANIZED AND SUPPORTED BY CABLE RACKS.
- 3. WHERE ADDITIONAL CONDUIT ENTRIES ARE REQUIRED BEYOND QUANTITY OF TERMINATORS SHOWN. CONTRACTOR SHALL FIELD CORE DRILL AS REQUIRED. WHERE 4" TERMINATORS ARE PROVIDED CONTRACTOR SHALL PROVIDE CONDUIT REDUCERS TO MATCH SITE CONDUIT SIZE REQUIREMENTS.
- 4. FOR ALTERNATE STYLE PULLBOXES CONTRACTOR SHALL FIELD CORE DRILL ALL CONDUIT ENTRIES 2" DIA
- 5. CONTRACTOR SHALL PROVIDE THE MANUFACTURER'S INSTALLATION INSTRUCTIONS FOR TRAFFIC RATING REQUIREMENTS AS PART OF THE SUBMITTALS.

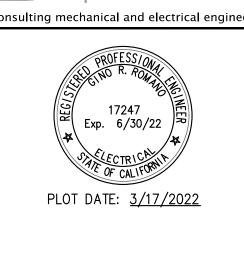
### 2 TRAFFIC RATED PULL BOX SCALE: NONE



3 TYPICAL TRENCH DETAIL
SCALE: NONE







## TRUCTURE AT MARK TWAII

SHADE STRUCTI

ELEMENTARY SO

Besieve the service of the service o

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DETAILS

PROJECT NO. 1504.14

DATE: 3/21/2022

SHEET

E3.1

SOCF DEAD LOAD (SUPERIMPOSED ON FRAME)   5 PSF	
ROOF LIVE LOAD         20 P           ROOF DEAD LOAD (SUPERIMPOSED ON FRAME)         5 PSF           ROOF PANEL DEAD LOAD         M=1.1 PSF, G=1.2           COLLATERAL DEAD LOAD         M=3.9 PSF, G=3.1           ROOF SNOW LOAD.         20 P           SROUND SNOW LOAD. Pg         20 P           ROOF SNOW LOAD: SLOPED, Pg         20 P           SITE APPLICATION DSA REVIEWER SHALL VERIFY THE STRUCTURE BE LOCATED AT LEAST 20 FEET FROM A SNOW LOAD LOPE FACTOR, Cg         11.6           SNOW EXPOSURE FACTOR, Cg         11.6           SNOW LOAD INPORTANCE FACTOR, Ig         1.1           HERRINA FACTOR, Cg         1.1           SASIC WIND SPEED (3 SECOND GUST), V <sub>cl</sub> 10.0           RISK CATEGORY         0           SASIC WIND SPEED (3 SECOND GUST), V <sub>cl</sub> 10.0           RISK CATEGORY         0           EXPOSURE CATEGORY         0           CACTORS, Kg, Kg, Sc         0.855, 1           Rg DOWER PASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED         CASE A (1.1 / -1.2) G           Cow, PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED         CASE A (0.6 / -0.9)           COMPONENTS & CLADDING - Cg (1 PRESSURE/SUCTION) CLEAR / OBSTRUCTED         CASE A (0.6 / -0.9)           COMPER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED         CASE A (0.6 /	VALUES
SOSP EAD LOAD (SUPERIMPOSED ON FRAME)   5 PSF	DOE
ROOF PANEL DEAD LOAD	_
COLLATERAL DEAD LOAD   ROOF SNOW LOAD	
ROOF SNOW LOAD   ROOF SNOW LOAD   ROOF SNOW LOAD   ROOF SNOW LOAD   ROOF SNOW LOAD SLOPED, P.   20 P   RISK CATEGORY   ■   ■   ROOF SNOW LOAD SLOPED, P.   20 P   STITE APPLICATION DSA REVIEWER SHALL VERIFY THE STRUCTURE BE LOCATED AT LEAST 20 FEET FROM A SNOW LOAD SLOPE FACTOR, C.   1.0   1	
I   ROBERT   ROBER	,
ROOF SNOW LOAD: SLOPED, P <sub>8</sub> STE APPLICATION DSA REVIEWER SHALL VERIFY THE STRUCTURE BE LOCATED AT LEAST 20 FEET FROM A SNOW LOAD SLOPE FACTOR, C <sub>9</sub> 1.6  SNOW LOAD SLOPE FACTOR, C <sub>9</sub> 1.7  SNOW LOAD SLOPE FACTOR, C <sub>9</sub> 1.8  SNOW LOAD IMPORTANCE FACTOR, I <sub>8</sub> 1.9  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 100 M  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 100 M  SISK CATEGORY  1.8  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 100 M  SISK CATEGORY  1.8  CATORS: K <sub>2</sub> , K <sub>2</sub> , K <sub>3</sub> 1.9  1.9  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC	PSF
SITE APPLICATION DSA REVIEWER SHALL VERIFY THE STRUCTURE BE LOCATED AT LEAST 20 FEET FROM A SNOW LOAD SLOPE FACTOR, C <sub>0</sub> 1.0.  SNOW EXPOSURE FACTOR, C <sub>0</sub> 1.0.  SNOW EXPOSURE FACTOR, C <sub>0</sub> 1.1.  SNOW EXPOSURE FACTOR, C <sub>0</sub> 1.1.  SNOW EXPOSURE FACTOR, C <sub>0</sub> 1.2.  SNOW EXPOSURE FACTOR, C <sub>0</sub> 1.3.  SNOW EXPOSURE FACTOR, C <sub>0</sub> 1.4.  SNOW EXPOSURE FACTOR, C <sub>0</sub> 1.5.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.6.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.7.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.8.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.9.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.	1
SNOW EXPOSURE FACTOR, C <sub>e</sub> 1.1. SNOW LOAD IMPORTANCE FACTOR, I <sub>e</sub> 1.2. THERMAL FACTOR, C <sub>1</sub> 3.  WIND DESIGN  BASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.  BASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.  BASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.  1.  1.  1.  1.  1.  1.  1.  1.  1	PSF
SNOW EXPOSURE FACTOR, C <sub>0</sub> 1.1.  SNOW LOAD IMPORTANCE FACTOR, I <sub>k</sub> 1.1.  THERMAL FACTOR, C <sub>1</sub> BASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 100 M  RISK CATEGORY  CACTORS: K <sub>0</sub> , K <sub>th</sub> , K <sub>d</sub> Question (3 M  Question (4 M  Question	ADJACENT STRUCTURE
SENOW LOAD IMPORTANCE FACTOR, I <sub>1</sub> THERMAL FACTOR, C <sub>1</sub> MIND DESIGN  BASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 100 M  RISK CATEGORY  RISK CATEGORY  CEXPOSURE CATEGORY  COACTION  A <sub>1</sub> = 0.00256 K, K <sub>1</sub> , K <sub>2</sub> K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>4</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>4</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>4</sub> B <sub>5</sub> = 0.00256 K, K <sub>4</sub> B <sub>4</sub> = 0.00256 K, K <sub>4</sub> B <sub>5</sub> = 0.00256 K, K <sub>4</sub> B <sub>4</sub> =	.0
THERMAL FACTOR, C, WIND DESIGN  BASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> . 100 M RISK CATEGORY   II  EXPOSURE CATEGORY   II  EXPOSURE CATEGORY   C  FACTORS, K <sub>x</sub> , K <sub>x</sub> , K <sub>x</sub>   0.85, 1  g <sub>x</sub> = 0.00256 K, K <sub>x</sub> , K <sub>x</sub>   0.85, 1  g <sub>x</sub> = 0.00256 K, K <sub>x</sub> , K <sub>x</sub>   0.85, 1  g <sub>x</sub> = 0.00256 K, K <sub>x</sub>   0.80 F ANGLE 18.43 - CLEAR / OBSTRUCTED   CASE A (1.1 /-1.2)    C <sub>ML</sub> PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED   CASE A (0.17 /-1.09)    C <sub>ML</sub> PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED   CASE A (-0.17 /-1.09)    C <sub>ML</sub> PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED   CASE A (-0.17 /-1.09)    COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED   CONE 3 - (0.29 /-  ZONE 2 - (1.77 /-  ZONE 1 - (1.15 /-  SEISMIC DESIGN   STEEL - ORDINARY CA  ANALYSIS PROCEDURE   STEEL - ORDINARY CA  SEISMIC IMPORTANCE FACTOR, Ie   EQUIVALENT LA  SEISMIC SITE CLASS   D  MCE <sub>R</sub> SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>3</sub>   0.9  SHORT PERIOD SITE COEFFICIENT, F <sub>a</sub>   1.2  LONG PERIOD COEFFICIENT, F <sub>a</sub>   1.7  FUNDAMENTAL PERIOD OF THE STRUCTURE, T   0.15  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> - USED   2.08 * 0.70  TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3)   1.0  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub>   1.0  SEISMIC DESIGN CATEGORY   1.2  RESPONSE MODIFICATION FACTOR, R   1.2  OVERSTRENGTH FACTOR, Ω   1.2  RESDUNDANCY FACTOR, Ω   1.2  RESD	.0
THERMAL FACTOR, C, WIND DESIGN  BASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> . 100 M RISK CATEGORY   II  EXPOSURE CATEGORY   II  EXPOSURE CATEGORY   C  FACTORS, K <sub>x</sub> , K <sub>x</sub> , K <sub>x</sub>   0.85, 1  g <sub>x</sub> = 0.00256 K, K <sub>x</sub> , K <sub>x</sub>   0.85, 1  g <sub>x</sub> = 0.00256 K, K <sub>x</sub> , K <sub>x</sub>   0.85, 1  g <sub>x</sub> = 0.00256 K, K <sub>x</sub>   0.80 F ANGLE 18.43 - CLEAR / OBSTRUCTED   CASE A (1.1 /-1.2)    C <sub>ML</sub> PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED   CASE A (0.17 /-1.09)    C <sub>ML</sub> PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED   CASE A (-0.17 /-1.09)    C <sub>ML</sub> PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED   CASE A (-0.17 /-1.09)    COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED   CONE 3 - (0.29 /-  ZONE 2 - (1.77 /-  ZONE 1 - (1.15 /-  SEISMIC DESIGN   STEEL - ORDINARY CA  ANALYSIS PROCEDURE   STEEL - ORDINARY CA  SEISMIC IMPORTANCE FACTOR, Ie   EQUIVALENT LA  SEISMIC SITE CLASS   D  MCE <sub>R</sub> SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>3</sub>   0.9  SHORT PERIOD SITE COEFFICIENT, F <sub>a</sub>   1.2  LONG PERIOD COEFFICIENT, F <sub>a</sub>   1.7  FUNDAMENTAL PERIOD OF THE STRUCTURE, T   0.15  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> - USED   2.08 * 0.70  TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3)   1.0  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub>   1.0  SEISMIC DESIGN CATEGORY   1.2  RESPONSE MODIFICATION FACTOR, R   1.2  OVERSTRENGTH FACTOR, Ω   1.2  RESDUNDANCY FACTOR, Ω   1.2  RESD	.0
WIND DESIGN           BASIC WIND SPEED (3 SECOND GUST), V <sub>ol</sub> R         100 M           RISK CATEGORY         0.85, 1           EXPOSURE CATEGORY         0.85, 1           10 <sub>a</sub> = 0.00256 K <sub>x</sub> K <sub>xt</sub> V <sub>x</sub> FOR ALL EAVE HEIGHTS (8', 10' & 12')         18.50           0.2 <sub>MV</sub> PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED         CASE A (1.1 / -1.2)           0.8 <sub>MV</sub> PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED         CASE A (-0.17 / -1.09)           0.2 <sub>MV</sub> PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED         CASE A (-0.6 / -0.9)           0.00MPONENTS & CLADDING - C <sub>N</sub> (PRESSURE/SUCTION) CLEAR / OBSTRUCTED         CASE A (-0.6 / -0.9)           0.00MPONENTS & CLADDING - C <sub>N</sub> (PRESSURE/SUCTION) CLEAR / OBSTRUCTED         ZONE 3 - (2.29 / -2.20	2
BASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> RISK CATEGORY  RISK CATEGORY  CATEGORY  CASEA (-0.85, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1,	
RISK CATEGORY  EXPOSURE CATEGORY  C EXPOSURE CATEGORY  C DATE OF THE EXPOSURE CATEGORY  RESPONSE MODIFICATION FACTOR, ρ  RESPONSE MODIFICATION FOR TICK. 20' WIDE, 30' WIDE, 40' WIDE)  1.10 DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  1.11 DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  1.11 DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  1.12 DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  1.14 DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  1.15 DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  1.16 DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  1.17 PENDAMENTAL OF CATEGORY  C DASS A (-0.0 f, 0.15)  C ASE A (-0.1 f, -1.2)	MPH
EXPOSURE CATEGORY FACTORS: K <sub>2</sub> , K <sub>2</sub> , K <sub>3</sub> 0.85, 1  The = 0.00256 K <sub>2</sub> K <sub>2</sub> K <sub>4</sub> V <sub>2</sub> Por All EAVE HEIGHTS (8', 10' & 12') 1.9, = 0.00256 K <sub>2</sub> K <sub>2</sub> K <sub>4</sub> V <sub>2</sub> Por All EAVE HEIGHTS (8', 10' & 12') 1.9, = 0.00256 K <sub>3</sub> K <sub>4</sub> K <sub>4</sub> V <sub>2</sub> Por All EAVE HEIGHTS (8', 10' & 12') 1.9, PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED 1.0, PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED 1.0, PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED 1.0, PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED 1.0, PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED 1.0, PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED 1.0, PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED 1.0, PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED 1.0, PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED 1.0, PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED 1.0, PER SUBJECT OBSTRUCTER, TO 1.1, PER SUBJECT OBSTRUCTED 1.0, PER SUBJECT OBSTRUCTER, TO 1.1, PER SUBJECT OBSTRUCTED 1.0, PER SUBJECT OBSTRUCTURE, TO 1.1, PER SUBJECT OBSTRUCTED 1.0, PER SUBJECT OBSTRUCTURE, TO 1.1, PER SUBJECT OBSTRUCTED	
FACTORS: K <sub>2</sub> , K <sub>2</sub> , K <sub>3</sub> Q <sub>1</sub> = 0.00256 K <sub>2</sub> K <sub>2</sub> , K <sub>3</sub> V <sup>2</sup> FOR ALL EAVE HEIGHTS (8', 10' & 12')  18.50 C <sub>MW</sub> PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED CASE A (1.1 / -1.2) C <sub>ML</sub> PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED CASE A (-0.17 / -1.09) C <sub>M</sub> PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLEAR - (OBSTRUCTED COMPONENTS & CLAD - (OBSTRUCTED COMPONENTS & CLACE COMPONENTS & COMPONENTS	
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	1, 0.85
C <sub>MM</sub> PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED         CASE A (-0.17 /-1.09)           C <sub>NL</sub> PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED         CASE A (-0.17 /-1.09)           C <sub>N</sub> PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED         CASE A (-0.6 /-0.9)           COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED         ZONE 3 - (2.29 /-2           ZONE 2 - (1.77 /-1         ZONE 1 - (1.15 /-1           SEISMIC DESIGN         ZONE 1 - (1.15 /-1           LATERAL FORCE RESISTING SYSTEM         STEEL - ORDINARY C/A           ANALYSIS PROCEDURE         EQUIVALENT LA           SEISMIC IMORTANCE FACTOR, Ie         11.0           SEISMIC SITE CLASS         D           MCE <sub>R</sub> SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>s</sub> 2.6           MCE <sub>R</sub> SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>s</sub> 2.6           MCE <sub>R</sub> SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>s</sub> 1.2           LONG PERIOD COEFFICIENT, F <sub>s</sub> 1.2           LONG PERIOD COEFFICIENT, F <sub>s</sub> 1.2           DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> - USED         2.08 * 0.70           DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> - USED         2.08 * 0.70           DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> 1.0           DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-	
$C_{NL} \ PER \ ASCE FIGURE 27.4-5 \ ROOF \ ANGLE 18.43 - CLEAR / OBSTRUCTED                                    $	
$ \begin{array}{c} C_N  \text{PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED} & CASE A  (-0.6  /  -0.9) \\ \hline COMPONENTS & CLADDING - C_N  (  \text{PRESSURE/SUCTION})  \text{CLEAR / OBSTRUCTED}} & ZONE  3 -  (2.29  /  \cdot  20  \text{Ne}  2 -  (1.77  /  -  20  /  -  20  \text{NE}  2 -  (1.28  /  -  20  ,  -  20  \text{NE}  2 $	· · · · · · · · · · · · · · · · · · ·
COMPONENTS & CLADDING - $C_N$ ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED  ZONE 3 - $(2.29)$ / $(2.29)$ / $(2.20)$	
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	· · · · · · · · · · · · · · · · · · ·
$ \begin{array}{c} \text{ZONE 1 - } 1.15 I - \\ \text{SEISMIC DESIGN} \\ \\ \text{LATERAL FORCE RESISTING SYSTEM} \\ \text{STEEL - ORDINARY CAMALYSIS PROCEDURE} \\ \text{SESIMIC IMORTANCE FACTOR, Ie} \\ \text{SEISMIC SITE CLASS} \\ \text{MCE}_R \text{ SPECTRAL RESPONSE ACCELERATION @ 0.2 \text{ s, S}_S } \\ \text{2.6} \\ \text{MCE}_R \text{ SPECTRAL RESPONSE ACCELERATION @ 0.2 \text{ s, S}_1 } \\ \text{3.1} \\ \text{3.2} \\ \text{4.1} \\ \text{5.2} \\ \text{5.2} \\ \text{5.3} \\ \text{5.4} \\ \text{5.2} \\ \text{6.4} \\ \text{6.2} \\ \text{6.2} \\ \text{6.2} \\ \text{6.2} \\ \text{6.2} \\ \text{6.2} \\ \text{6.3} \\ \text{6.2} \\ \text{6.2} \\ \text{6.3} \\ \text{6.2} \\ \text{6.2} \\ \text{6.3} \\ \text{6.2} \\ \text{6.3} \\ \text{6.2} \\ \text{6.3} \\ \text{6.3} \\ \text{6.4} \\ \text{6.2} \\ \text{6.2} \\ \text{6.2} \\ \text{6.2} \\ \text{6.3} \\ \text{6.2} \\ \text{6.3} \\ \text{6.3} \\ \text{6.4} \\ \text{6.2} \\ \text{6.2} \\ \text{6.3} \\ \text{6.2} \\ \text{6.3} \\ \text{6.3} \\ \text{6.3} \\ \text{6.3} \\ \text{6.4} \\ \text{6.4} \\ \text{6.2} \\ \text{6.2} \\ \text{6.4} \\ \text{6.2} \\ \text{6.4} \\ 6.4$	
LATERAL FORCE RESISTING SYSTEM  ANALYSIS PROCEDURE  EQUIVALENT LA SESIMIC IMORTANCE FACTOR, Ie  SEISMIC SITE CLASS  D $MCE_R$ SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>S</sub> $C.6$ $MCE_R$ SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>1</sub> $C.6$ SHORT PERIOD SITE COEFFICIENT, F <sub>a</sub> $C.6$ LONG PERIOD COEFFICIENT, F <sub>v</sub> $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $C.6$ DESIGN CATEGORY  EQUIVALENT LA  DESIGN CATEGORY  EQUIVALENT LA  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $C.6$	-1.05) / (0.5 / -1.5)
ANALYSIS PROCEDURE  SESIMIC IMORTANCE FACTOR, le  1.6  SEISMIC SITE CLASS $MCE_R$ SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>S</sub> 2.6 $MCE_R$ SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>S</sub> 3.7  SHORT PERIOD SITE COEFFICIENT, F <sub>a</sub> 1.7  FUNDAMENTAL PERIOD OF THE STRUCTURE, T  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> 2.0  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> - USED  TO DETERMINE CS (WITH CAP PER ASCE-7 12.8.1.3)  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> 1.0  SEISMIC DESIGN CATEGORY  E  RESPONSE MODIFICATION FACTOR, R  1.2  OVERSTRENGTH FACTOR, $\Omega$ 1.2  REDUNDANCY FACTOR, $\rho$ 1.4  HORIZONTAL OR VERTICAL IRREGULARITIES  SEISMIC RESPONSE COEFFICIENT, CS (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  1.2.73 PSF, 13.41	ANTILEVER COLUMN
SEISMIC SITE CLASS $MCE_R$ SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>S</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>S</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> - USED $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> - USED $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S <sub>D2</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S <sub>D2</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S <sub>D2</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S <sub>D2</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S <sub>D2</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S <sub>D2</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S <sub>D2</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S <sub>D2</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S <sub>D2</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S <sub>D2</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT S	
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	.0
MCE <sub>R</sub> SPECTRAL RESPONSE ACCELERATION @ $0.2 \text{ s}$ , $S_1$ SHORT PERIOD SITE COEFFICIENT, $F_a$ LONG PERIOD COEFFICIENT, $F_v$ 1.7  FUNDAMENTAL PERIOD OF THE STRUCTURE, T  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ 2.0  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ - USED  TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3)  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, $S_{D1}$ 1.0  SEISMIC DESIGN CATEGORY  ERSPONSE MODIFICATION FACTOR, R  OVERSTRENGTH FACTOR, $D$ 1.2  REDUNDANCY FACTOR, $D$ 1.4  HORIZONTAL OR VERTICAL IRREGULARITIES  SEISMIC RESPONSE COEFFICIENT, Cs ( $D_{CO}$ WIDE, $D_{CO}$ W	)
SHORT PERIOD SITE COEFFICIENT, $F_a$ LONG PERIOD COEFFICIENT, $F_v$ 1.7  FUNDAMENTAL PERIOD OF THE STRUCTURE, T  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ 2.0  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ - USED  TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3)  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, $S_{D1}$ 1.0  SEISMIC DESIGN CATEGORY  ERSPONSE MODIFICATION FACTOR, R  OVERSTRENGTH FACTOR, $\Omega$ 1.2  REDUNDANCY FACTOR, $\rho$ HORIZONTAL OR VERTICAL IRREGULARITIES  NON  SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  12.73 PSF, 13.41	60
LONG PERIOD COEFFICIENT, $F_V$ FUNDAMENTAL PERIOD OF THE STRUCTURE, T  0.15.  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ 2.0  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ - USED  TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3)  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, $S_{D1}$ 1.0  SEISMIC DESIGN CATEGORY  ERSPONSE MODIFICATION FACTOR, R  1.2  OVERSTRENGTH FACTOR, $\Omega$ 1.2  REDUNDANCY FACTOR, $\rho$ HORIZONTAL OR VERTICAL IRREGULARITIES  NON  SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  12.73 PSF, 13.41	90
FUNDAMENTAL PERIOD OF THE STRUCTURE, T 0.15.  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ 2.0  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ - USED 2.08 * 0.70  TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3)  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, $S_{D1}$ 1.0  SEISMIC DESIGN CATEGORY E RESPONSE MODIFICATION FACTOR, R 1.2  OVERSTRENGTH FACTOR, $\Omega$ 1.2  REDUNDANCY FACTOR, $\rho$ 1.0  HORIZONTAL OR VERTICAL IRREGULARITIES NON SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE) 1.16  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE) 12.73 PSF, 13.41	20
FUNDAMENTAL PERIOD OF THE STRUCTURE, T 0.15.  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ 2.0  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ - USED 2.08 * 0.70  TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3)  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, $S_{D1}$ 1.0  SEISMIC DESIGN CATEGORY E RESPONSE MODIFICATION FACTOR, R 1.2  OVERSTRENGTH FACTOR, $\Omega$ 1.2  REDUNDANCY FACTOR, $\rho$ 1.0  HORIZONTAL OR VERTICAL IRREGULARITIES NON SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE) 1.16  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE) 12.73 PSF, 13.41	70
DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ 2.0  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ - USED 2.08 * 0.70  TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3)  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, $S_{D1}$ 1.0  SEISMIC DESIGN CATEGORY  RESPONSE MODIFICATION FACTOR, R 1.2  OVERSTRENGTH FACTOR, $\Omega$ 1.2  REDUNDANCY FACTOR, $\rho$ 1.0  HORIZONTAL OR VERTICAL IRREGULARITIES NON  SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE) 1.16  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE) 12.73 PSF, 13.41	52 s
DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ - USED  TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3)  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, $S_{D1}$ SEISMIC DESIGN CATEGORY  RESPONSE MODIFICATION FACTOR, R  OVERSTRENGTH FACTOR, $\Omega$ REDUNDANCY FACTOR, $\rho$ HORIZONTAL OR VERTICAL IRREGULARITIES  SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  12.73 PSF, 13.41	
TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3)  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> SEISMIC DESIGN CATEGORY  RESPONSE MODIFICATION FACTOR, R  OVERSTRENGTH FACTOR, Ω  REDUNDANCY FACTOR, ρ  HORIZONTAL OR VERTICAL IRREGULARITIES  SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  12.73 PSF, 13.41	08
TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3)  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> SEISMIC DESIGN CATEGORY  RESPONSE MODIFICATION FACTOR, R  OVERSTRENGTH FACTOR, Ω  REDUNDANCY FACTOR, ρ  HORIZONTAL OR VERTICAL IRREGULARITIES  SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  12.73 PSF, 13.41	
DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> SEISMIC DESIGN CATEGORY  RESPONSE MODIFICATION FACTOR, R  OVERSTRENGTH FACTOR, Ω  REDUNDANCY FACTOR, ρ  HORIZONTAL OR VERTICAL IRREGULARITIES  SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  12.73 PSF, 13.41	<b>7</b> 0 = 1.456
SEISMIC DESIGN CATEGORY  RESPONSE MODIFICATION FACTOR, R  1.2  OVERSTRENGTH FACTOR, Ω  REDUNDANCY FACTOR, ρ  HORIZONTAL OR VERTICAL IRREGULARITIES  SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  12.73 PSF, 13.41	
SEISMIC DESIGN CATEGORY  RESPONSE MODIFICATION FACTOR, R  1.2  OVERSTRENGTH FACTOR, Ω  REDUNDANCY FACTOR, ρ  HORIZONTAL OR VERTICAL IRREGULARITIES  SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  12.73 PSF, 13.41	
RESPONSE MODIFICATION FACTOR, R  OVERSTRENGTH FACTOR, $\Omega$ REDUNDANCY FACTOR, $\rho$ HORIZONTAL OR VERTICAL IRREGULARITIES  SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  12.73 PSF, 13.41	
OVERSTRENGTH FACTOR, $\Omega$ 1.2  REDUNDANCY FACTOR, $\rho$ 1.0  HORIZONTAL OR VERTICAL IRREGULARITIES NON SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE) 1.10  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE) 12.73 PSF, 13.41	
REDUNDANCY FACTOR, ρ  HORIZONTAL OR VERTICAL IRREGULARITIES  SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  12.73 PSF, 13.41	
HORIZONTAL OR VERTICAL IRREGULARITIES  SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  12.73 PSF, 13.41	
SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  1.10  1.273 PSF, 13.41	
ALLOWABLE SOIL BEARING FOR FOUNDATIONS  VARIES - SEE FOUN	· ·
ALLOWABLE SOIL BEARING FOR FOUNDATIONS VARIES - SEE FOUN	
	NDATION CHARTS
FLOOD DESIGN - DESIGN IS ASSUMED TO NOT BE IN FLOOD HAZARD AREA	
IF PROJECT IS LOCATED IN A FLOOD ZONE OTHERTHAN ZONE X, A LETTER STAMPED & SIGNED FROM A SOILS ENGINEER IS REQUIRED TO VALIDATE THE	

ALL DEFLECTIONS SHOWN ALSO INCLUDE THE P-DE	LTA ROTATION PER IR PC-7	DEFLECT	IONS ARE FOR (1) STI	RUCTURE
		SOIL	CLASSES PER CBC TABLE 18	06A.2
MAXIMUM DRIFT $\delta_{max}$ SIDE COLUMNS		Soil Class 5	Soil Class 4	Soil Class 3
20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.40	2.55	2.65
30' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.25	2.35	2.45
40' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT) MINIMUM SEPARATION $(\delta_m = C_d \ \delta_{max})$ $C_d = 1.25$	(INCHES)	2.20	2.25	2.20
20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	3.00	3.19	3.31
30' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.81	2.94	3.06
40' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.75	2.81	2.75
MAXIMUM DRIFT $\delta_{max}$ CORNER COLUMNS		Soil Class 5	Soil Class 4	Soil Class 3
20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.20	2.30	2.40
30' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.30	2.45	2.50
40' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT) MINIMUM SEPARATION $(\delta_m = C_d \ \delta_{max})$ $C_d = 1.25$	(INCHES)	2.40	2.55	2.65
20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.75	2.88	3.00
30' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.88	3.06	3.13
40' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	3.00	3.19	3.31
MAXIMUM DRIFT $\delta_{max}$ END COLUMNS		Soil Class 5	Soil Class 4	Soil Class 3
20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	1.60	1.70	1.75
30' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.00	2.45	2.25
40' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT) MINIMUM SEPARATION $(\delta_m = C_d \ \delta_{max})$ $C_d = 1.25$	(INCHES)	2.50	2.30	2.80
20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.00	2.13	2.19
30' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.50	3.06	2.81
40' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	3.13	2.88	3.50

STRUCTURAL SEPARATION

ARCHITEC TURAL REQUIREMENTS	
DESC RIPTION	DESIGN VAULES
TYPE OF CONSTRUCTION	II-B
OCCUPANCY CLASSIFICATION	A-3
NUMBER OF STORIES	1
FIRE SPRINKLER SYSTEM	NOT BY ICON/WEIGHT NOT INCLUDED IN DESIGN

### RELATED BUILDING CODES AND STANDARDS

### TITLE 24 CODES:

2019 CALIFORNIA ADMINISTRATIVE CODE (CAC).. ..(PART 1, TITLE 24, CCR) 2019 CALIFORNIA BUILDING CODE (CBC), VOLUMES 1, AND 2.(PART 2, TITLE 24,

2019 CALIFORNIA ELECTRICAL CODE. .(PART 3, TITLE 24, CCR) 2019 CALIFORNIA MECHANICAL CODE (CMC). .(PART 4, TITLE 24, CCR) (PART 5, TITLE 24, CCR) 2019 CALIFORNIA PLUMBING CODE (CPC)... 2019 CALIFORNIA ENERGY CODE. (PART 6, TITLE 24, CCR) 2019 CALIFORNIA FIRE CODE (CFC) .(PART 9, TITLE 24, CCR` 2019 CALIFORNIA GREEN BUILDING STANDARDS CODE.....(PART 11, TITLE 24, CCR) 2019 CALIFORNIA REFERENCE STANDARDS CODE.. ..(PART 12, TITLE 24, CCR)

REFERENCE CODE SECTIONS FOR APPLICABLE STANDARDS: 2019 CBC, CHAPTER 35 2019 CFC, CHAPTER 80

### SCOPE OF WORK NARRATIVE

THESE DRAWINGS ILLUSTRATE THE FABRICATION AND INSTALLATION REQUIREMENTS FOR A FREE-STANDING PREFABRIC ATED STEEL SHADE STRUCTURE. THE ENTIRE STRUCTURAL SYSTEM IS COMPRISED OF HOLLOW STRUCTURAL STEEL MEMBERS SUPPORTED BY CONCRETE FOUNDATIONS. THE FLEXIBILITY INCLUDED HEREIN

ALLOWS THE STRUCTURE TO COMPLY WITH A WIDE VARIETY OF PROJECT SITES AND LOADING REQUIREMENTS.

### <u>GENERAL:</u>

- 1. GENERAL NOTES AND TYPICAL DETAILS SHALL APPLY TO ALL PARTS OF THE JOB EXCEPT WHERE THEY MAY CONFLICT WITH DETAILS AND NOTES ON OTHER SHEETS. WHERE CONDITIONS ARE NOT SPECIFICALLY INDICATED BUT ARE OF SIMILAR CHARACTER TO DETAILS SHOWN, SIMILAR DETAILS OF CONSTRUCTION SHALL BE USED SUBJECT TO REVIEW BY THE STRUCTURAL ENGINEER FOR THIS PROJECT
- 2. WORK SHALL CONFORM TO THE REQUIREMENTS, AS AMENDED TO DATE, OF THE LATEST ADOPTED EDITION OF THE CBC, C.A.C. TITLE 24, AND ALL OTHER LOCAL, STATE AND FEDERAL REGULATIONS.
- 3. OMISSIONS OR CONFLICTS BETWEEN THE VARIOUS ELEMENTS OF THE WORKING DRAWINGS AND/OR SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER FOR THIS PROJECT PRIOR TO PROCEEDING
- 4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING THE WORK OF ALL TRADES AND SHALL CHECK ALL DIMENSIONS, ALL DISCREPANCIES SHALL BE CALLED TO THE ATTENTION OF THE STRUCTURAL ENGINEER FOR THIS PROJECT AND BE RESOLVED BEFORE PROCEEDING WITH THE WORK.
- 5. THESE CONSTRUCTION DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE AND DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND SHALL BE SOLELY RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES, INCLUDING, BUT NOT LIMITED TO, BRACING, TEMPORARY SUPPORTS, AND SHORING. OBSERVATION VISIT TO THE SITE BY FIELD REPRESENTATIVES OF THE ARCHITECT/ENGINEER SHALL NOT INCLUDE INSPECTIONS OF THE PROTECTIVE MEASURES OR THE CONSTRUCTION PROCEDURES. ANY SUPPORT SERVICES PERFORMED BY THE ARCHITECT/ENGINEER DURING THE CONSTRUCTION SHALL BE DISTINGUISHED FROM CONSTRUCTION AND DETAILED INSPECTION SERVICES WHICH ARE FURNISHED BY OTHERS. THESE SUPPORT SERVICES PERFORMED BY THE ARCHITECT/ENGINEER, WHETHER OF MATERIAL OR WORK, ARE FOR THE PURPOSE OF ASSISTING IN QUALITY
- CONTROL AND IN ACHIEVING CONFORMANCE WITH CONTRACT DOCUMENTS, BUT DO NOT GUARANTEE CONSTRUCTION. 6. ASTM DESIGNATIONS AND ALL STANDARDS REFER TO THE LATEST AMENDMENTS.
- 7. CONFORM TO APPLICABLE CAL/OSHA CONSTRUCTION SAFETY REGULATIONS FOR ALL WORK PERFORMED DURING CONSTRUCTION. JOB SITE SAFETY IS STRICTLY THE RESPONSIBILITY OF THE CONTRACTOR AND NOT THE ARCHITECT/ENGINEER OR OWNER.
- 8. THE ENGINEER AND THEIR CONSULTANTS SHALL HAVE NO RESPONSIBILITY FOR THE DISCOVERY, HANDLING, REMOVAL OR DISPOSAL OF HAZARDOUS MATERIALS AT THE PROJECT SITE, INCLUDING BUT NOT LIMITED TO
- ASBESTOS, ASBESTOS PRODUCTS, POLYCHLORINATED BIPHENYL (PCB) OR OTHER TOXIC SUBSTANCES. 9. SHOULD ANY CONDITIONS DEVELOP NOT COVERED BY THE CONTRACT DOCUMENTS, OR IF A CHANGE IN THE SCOPE OF WORK IS PROPOSED, A CONSTRUCTION CHANGE DOCUMENT DETAILING AND SPECIFYING THE REQUIRED
- CHANGE(S) SHALL BE SUBMITTED TO AND APPROVED BY DSA BEFORE PROCEEDING WITH THE WORK. 10. THE SCHOOL DISTRICT INSPECTOR ON RECORD SHALL INSPECT AND APPROVE THE ERECTED FRAME PRIOR TO ROOF
- 11. SEE REQUIREMENTS FOR LOCATION IN ANY FIRE HAZARD SEVERITY ZONE FOR WILDLAND URBAN INTERFACE AREAS (WUI) AS SPECIFIED IN THE APPLICABLE VERSION OF THE CALIFORNIA BUILDING CODE. PROVIDE PROTECTION AND DETAILS OF ALL AREAS COMPLYING WITH THE WUI REQUIREMENTS.
- 12. LOCATING THIS STRUCTURE CLOSER THAN 20 FEET TO OTHER STRUCTURES MAY AFFECT THE ALLOWABLE AREA
- FOR THE EXISTING CONSTRUCTION PER THE APPLICABLE VERSION OF THE CALIFORNIA BUILDING CODE. 13. VIEWS AND DETAILS ARE NOT DRAWN TO SCALE (UNLESS NOTED OTHERWISE). DO NOT SCALE THESE DRAWINGS.

### STRUCTURAL AND MISCELLANEOUS STEEL:

- 1. ALL STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE AMERICAN INSTITUE OF STEEL CONSTRUCTION (AISC) SPECIFICATION MANUAL REFERENCED BY THE LATEST EDITION OF THE CALIFORNIA BUILDING CODE.
- 2. PIPE SECTIONS SHALL CONFORM TO ASTM A53, Fy = 35 KSI, GRADE B OR A501 UNLESS NOTED OTHERWISE. 3. STRUCTURAL TUBING (HSS SHAPES) SHALL CONFORM TO ASTM A-500, GRADE B (OR C), Fy = 46 KSI (MIN).
- 4. IF MATERIAL AVAILABILITY IS LIMITED, MEMBER THICKNESS CAN BE INCREASED BEYOND WHAT IS SHOWN IN THESE DRAWINGS (MAXIMUM INCREASE OF 1/8").
- 5. ALL CHANNELS, ANGLES, AND MISC. STEEL SHALL CONFORM TO ASTM A-36, Fy =36 KSI.
- 6. ALL PLATE STEEL SHALL CONFORM TO ASTM A-572, Fy= 50 KSI.
- 7. ALL COLD FORM STEEL SHALL CONFORM TO ASTM A-653, CS = TYPE B, Fy = 50 KSI. 8. STRUCTURAL STEEL AND DECK SHALL BE IDENTIFIED FOR CONFORMITY PER CBC 2202A.1.
- 9. ALL ROOF DECKS SHALL HAVE KYNAR 500 METAL COATING.
- 10.ALL ROOF DECKS SHALL CONFORM TO ASTM A-792, Fy = 50 KSI.

### INSTRUCTIONS FOR ARCHITECTS SUBMITTING THESE PRE-CHECKED DRAWING TO DSA: BEFORE SUBMITTING THESE PRE-CHECKED DRAWINGS FOR YOUR PROJECT, FOLLOW THE

STEP 1: SELECT FRAME DIMENSIONS FOR YOUR PROJECT -GABLE STRUCTURES UP TO 20' WIDE USE THE "RG 20" BASE FRAME -GABLE STRUCTURES UP TO 30' WIDE USE THE "RG 30" BASE FRAME

STEPS BELOW TO PROPERLY DEFINE THE APPROVED OPTIONS:

-GABLE STRUCTURES UP TO 40' WIDE USE THE "RG 40" BASE FRAME -MAXIMUM WIDTH IS 40' (SEE "ARCHITECTURAL VIEWS" SHEET FOR REFERENCE) -THE 24', 44', 64', 84' AND 104' LENGTHS ARE SUGGESTED BECAUSE THEY ARE THE MOST COMMON

(20' BAYS ARE THE MOST ECONOMICAL) -FRAME LENGTHS ASSUME 2' OVERHANGS (UNO BY ARCHITECT - 2' MAX DIMENSION)

STEP 2: SELECT ROOF DECK FOR YOUR PROJECT -"M" REPRESENTS McELROY METAL "MULTI-RIB" ROOF PANEL

-"G" REPRESENTS McELROY METAL "MEGA-RIB" ROOF PANEL -"S" REPRESENTS MCELROY METAL "MEDALLION-LOK" 16" STANDING SEAM ROOF PANEL

STEP 3: IDENTIFY THE Ss ACCELERATION (q) FOR YOUR PROJECT

-Ss VALUE DETERMINES THE REQUIRED SEISMIC DESIGN FORCES -Ss VALUE DEPENDS ON THE PROJECTS GEOGRAPHICAL LOCATION (VALUES RANGE FROM 0.00 TO 3.73)

STEP 4: IDENTIFY THE Ss REGION FOR YOUR PROJECT

-THE REGIONS ARE DEPENDANT ON THE Ss VALUE DETERMINED IN STEP 3 -THE SS REGION DICTATES THE MAXIMUM DEAD LOAD PERMITTED ON THE FRAME (SEE TABLE TO RIGHT)

STEP 5: IDENTIFY THE ROOF DEAD LOAD FOR YOUR PROJECT -THE ROOF DECK DEAD LOAD WILL ALWAYS BE INCLUDED -THE COLLATERAL LOAD REPRESENTS ADDITIONAL LOAD THAT CAN BE SUPPORTED BY THE FRAME -BE SURE THE TOTAL ROOF DEAD LOAD FOR YOUR PROJECT IS LESS THAN OR EQUAL TO THE MAX

-MARK UP PC DRAWINGS WITH SIZE AND LOCATION OF CUTOUTS BEFORE SUBMITTING TO DSA

DEAD LOAD SHOWN IN STEP 4 FOR YOUR SE VALUE -Sds value used in calculation is the capped Sds (see design criteria) STEP 6: IDENTIFY THE FOUNDATION REQUIREMENTS FOR YOUR PROJECT

-IDENTIFY SOIL CLASS FOR PROJECT SITE PER SITE SPECIFIC SOIL CONDITIONS -USE THIS TO SELECT CORRECT FOUNDATION SIZE ON FOUNDATION SHEET STEP 7: SELECT MISCELLANEOUS OPTIONS FOR YOUR PROJECT -MAXIMUM CLEAR HEIGHT IS 12'-0"; (SEE "ARCHITECTURAL VIEWS" SHEET FOR REFERENCE)

STEP 8: SELECT APPLICABLE SHEET INDEX FOR YOUR PROJECT -REFERENCE THE BASE FRAME (STEP 1) AND THE ROOF PANEL TYPE (STEP 2) -IDENTIFY THE APPLICABLE SHEÈT INDEX

STEP 9: INCLUDE APPLICABLE SHEETS WITH YOUR DSA SUBMITTAL -INCLUDE 'MISC DESIGN OPTIONS' SHEET FOR PROJECTS WITHOUT ELECTRICAL CUTOUTS OR GUTTERS

### NOTICE OF DISCLAIMER FOR STRUCTURAL ENGINEERING RESPONSIBILITY

- 1. PER TITLE 24, PART 1, SECTION 4-316(e) OF THE CALIFORNIA CODE OF REGULATIONS, THIS NOTICE SHALL
- BE GIVEN TO DSA PRIOR TO THE APPROVAL OF PLANS AND SPECIFICATIONS. 2. FOR THE SITE SPECIFIC PROJECT, J. R. MILLER & ASSOCIATES IS NOT THE DESIGN PROFESSIONAL IN
- GENERAL RESPONSIBLE CHARGE. 3. FOR THE SITE SPECIFIC PROJECT, J.R. MILLER & ASSOCIATES' RESPONSIBILITY IS LIMITED TO THE PREPARATION OF THE PLANS AND SPECIFICATIONS FOR THE SHELTERS OF THIS PC ONLY.
- 4. STRUCTURAL OBSERVATION OF CONSTRUCTION IS SPECIFICALLY EXCLUDED FROM J.R. MILLER & ASSOCIATES' RESPONSIBILITY FOR THE SITE SPECIFIC PROJECT. 5. ALL CONSTRUCTION ACTIVITIES RELATED TO STRUCTURAL ENGINEERING SHALL BE DELEGATED TO A QUALIFIED ENGINEER BY THE DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE CHARGE. THESE ACTIVITIES INCLUDE.

BUT ARE NOT LIMITED TO, APPROVAL OF INSPECTOR QUALIFICATIONS, STRUCTURAL OBSERVATION OF

CONSTRUCTION, REVIEW OF INSPECTION REPORTS, AND SIGNING OFF OF THE VERIFIED REPORT FOR

COMPLETED WORK. 6. J.R. MILLER & ASSOCIATES WILL BE RESPONSIBLE FOR RESPONDING TO QUESTIONS PERTAINING TO THE PLANS AND SPECIFICATIONS FOR THE SHELTERS OF THIS PC WHICH ARISE DURING PLAN REVIEW AND CONSTRUCTION.

- 1. ALL WELDING SHALL COMPLY WITH AWS D1.1 SPECIFICATIONS AND SHALL BE DONE BY AWS QUALIFIED WELDERS CERTIFIED FOR THE TYPE OF WELDING TO BE PERFORMED AS REQUIRED BY DSA.
- 2. ALL WELDING SHALL BE DONE BY GAS METAL ARC PROCESS WITH E70XX ELECTRODES. FLUX CORE ARC WELD SHALL CONFORM TO CHARPY NOTCH TOUGHNESS RATING OF 20 ft-16 @ ( 0° F).
- 3. ALL WELDING SHALL BE DONE IN THE SHOP WITH REQUIRED INSPECTION, PRE-APPROVED BY DSA, TO ENSURE PROPER MATERIAL ID AND WELDING.
- 4. WELD FILLER METAL MANUFACTURER SHALL PROVIDE WRITTEN CERTIFICATION OF COMPLIANCE WITH CODE AND SPECIFIC ATIONS.

- 1. ALL BOLTS SHOWN ON THESE DRAWINGS ARE ASTM F3125 GRADE A325 HIGH STRENGTH BOLTS (UNO), WITH THE NUTS CONFORMING TO ASTM A-563.
- 2. HIGH STRENGTH BOLTS SHALL BE VERIFIED AND INSPECTED PER CBC 1705A2.1
- 3. BEFORE ERECTING THE FRAME, VERIFY ALL BOLTS AND NUTS ARE CLEAN OF DEBRIS AND BURRS INCLUDING THE HARDWARE ALREADY FASTENED INSIDE THE MEMBERS. CHASING SOME OF THE BOLTS AND NUTS MAY BE
- 4. HARDENED STEEL WASHERS SHALL CONFORM TO ASTM F-436.
- 5. THE BOLTING INSTALLATION REQUIREMENTS OUTLINED BELOW ARE CRITICAL TO THE STRUCTURE'S DESIGN AND PERFORMANCE. THE INSTALLER IS REQUIRED TO COORDINATE THIS PHASE OF CONSTRUCTION WITH THE SPECIAL BOLTING INSPECTOR AND THE INSPECTOR OF RECORD PRIOR TO THE ERECTION OF THE FRAME. ALL BOLTS SHALL BE INSTALLED AND INSPECTED PER THE APPLICABLE VERSION OF AISC'S "SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS", CBC 1705A.2.1; AISC 341-16 J7; AISC 360-16 N5.6.
  - A)PRETENSIONED JOINTS MUST BE INSTALLED AND INSPECTED TO MEET ONE OF THE FOLLOWING REQUIREMENTS:
  - 1. TURN-OF-NUT PRETENSIONING
  - 2. CALIBRATED WRENCH PRETENSIONING 3. DIRECT-TENSION-INDICATOR PRETENSIONING (CONTRACTOR RESPONSIBLE FOR PURCHASE OF

- 1. ALLOWABLE SOIL PRESSURES ASSUME CLASS 5 SOIL CLASSIFICATION PER CBC TABLE 1806A, UNLESS NOTED
- 2. PER CBC SECTION 1803A.2, GEOTECHNICAL REPORTS ARE NOT REQUIRED FOR ONE-STORY LIGHT-STEEL FRAME BUILDINGS OF TYPE II CONSTRUCTION AND 4,000 SQUARE FOOT OR LESS IN FLOOR AREA AND NOT LOCATED WITHIN EARTHQUAKE FAULT ZONESOR SIESMIC HAZARD ZONES AS SHOWN ON THE MOST RECENT MAPS PUBLISHED BY THE CGS. ALLOWABLE FOUNDATION AND LATERAL SOIL PRESSURE VALUES MAY BE DETERMINED FROM TABLE 1806A.2.
- 3. FILL AND BACKFILL SHALL BE COMPACTED TO 95% OF MAX. DENSITY IN ACCORDANCE WITH ASTM TEST METHOD D-1557 OR AS RECOMMENDED BY THE GEO-TECH ENGINEER. FLOODING NOT PERMITTED.
- 4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SHORING, ETC. NECESSARY TO SUPPORT CUT AND/OR FILL BANKS DURING EXCAVATION, AND FORMING AND PLACEMENT OF CONCRETE.
- 5. MINIMUM SETBACK FROM TOE OF SLOPE ON AN ASCENDING SLOPE SHALL BE 15 FEET AND MINIMUM SETBACK
- FROM TOE OF SLOPE ON A DESCENDING SLOPE SHALL BE 40 FEET
- 6. PER CBC SECTION 1803A.6, GEOHAZARD REPORTS ARE NOT REQUIRED FOR ONE-STORY LIGHT-STEEL FRAME BUILDINGS OF TYPE II CONSTRUCTION AND 4,000 SQUARE FOOT OR LESS IN FLOOR AREA AND NOT LOCATED WITHIN EARTHQUAKE FAULT ZONESOR SIESMIC HAZARD ZONES AS SHOWN ON THE MOST RECENT MAPS PUBLISHED BY THE CGS.
- 7. GEOHAZRD REPORTS ARE TO COMPLY WITH DSA IR A-4 PER IR-7 SECTION 1.8
- 8. SITE SPECIFIC GEOTECHNICAL REPORT IS REQUIRED AT THE TIME OF SITE APPLICATION IS USING OTHER THAN
- 9. LATERAL BEARING HAS BEEN INCREASED PER CBC 1806A.3.4 & HAS BEEN DESIGNED FOR P-DELTA EFFECTS

### 1. MIX DESIGN REQUIREMENTS: (NORMAL WEIGHT CONCRETE)

STRENGTH Pc (28 DAYS)	W/C RATIO (NON—AIR ENTRAINED)	W/C RATIO (AIR ENTRAINED)	SLUMP (±1")	UNIT WEIGHT (NORMAL WEIGHT)				
4500 PSI	0.44	0.35	3"	150 PCF				
2 CONCRETE MIX DESIGN DARAMETERS ARE COOR FOR EXPOSURE CATECORIES FO. E1. 9: E2. THE AIR								

ENTRAINMENT FOR THESE CATEGORIES SHALL BE AS FOLLOWS: F0-0, F1-4.5, F2-6 3. AGGREGATES SHALL CONFORM TO THE ASTM C-33 WITH PROVEN SHRINKAGE CHARACTERISTICS OF LESS THAN 0.005.

SCHOOL DISTRICT:

- MAX AGGREGATE SIZE = 1". 4. CEMENT SHALL CONFORM TO ASTM C-150 (TYPE V) UNLESS NOTED OTHERWISE ON THE DRAWINGS.
- 5. CONCRETE SHALL BE MAINTAINED IN A MOIST CONDITION FOR A MINIMUM OF FIVE DAYS AFTER PLACEMENT.
- ALTERNATE METHODS WILL BE APPROVED IF SATISFACTORY PERFORMANCE CAN BE ASSURED. 6. CONCRETE SHALL NOT FREE FALL MORE THAN FIVE FEET.
- 7. CONCRETE DURABILITY SHALL BE PER CBC 1904A.1 & ACI 318-14 CHAPTER 19.
- 8. CONCRETE SHALL BE TESTED PER CBC 1903A, TABLE 1705A.3. AND ACI 318-14 SECTION 26.12.

### STEP 10: IDENTIFY PROJECT NAME AND SCHOOL DISTRICT

CONSTRUCTION NOTES

TESTS AND INSPECTIONS FOR THE PROJECT.

SHALL COMPLY WITH ALL LOCAL ORDINANCES

PROJECT NAME:

	FRAME DIMENSIONS								
1		SUGGESTED				OTHER			
STEF	FRAME WIDTH	[] 20'	<b>&gt;</b> 30°	[] 40'		[ ] (40' MAX)			
	FRAME LENGTH	[] 44'	<b>×</b> 64	[]84'	[] 104'	[] (NO MAX)			

7		ROOF PANEL	
STEP	ROOF PANEL TYPE	<b>⋈</b> м [] G [] S	
3 E		PROJECT SITE — Ss ACCELERATION (g)	
ST		0.642	

	Ss REGION		
		Ss REGIONS	MAX DEAD LOAD
	Х	0 < Ss <= 2.14	5 PSF
		2.14 < Ss <= 2.50	5 PSF
DESCRIPTION		2.50 < Ss <= 2.75	5 PSF
		2.75 < Ss <= 3.00	4 PSF
		Ss > 3.73 MAX	3 PSF
	DESCRIPTION	X	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$

		TOTAL ROOF DEAD LOA	AD
		DEAD LOAD	EXAMPLES
ГР 5	ROOF DECK	<u>1.1</u> PSF	M=1.1PSF; G=1.2PSF;S=1.3PSF (SEE STEP 2)
STE	COLLATERAL	<u>0</u> PSF	LIGHTING, ETC
	TOTAL	<u>1.1</u> PSF	ADD ROOF DECK AND COLLATERAL LOADS (MAX 5 PSF)

1. A DSA-CERTIFIED CLASS 3 PROJECT INSPECTOR IS REQUIRED FOR THIS PROJECT.

BEFORE PROCEEDING WITH THE WORK, (SECTION 4-317(c), PART 1, TITLE 24, CCR)

2. CHANGES TO THE APPROVED DRAWINGS AND SPECIFICATIONS SHALL BE MADE BY ADDENDA OR CONSTRUCTION CHANGE

CONTINUOUS INSPECTION OF WORK, THE DUTIES OF THE INSPECTOR ARE DEFINED IN SECTION 4-342, PART 1, TITLE 24, CCR.

RECONSTRUCTION IS TO BE IN ACCORDANCE WITH TITLE 24, CCR. SHOULD ANY EXISTING CONDITIONS SUCH AS DETERIORATION

FINISHED WORK WILL NOT COMPLY WITH TITLE 24, CCR, A CONSTRUCTION CHANGE DOCUMENT (CCD), OR A SEPARATE SET OF

PLANS AND SPECIFICATIONS, DETAILING AND SPECIFYING THE REQUIRED WORK SHALL BE SUBMITTED TO AND APPROVED BY DSA

OR NON-COMPLYING CONSTRUCTION BE DISCOVERED WHICH IS NOT COVERED BY THE CONTRACT DOCUMENTS WHEREIN THE

6. GRADING PLANS, DRAINAGE IMPROVEMENTS, ROAD AND ACCESS REQUIREMENTS AND ENVIRONMENTAL HEALTH CONSIDERATIONS

4. A DSA ACCEPTED TESTING LABORATORY DIRECTLY EMPLOYED BY THE DISTRICT (OWNER) SHALL CONDUCT ALL THE REQUIRED

5. THE INTENT OF THESE DRAWINGS AND SPECIFICATIONS ARE THAT ALL THE WORK OF THE ALTERATION, REHABILITATION OR

 $\,$  3. A "DSA CERTIFIED" PROJECT INSPECTOR EMPLOYED BY THE DISTRICT (OWNER) AND APPROVED BY DSA SHALL PROVIDE

DOCUMENT (CCD) APPROVED BY DSA, AS REQUIRED BY SECTION 4-338, PART 1, TITLE 24, CCR.

- 1. REINFORCING STEEL SHALL BE DEFORMED STEEL CONFORMING TO THE REQUIREMENTS OF ASTM A-615,
  - GR 60: (#4 BARS AND LARGER)
  - GR 40: (#3 BARS)
- DETAILING, FABRICATION, AND ERECTION OF REINFORCING BARS SHALL CONFORM TO THE ACL "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCING CONCRETE STRUCTURES."
- 3. MIN. COVER FOR CAST-IN-PLACE CONCRETE SHALL BE AS FOLLOWS:
- A. CAST AGAINST EARTH ..... B. CAST AGAINST FORM BELOW GRADE .....
- C. FORMED SLABS (#11 BAR & SMALLER)......3/4"
- D. SLABS ON GRADE (FROM TOP OF SLAB)......1"
- 4. BARS SHALL BE CLEAN OF RUST, GREASE OR OTHER MATERIAL LIKELY TO IMPAIR BOND. BENDS SHALL BE MADE COLD.
- REINFORCING SHALL BE LAP SPLICED PER ACI 318-14 SECTION 25.5.
- 6. PRIOR TO PLACING OF CONCRETE, REINFORCING STEEL AND EMBEDDED ITEMS SHALL BE WELL SECURED IN POSITION. 7. WELDING OF REINFORCING IS NOT ALLOWED.

### 8. REINFORCING STEEL SHALL BE INSPECTED PER CBC 1705A.3.

- POWDER-COAT FINISH SYSTEM: ALL BUILDINGS THAT HAVE A POWDER-COATED FINISH SHALL MEET THE FOLLOWING SPECIFICATIONS:
- 1. THE STEEL FRAME SHALL BE SHOT-BLASTED TO A NEAR WHITE CONDITION PER SSPC-10 SPECIFICATIONS. 2. THE STEEL SHALL BE WASHED IN A ZINC PHOSPHATE IN AN MINIMUM EIGHT STAGE ELECTRO DEPOSITION
- 3. IMMEDIATELY FOLLOWING PRE-TREATMENT THE STEEL SHALL BE TOTALLY IMMERSED IN A LIQUID EPOXY
- PRIMER(E-COAT) AND COATED TO A UNIFORM THICKNESS OF A MINIMUM OF 0.7 TO 0.9 MILS. THE E-COATING SHALL
- PROVIDE A MINIMUM OF 1000 HOURS OF SALT SPRAY CORROSION PROTECTION TO THE STEEL. 4. THE STEEL SHALL THEN HAVE A TGIC POLYESTER COLOR COAT APPLIED OVER THE E-COATED SURFACE.
- 5. THE COLOR COAT SHALL THEN HAVE A CLEAR TGIC COATING APPLIED TO SEAL IN THE COLOR COAT AND RESIST
- ULTRAVIOLET LIGHT, TO HELP PREVENT FADING. 6. THE FINISH THICKNESS OF THESE THREE APPLICATIONS SHALL BE A MINIMUM OF 8 TO 12 MILS

MINIMUM MISC ELLANEOUS

7. ALL CARBON STEEL MEMBERS (COLUMNS, BEAMS, PLATES, ETC.) NOT POWDER-COATED SHALL BE PAINTED WITH PRIME COAT PER THE "AISC CODE OF STANDARD PRACTICE" AND THE "AISC SPECIFICATION SECTION M3" (UNLESS NOTED

OTHERWISE).	
ABBREVIATIONS:	

REINFORCING STEEL:

<u>ABBREVI</u>	ATIONS:		
ACI	AMERICAN CONCRETE INSTITUTE	MPH	MILES PER HOUR
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION	М	MULTI-RIB ROOF PANEL (MCELROY)
ASM	ASSEMBLY (INTERNAL REFERENCE)	NTS	NOT TO SCALE
ASTM	AMERICAN SOCIETY FOR TESTING AND MAT'LS	NO	NUMBER
AWS	AMERICAN WELDING SOCIETY	ОС	ON CENTER
CBC	CALIFORNIA BUILDING CODE	OSHA	OCCUPATIONAL HEALTH AND SAFETY ADMIN
CJP	COMPLETE JOINT PENETRATION	PCF	POUNDS PER CUBIC FOOT
CLR	CLEAR	PJ	PRETENSIONED JOINT
DEG	DEGREE	PLCS	PLACES
DIA	DIAMETER	PLT	PLATE
DIM	DIMENSION	PSF	POUNDS PER SQUARE FOOT
DSA	DIVISION OF THE STATE ARCHITECT	PSI	POUNDS PER SQUARE INCH
EQ	EQUAL	QTY	QUANTITY
FT	FEET	REF	REFERENCE
GA	GAGE	SQ	SQUARE
IN	INCHES	SS	STANDING SEAM ROOF PANEL (MCELROY)
KSI	KIPS PER SQUARE INCH	TYP	TYPIC AL
MAX	MAXIMUM	UNO	UNLESS NOTED OTHERWISE

U.S. GEOLOGIC AL SURVEY

DIV. OF THE STATE ARCHITECT APP: 04-120013 PC REVIEWED FOR SS 🗹 FLS 🗹 ACS 🗹 CG 🗆 DATE: 08/06/2021

DRAWN BY

DATE

REV

REV DATE

ARCHITECTS ENGINEERS

2700 SATURN STIBREA, CA 92821

. 714.524.1870 | F. 714.524.1875

**ANGEL** 

4/2/202

FOUNDATION REQUIREMENTS SOIL CLASS 5 (BEARING)-1500 PSF 📈 | SOIL CLASS 4 (BEARING)-2000 PSF [] SOIL CLASS 3 (BEARING)-3000 PSF [ SOIL CLASS 5 (LATERAL BEARING)—100 PSF | SOIL CLASS 4 (LATERAL BEARING)—150 PSF SOIL CLASS 3 (LATERAL BEARING)—200 PSF

SHEET INDEX

	MIS	SC ELLANEOUS	
		DESIGN	OPTIONS
<u>Е</u>	CLEAR HEIGHT	[]8' 🔀 10' []12'	[] ' (12' MAX)
S	ELECTRICAL CUTOUTS	<b>⋈</b> YES	[ ] NO
	GUTTERS	<b>⋈</b> YES	[ ] NO

	BASE FRAME		RG 20			RG 30			RG 40	
	ROOF PANEL TYPE	М	G	S	М	G	S	М	G	S
	SELECT ONE	[]	[]	[]	[]	[]	[X]	[]	[]	[]
	GENERAL NOTES	LS1.0								
	DSA 103 EXAMPLE	LS1.1								
ω	FOUNDATION PLAN	LS2.0	LS2.0	LS2.0	LS3.0	LS3.0	LS3.0	LS4.0	LS4.0	LS4.0
STEP	FRAMING PLAN	LS2.1	LS2.1	LS2.1	LS3.1	LS3.1	LS3.1	LS4.1	LS4.1	LS4.1
	FRAME CONNECTION DETAILS	LS2.1	LS2.1	LS2.1	LS3.1	LS3.1	LS3.1	LS4.2	LS4.2	LS4.2
	ROOFING LAYOUT & DETAILS	LS2.2	LS2.3	LS2.4	LS3.2	LS3.3	LS3.4	LS4.3	LS4.4	LS4.5
	MISC DESIGN OPTIONS	LS5.0								

ISTINCTIVE STEEL SHELTERS

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PRE-CHECK (PC) DOCUMENT Code: 2019 CBC

A separate project application for construction is required.

616.396.0919 800.748.0985 616.396.0944 FX

PRINTED ON:

<b>SA 103-19: LISTING OF</b>	STRUCTURAL TESTS & SPECIAL IN	SPECTIONS, 2019 CBC			S15 & SPECIAL IN	ISPECTIO	NS (SOILS), 2019 CBC	DSA 103-19: LIST Application Number:	School Name:	J & JI LOIAL INJ	School District:	DSA 1 Applicat
pplication Number: 4-000000	School Name: ICON Shelter Systems	School District: PC Submittal	Application Number: 04-000000 DSA File Number:	School Name: ICON Shelter Systems Increment Number:			School District: PC Submittal Date Created:	04-000000 DSA File Number:	ICON Shelter Systems Increment Number:		PC Submittal  Date Created:	Applica 04-0000 DSA File
A File Number:	Increment Number:	Date Created: 2021-07-14 05:50:33					2021-07-14 05:50:33				2021-07-14 05:50:33	
												5 T
	2010	ono.	Geotechnical Reports:	: Project has a geotechnic	cal report, or CDs in	ndicate soil	s special inspection is required by GE	C. Compaction tes	ting.	Test L(	* Under the supervision of the geotechnical engineer. (Refer to specific items identified in the Appendix for exemptions when	re 🗆 a
IMPORTANT: This fo	<b>2019</b> orm is only a summary list of structural test:	CBC and some of the special inspections required for the project.	1. GENERAL:	· ·	Table 1705A.6						soils testing may be conducted under the supervision of a geotechnical engineer or LOR's engineering manager. In such cases, the LOR's form 291 shall satisfy the soil test reporting requirements for the exempt ite	DSA L
Generally, the structural te	ests and special inspections noted on this f	orm are those that will be performed by the Geotechnical Engineer uplete test and inspection program must be performed as detailed	Test or Special Inspe	ection	Type Pe By	erformed C y	ode References and Notes				251 shall satisfy the soil test reporting requirements for the exempt te	□ <b>b</b>
on the DSA approved doci	cuments. The appendix at the bottom of thi	of form identifies work NOT subject to DSA requirements for special for providing inspection of all facets of construction, including but		ared properly prior to placement of	Periodic of		By geotechnical engineer or his or her qualified representative. See Appendix for exemptions.)	4. CAST-IN-PLACE	E DEEP FOUNDATIONS (PIERS):	Table 1705A.8		□ C.
not limited to, special inspe	ections not listed on this form such as struc	tural wood framing, high-load wood diaphragms, cold-formed steel etc., per Title 24, Part 2, Chapter 17A (2019 CBC).	Foundation excavation	excavations for foundations. tions are extended to proper deptl	th			Test or Special In	spection	Type Perfo	rmed   Code References and Notes	d
_		is document are from the CBC, or California Building Code.	and have reached pro  • Materials below foot design bearing capaci	tings are adequate to achieve the				✓ a. Inspect drilling of and accurate record	operations and maintain complete	Continuous G	E*  * By geotechnical engineer or his or her qualified representative.  (See Appendix for exemptions.)	e
TO COLUMNS	ica section and table references round in th	is document are from the CDC, of Camornia Building Code.		·				□ <b>b.</b> Verify pier locati	ions, diameters, plumbness, bell cable), lengths and embedment into	Continuous G	E* * By geotechnical engineer or his or her qualified representative.  (See Appendix for exemptions.)	6
1. TYPE		2. PERFORMED BY	2, SOIL COMPACTION Test or Special Inspe		Table 1705A.6	erformed C	ode References and Notes		ible); record concrete or grout		(See Appendix for exemptions.)	
ntinuous – Indicates that a cont	ntinuous special inspection is	GE – Indicates that the special inspection shall be performed by a registered geotechnical engineer or his or her authorized representative.	□ a. Perform classification	ion and testing of fill materials.	By	LOR* *	Under the supervision of the geotechnical engineer.	C. Confirm adequa	te end strata bearing capacity.	Continuous G	E* * By geotechnical engineer or his or her qualified representative. (See Appendix for exemptions.)	
uired		COR – Indicates that the test or special inspection shall be performed by a testing	b. Verify use of proper	er materials, densities and	Continuous	GE* *	By geotechnical engineer or his or her qualified representative. (Refer to be decific items identified in the Appendix for exemptions where soils SI and	d. Concrete piers.		Provide tests and insp	pections per CONCRETE section below.	b
iodic – Indicates that a periodic	ic special inspection is required	laboratory accepted in the DSA Laboratory Evaluation and Acceptance (LEA) Program. See CAC Section 4-335.	during placement of f	es, placement and compaction fill.		te	sting may be conducted under the supervision of a geotechnical ngineer or LOR's engineering manager. In such cases, the LOR's form DSA					□ c.
		PI – Indicates that the special inspection may be performed by a project inspector when specifically approved by DSA.				29 ite	21 shall satisfy the soil SI and test reporting requirements for the exempt ems.)					
t – Indicates that a test is require	ired	SI – Indicates that the special inspection shall be performed by an appropriately						DGS DSA 103-19 (Revised 07.	/16/2020)			DGS DSA
		qualified/approved special inspector.						DIVISION OF THE STATE ARC	HITECT	DEPARTMENT OF GEN Page 3 o		DRNIA DIVISION
OSA 103-19 (Revised 07/16/2020)			DGS DSA 103-19 (Revised 07/16/	,	DEPARTMENT OF C	GENERAL SERVI	CES STATE OF CALIFORNIA					
ON OF THE STATE ARCHITECT	DEPARTMENT OF GI Page 1					2 of 11						
					ESTS & SPECIAL I	INSPECTI	ONS (Concrete), 2019 CBC		TING OF STRUCTURAL TEST -14 Sections 26.12 & 26.13	TS & SPECIAL INS	PECTIONS (Concrete), 2019 CBC	<b>DSA 10</b> Table 170
cation Number:	STRUCTURAL TESTS & SPECIAL IN School Name: ICON Shelter Systems	SPECTIONS (SOILS), 2019 CBC  School District: PC Submittal	Table 1705A.3; ACI 318-7 Application Number: 04-000000	-14 Sections 26.12 & 26.13 School Name: ICON Shelter System:	ns.		School District: PC Submittal	Application Number: 04-000000 DSA File Number:	School Name: ICON Shelter Systems Increment Number:		School District: PC Submittal Date Created:	Applicati 04-00000 DSA File I
ile Number:	Increment Number:	Date Created: 2021-07-14 05:50:33	DSA File Number:	Increment Number:			Date Created: 2021-07-14 05:50:33	DSA FIJE NUMBER:	mcrement wumber:		2021-07-14 05:50:33	
7. CAST-IN-PLACE CONCRE			17. STRUCTURAL  Material Verification and	L STEEL, COLD-FORMED STEEL AN	ND ALUMINUM USED FO	OR STRUCTU	RAL PURPO	☑ b. Test high-stren	ngth bolts, nuts and washers.	Test I	OR Table 1705A.2.1 Item 1c, 2213A.1; RCSC 2014 Section 7.2; DSA IR 17-	
Test or Special Inspection	Ву	rformed Code References and Notes	Test or Special Ins		Туре	Performed	Code References and Notes	Inspection of High-Stre				Te:
rial Verification and Testing: a. Verify use of required desig		SI Table 1705 A.3 Item 5, 1910 A.1.		ation of all materials and:	Periodic		Table 1705A.2.1 Item 3a–3c. 2202A.1; AISI S100-16 Section A3.1 & A3.2, AISI S240-15 Section A3 & A5, AISI S220-15 Sections A4 & A6. * By special		'snug tight") connections.	Periodic	SI Table 1705A.2.1 Item 2a, 1705A.2.6, 2204A.2; AISC 360-16 J3.1, J3.2 M2.5 & N5.6; RCSC 2014 Section 9.1; DSA IR 17-9.	fille
b. Identifiy, sample, and test r	reinforcing steel. Test	LOR 1910A.2; ACI 318-14 Section 26.6.1.2; DSA IR 17-10. (See Appendix for exemptions.)	with requirements  • Material sizes, typ		, mpi		inspector or qualified technician when performed off-site.	d. Pretensioned a	and slip-critical connections.	*	SI Table 1705A.2.1 Items 2b & 2c, 1705A.2.6, 2204A.2; AISC 360-16 J3 J3.2, M2.5 & N5.6; RCSC 2014 Sections 9.2 & 9.3; DSA IR 17-9. * "Continuous" or "Periodic" depends on the tightening method used.	.1, de
<b>c.</b> During concrete placement for strength tests, perform slu	nt, fabricate specimens Test  Jump and air content	LOR Table 1705A.3 Item 6; ACI 318-14 Sections 26.5 & 26.12.	requirements.  D. Test unidentified	ed materials	Test	LOR	2202A.1.					
tests, and determine the temp concrete.			c. Examine seam w	velds of HSS shapes	Periodic	SI	DSA IR 17-3.	19. WELDI		D1.2 for Aluminum;	705A.2.1 Items 4 & 5; AWS D1.1 and AWS D1.8 for structural steel; AWS AWS D1.3 for cold-formed steel; AWS D1.4 for reinforcing steel; DSA IR 17-	□ d. v
<b>d</b> . Test concrete (f'c).	Test	LOR 1905A.1.15; ACI 318-14 Section 26.12.	<u> </u>	ıment steel fabrication per DSA-app uments.	proved Periodic		Not applicable to cold-formed steel light-frame construction, except for russes (1705A.2.4).	   Verification of Materia	ls, Equipment, Welders, etc.:	3 (See Appendix for	exemptions.)	e.i
e. Batch plant inspection:	See Notes	SI Default of 'Continuous' per 1705A.3.3. If approved by DSA, batch plant inspection may be reduced to 'Periodic' subject to requirements in		GTH BOLTS: RCSC 2		l'	NOSES (TOOKILITY).	Test or Special II	nspection	Type Perf By	ormed Code References and Notes	23.
		Section 1705A.3.3.1, or eliminated per 1705A.3.3.2. (See Appendix for exemptions.)		nd Testing of High-Strength Bolts	s, Nuts and Washers:				er material identification markings per listed on the DSA-approved document:		SI DSA IR 17-3.	Te
f. Welding of reinforcing steel	el. Provide special i	rspection per STEEL, Category 19.1(d) & (e) and/or 19.2(g) & (h) below.	Test or Special Ins	·	, i	Ву	Code References and Notes		er material manufacturer's certificate of	Periodic	SI DSA IR 17-3.	✓ a./
			certificates of com	ation markings and manufacturer's npliance conform to ASTM standard ISA-approved documents.			Table 1705A.2.1 Items 1a & 1b, 2202A.1; AISC 360-16 Section A3.3, J3.1, and N3.2; RCSC 2014 Section 1.5 & 2.1; DSA IR 17-8 & DSA IR 17-9.	c. Verify WPS, we	lder qualifications and equipment.	Periodic	SI DSA IR 17-3.	□ b.
												DGS DSA 10
SA 103-19 (Revised 07/16/2020)			DGS DSA 103-19 (Revised 07,	•	DEPARTMENT O	DE GENERAL SEE	VICES STATE OF CALIFORNIA	DGS DSA 103-19 (Revised 0 DIVISION OF THE STATE ARG	·	DEPARTMENT OF GE	NERAL SERVICES STATE OF CALIF	DIVISION O
N OF THE STATE ARCHITECT	DEPARTMENT OF C Page		NIA DIVISION OF THE STATE ANCI	nitect		ge 6 of 11	VICES STATE OF CALIFORNIA			Page 7 o	of 11	
		SPECTIONS (Steel and Aluminum), 2019 CBC	DSA 103-19: LIST Application Number:	TING OF STRUCTURAL T School Name:	TESTS & SPECIAL	INSPECT	ONS(SIGNATURE), 2019 CBC School District:	DSA 103-19: LIST Application Number: 04-000000	OF REQUIRED VERIFIED RE School Name: ICON Shelter Systems	PORTS, CBC 2019	School District: PC Submittal	
tion Number:	103-16, AISC 341-16, AISC 358-16, AISC 360-16; School Name: ICON Shelter Systems	AISI S100-16 School District: PC Submittal	04-000000 DSA File Number:	ICON Shelter System Increment Number			PC Submittal  Date Created: 2021-07-14 05:50:33	DSA File Number:	Increment Number:		Date Created: 2021-07-14 05:50:33	
	Increment Number:	Date Created: 2021-07-14 05:50:33										
								1				
			Name of Architect or Enginee	eer in general responsible charge:				1. Soils Testing and	d Inspection: Geotechnical Verified R	eport Form DSA 293		_
3. ANCHOR BOLTS AND ANG	ICHOR RODS:		Name of Structural Engineer	r (When structural design has been deleg	gated):				ng and Inspection: Laboratory Verified	·		_
est or Special Inspection		rformed Code References and Notes						<sup>3.</sup> DSA 292			independently contracting SI, Special Inspection Verified Report Form	
. Anchor Bolts and Anchor Ro	ods Test	LOR Sample and test anchor bolts and anchor rods not readily identifiable p procedures noted in DSA IR 17-11.	er Signature of Architect or Stru	uctural Engineer:	Date:			High-Strength B 4. Report Form DS		ry Verified Report Form	DSA 291, or, for independently contracting SI, Special Inspection Verified	i 
o. Threaded rod not used for fo	foundation anchorage. Test	LOR Sample and test threaded rods not readily identifiable per procedures noted in DSA IR 17-11.	Note: To facilitate DS	SA electronic mark-ups and idoas	ication stamp application	n. DSA recom	mends against using secured electronic or digital signatures.					
			NOTE. 10 IACIIITATE DS	and Identifi	жилон эсангр application	יי וecom	DSA STAMP	]				
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A 103-19 (Revised 07/16/2020) N OF THE STATE ARCHITECT	DEPARTMENT OF C	ENERAL SERVICES STATE OF CALIFO	DIVISION OF THE STATE ARC		DEPARTMENT C	OF GENERAL SE age 10 of 11	RVICES STATE OF CALIFORNIA		HITECT	DEPARTMENT OF GENE Page 11 o		NIA

Application Number: 04-000000 ICON Shelter Systems Increment Number: DSA File Number: Date Created: 5. RETAINING WALLS: Type Performed Code References and Notes Test or Special Inspection Continuous GE\* 1705A.6.1. \* By geotechnical engineer or his or her qualified representative. (See Section 2 above). a. Placement, compaction and inspection of backfill. □ b. Placement of soil reinforcement and/or drainage Continuous | GE\* | \* By geotechnical engineer or his or her qualified representative Continuous

# By geotechnical engineer or his or her qualified representative See DSA IR 16-3. c. Segmental retaining walls; inspect placement of units, dowels, connectors, etc. d. Concrete retaining walls. Provide tests and inspections per CONCRETE section below. e. Masonry retaining walls. Provide tests and inspections per MASONRY section below. 6. OTHER SOIL Test or Special Inspection Type Performed Code References and Notes a. Soil Improvements Test GE\* Submit a comprehensive report documenting final soil improvements constructed, construction observation and the results of the confirmation testing and analysis to CGS for final acceptance. \* By geotechnical engineer or his or her qualified representative b. Inspection of Soil Improvements Continuous GE\* \* By geotechnical engineer or his or her qualified representative DGS DSA 103-19 (Revised 07/16/2020) INIA DIVISION OF THE STATE ARCHITECT DEPARTMENT OF GENERAL SERVICES STATE OF CALIFORNIA Page 4 of 11 DSA 103-19: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (Concrete), 2019 CBC Table 1705A.3; ACI 318-14 Sections 26.12 & 26.13
Application Number: School Name: PC Submittal ICON Shelter Systems Date Created: 2021-07-14 05:50:33 DSA File Number: Increment Number: 19.1 SHOP WELDING: Type Performed Code References and Notes Test or Special Inspection a. Inspect groove welds, multi-pass fillet welds, single pass | Continuous fillet welds > 5/16", plug and slot welds. applicable); DSA IR 17-3. ✓ b. Inspect single-pass fillet welds ≤ 5/16", floor and roof
 Periodic
 SI
 1705A.2.2, Table 1705A.2.1 Items 5a.5 & 5a.6; AISC 360-16 (and AISC) 341-16 as applicable); DSA IR 17-3. deck welds. c. Inspect welding of stairs and railing systems. Periodic SI 1705A.2.1; AISC 360-16 (and AISC 341-16 as applicable); AWS D1.1 & D1.3; d. Verification of reinforcing steel weldability Periodic SI 1705A.3.1; AWS D1.4; DSA IR 17-3. Verify carbon equivalent reported on other than ASTM A706. Continuous SI Table 1705A.2.1 Item 5b, 1705A.3.1, Table 1705A.3 Item 2, 1903A.8; AWS D1.4; DSA IR 17-3. e. Inspect welding of reinforcing steel. 23. ANCHOR BOLTS AND ANCHOR RODS: Type Performed Code References and Notes Test or Special Inspection ☑ a. Anchor Bolts and Anchor Rods LOR Sample and test anchor bolts and anchor rods not readily identifiable per procedures noted in DSA IR 17-11. LOR Sample and test threaded rods not readily identifiable per procedures noted in DSA IR 17-11. **b.** Threaded rod not used for foundation anchorage.

DEPARTMENT OF GENERAL SERVICES

STATE OF CALIFORNIA

DSA 103-19: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (SOILS), 2019 CBC

DIV. OF THE STATE ARCHITECT APP: 04-120013 PC REVIEWED FOR SS 🗹 FLS 🗹 ACS 🗹 CG 🗌 DATE: 08/06/2021

RH/DSA-PC

ANGEL

4/2/2021

ARCHITECTS ENGINEERS

2700 SATURN ST I BREA, CA 92821

T. 714.524.1870 | F. 714.524.1875 WWW.JRMA.COM

ICON STD

DRAWN BY

DATE

REV

REV DATE

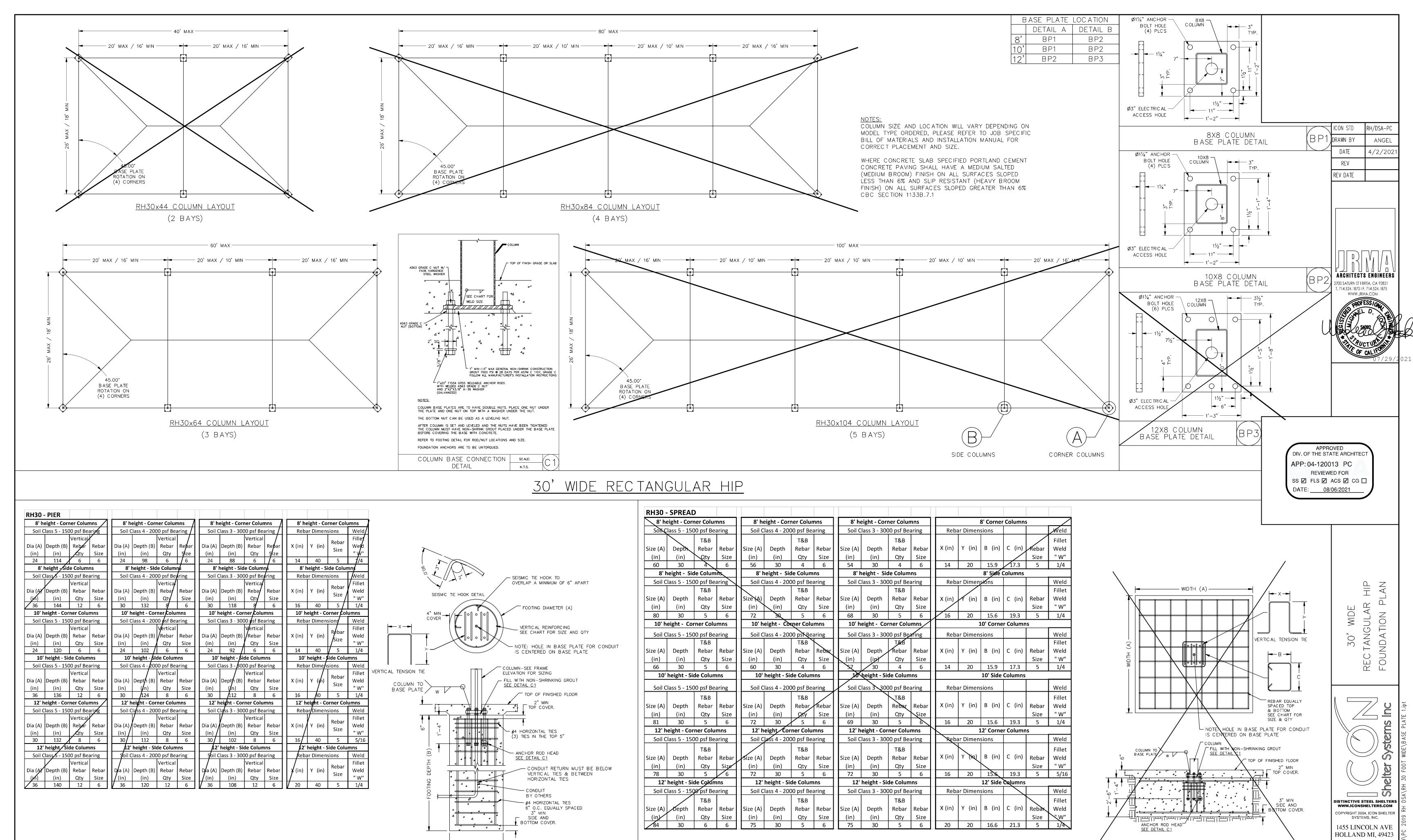
103 DSA

DISTINCTIVE STEEL SHELTERS COPYRIGHT 2004, ICON SHELTER 1455 LINCOLN AVE

HOLLAND MI, 49423

616.396.0919 800.748.0985 616.396.0944 FX

PRE-CHECK (PC) DOCUMENT Code: 2019 CBC A separate project application for construction is required.



FOOTING DIAMETER (A)

SEE DETAILS BP1. BP2 OR BP3 FOR ANCHOR BOLT PATTERNS

BP1 & BP2 ARE (4) BOLT PATTERN WHILE B3 IS A (6) BOLT

PRE-CHECK (PC) DOCUMENT
Code: 2019 CBC
A separate project application for construction is required.

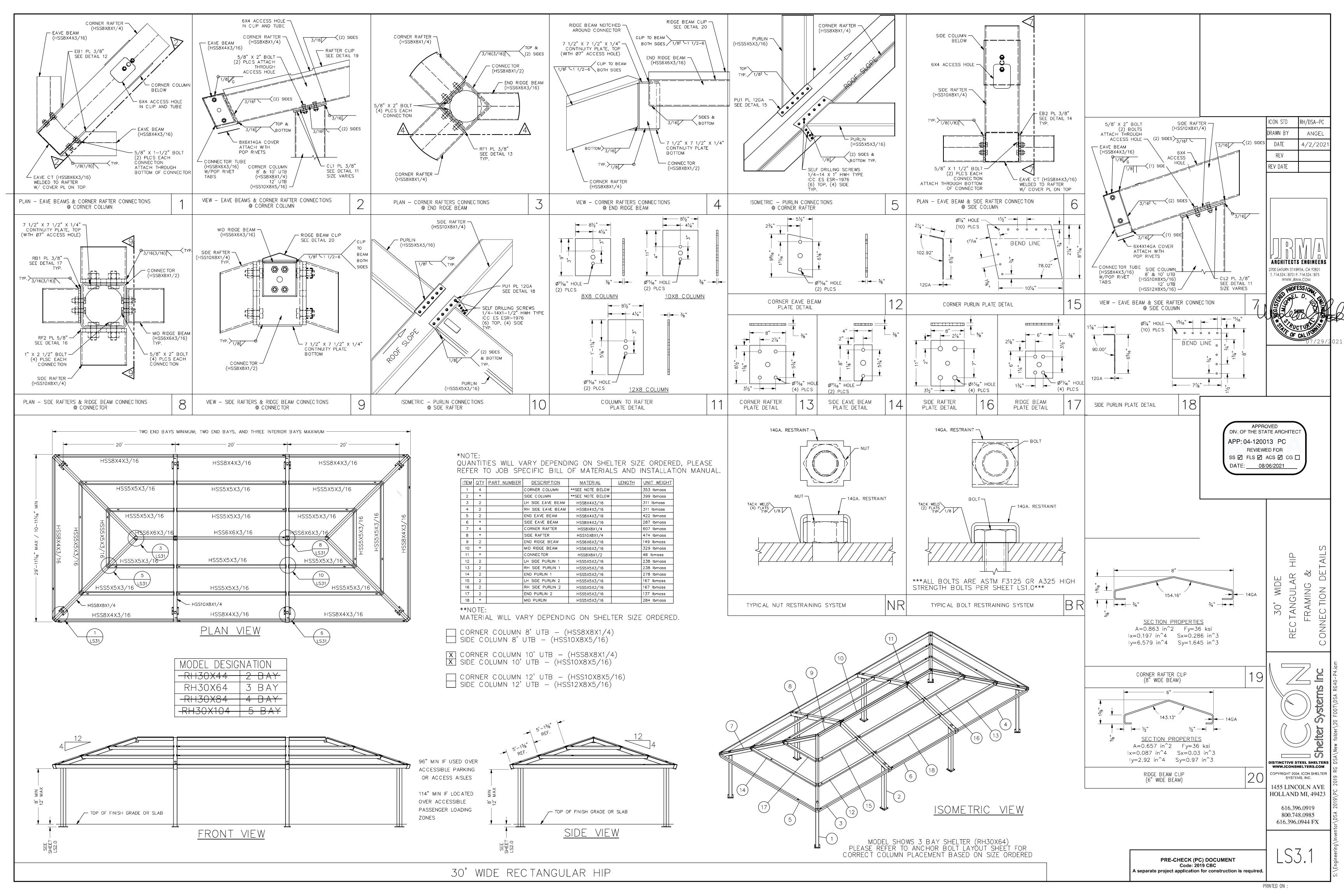
DETAILS BP1. BP2 OR BP3 FOR ANCHOR BOLT PAT**N**ERNS

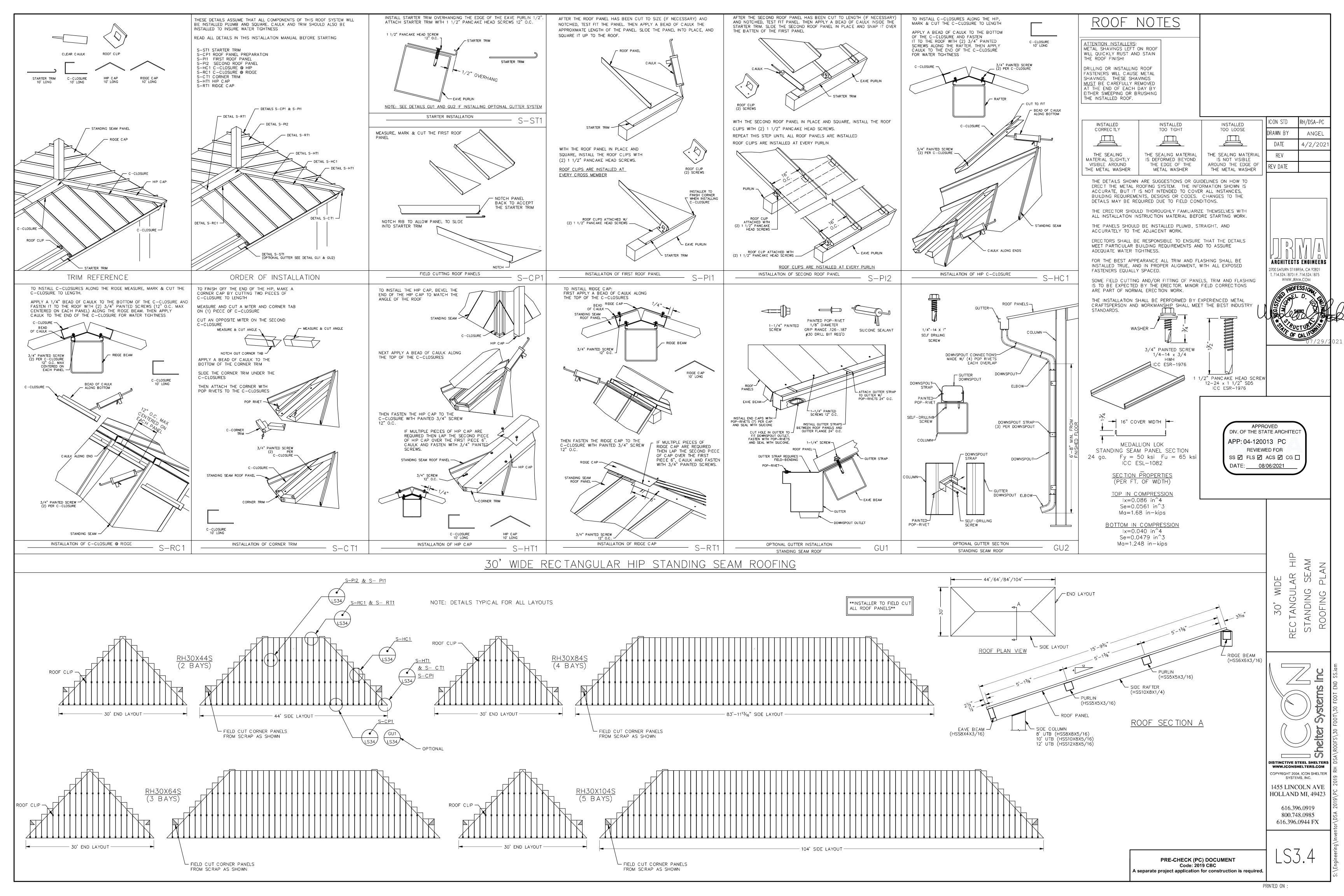
 $t ar{eta}$ P1 & BP2 are (4) bolt pattern while b3 is a (6) b $\delta$ L^

PRINTED ON :

616.396.0919 800.748.0985

616.396.0944 FX



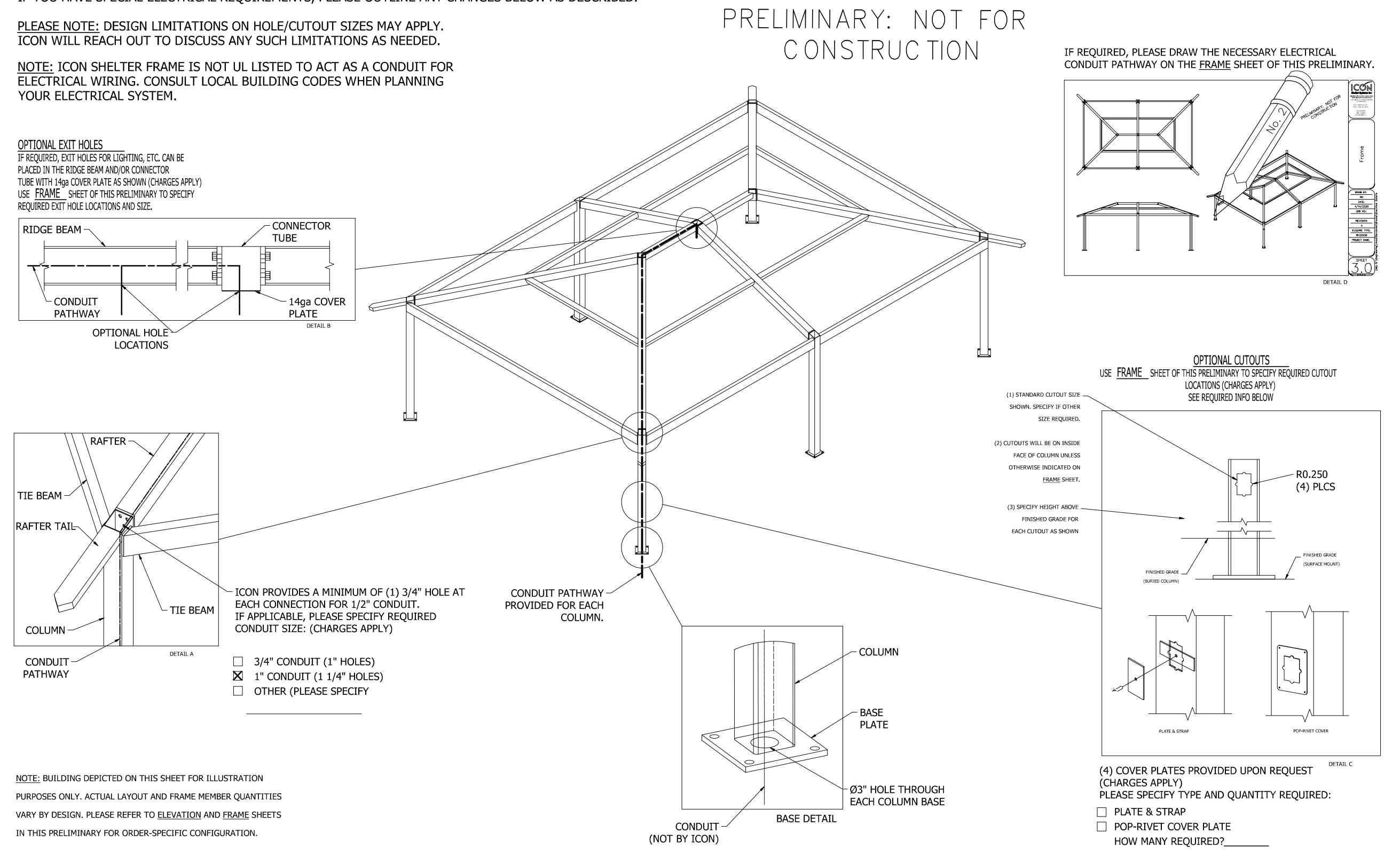


### ELECTRICAL INFORMATION - RECTANGULAR HIP

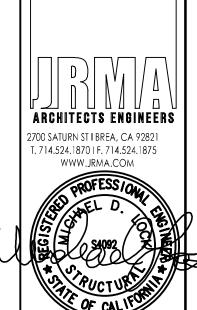
ICON'S STANDARD ELECTRICAL IS DESIGNED TO ACCOMMODATE Ø1/2" CONDUIT WITH A Ø3" INLET HOLE ON THE BOTTOM OF EACH COLUMN. THE CONDUIT PATHWAY RUNS THROUGH THE COLUMN, RAFTER, AND RIDGE BEAM THROUGH ALL BOLTED CONNECTIONS AS SHOWN. IF YOU HAVE SPECIAL ELECTRICAL REQUIREMENTS, PLEASE OUTLINE ANY CHANGES BELOW AS DESCRIBED.

2. ELECTRICAL EXIT HOLES (DETAIL B) 3. ELECTRICAL ACCESS & COVER PLATES (DETAIL C) 4. ELECTRICAL CONDUIT PATHWAY (DETAIL D)

1. CONDUIT HOLE SIZE (DETAIL A)

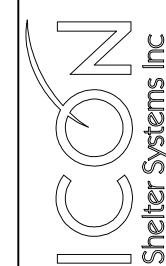


REV DATE



APPROVED DIV. OF THE STATE ARCHITEC APP: 04-120013 PC SS 🗹 FLS 🗹 ACS 🗹 CG 🗌

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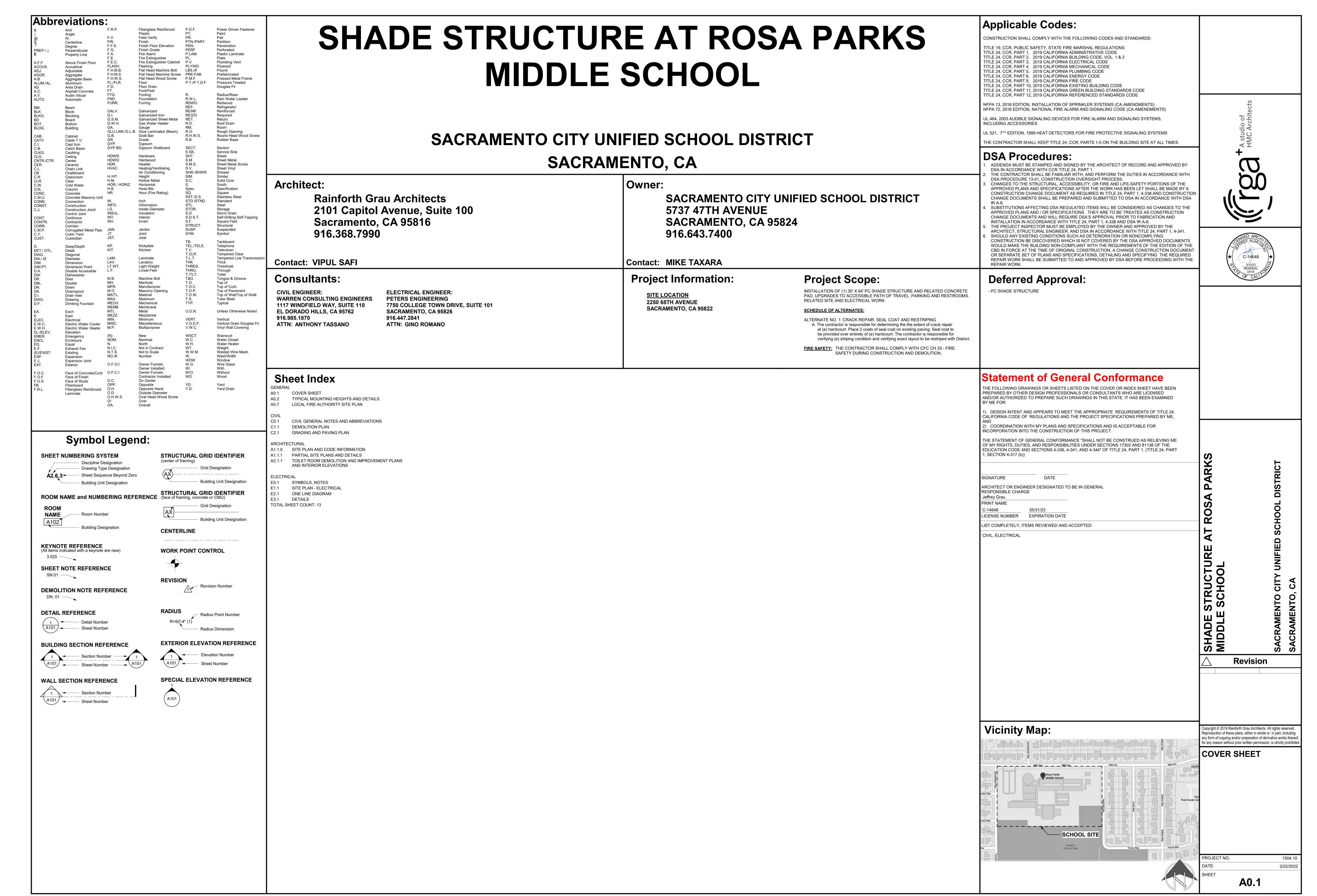


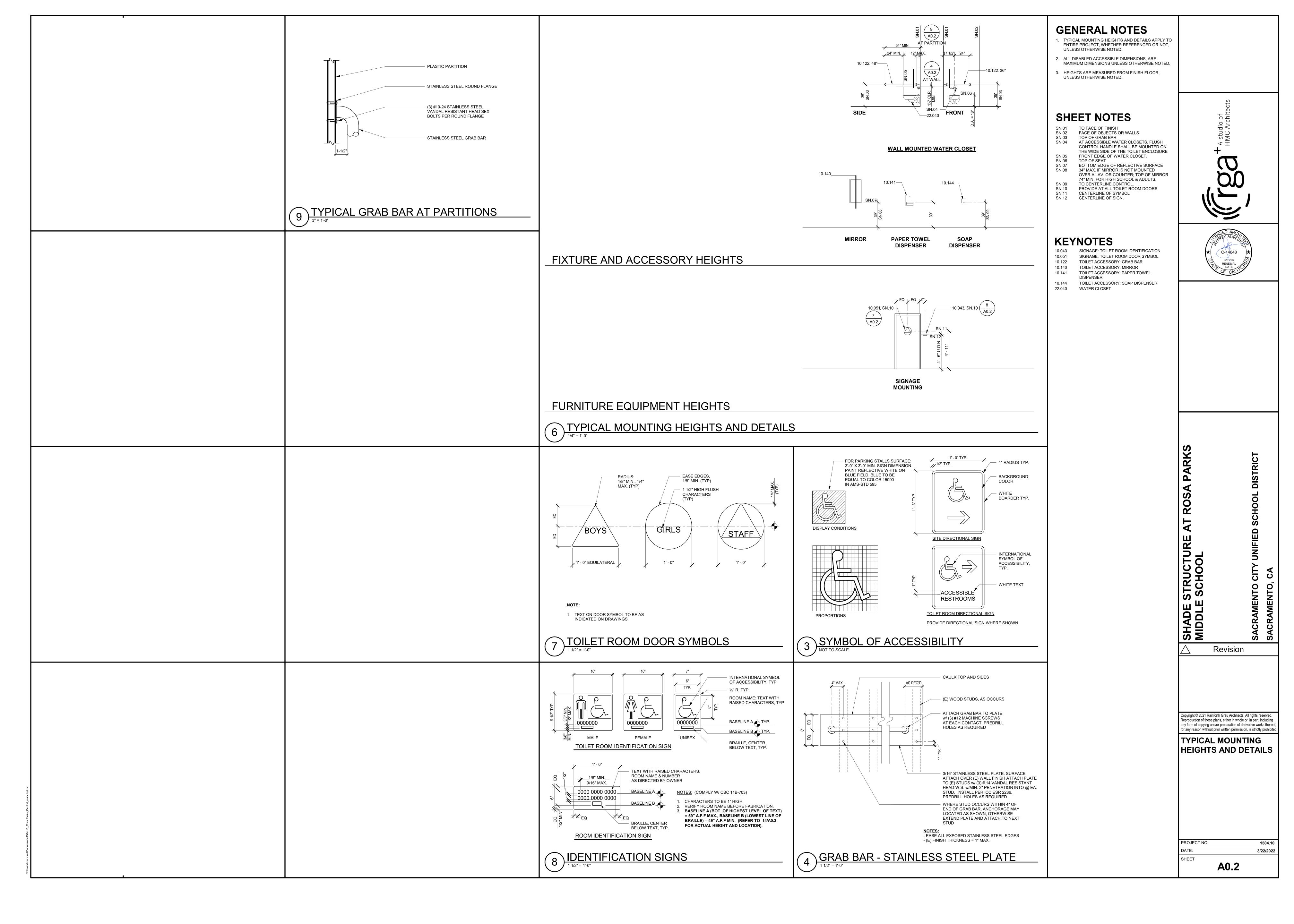
COPYRIGHT 2004, ICON SHELTER 1455 LINCOLN AVE HOLLAND MI, 49423 616.396.0919

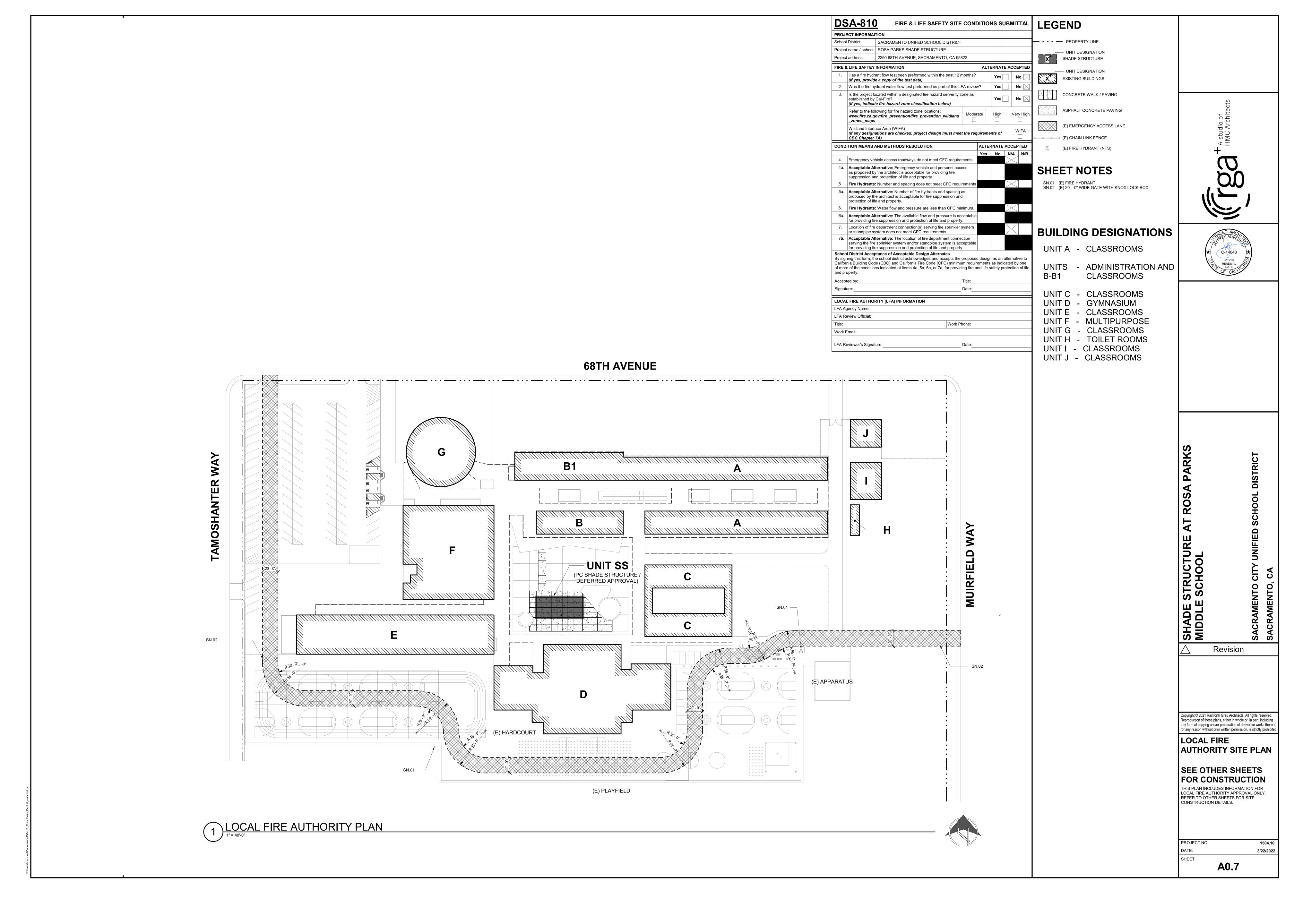
> 800.748.0985 616.396.0944 FX

PRE-CHECK (PC) DOCUMENT Code: 2019 CBC

A separate project application for construction is required.







= STORM DRAIN LINE

= SIGN

= POST OR BOLLARD

= GROUND ELEVATION

= HARD SURFACE ELEVATION

(SIZE & DIRECTION OF FLOW)  $\underline{\phantom{a}}$  = STORM DRAIN LINE (RECORD INFORMATION)  $\underline{\phantom{a}}$  12"SD  $\underline{\phantom{a}}$  = STORM DRAIN LINE (UNDERGROUND LOCATING) = STORM DRAIN MANHOLE = STORM DRAIN CLEANOUT = DROP INLET = AREA DRAIN = RAIN WATER LEADER

= DOWNSPOUT SANITARY SEWER LINE (SIZE & DIRECTION OF FLOW) (RECORD INFORMATION) \_\_\_\_\_\_\_\_\_\_\_\_\_\_\_\_ = SANITARY SEWER LINE (UNDERGROUND LOCATING)

= SANITARY SEWER MANHOLE = SANITARY SEWER CLEANOUT = WATER LINE (SIZE INDICATED) - -W - -W = WATER LINE (RECORD INFORMATION)

-W- - W = WATER LINE (UNDERGROUND LOCATING)

= WATER MANHOLE = WATER VALVE = WATER METER

= WATER BOX = IRRIGATION CONTROL VALVE = FIRE HYDRANT = BACKFLOW PREVENTER

= SPRINKLER = HOSE BIBB -OH-E- = OVERHEAD ELECTRIC LINE

---E = UNDERGROUND ELECTRIC LINE ---E---= UNDERGROUND ELECTRIC LINE

— — E — — = UNDERGROUND ELECTRIC LINE (UNDERGROUND LOCATING) = ELECTRIC MANHOLE

= UTILITY POLE (WITH GUY WIRE) = ELECTRIC METER

= ELECTRIC BOX = STREET LIGHTING BOX  $\square$   $\square$   $\square$   $\square$   $\square$  = LIGHT STANDARD

□ □ □ □ □ = SIGNAL LIGHT = FLOOD LIGHT = ELECTRICAL OUTLET ---- G ---- = GAS LINE (SIZE INDICATED)

---G--- = GAS LINE (RECORD INFORMATION)--G--= GAS LINE (UNDERGROUND LOCATING)

= GAS MANHOLE = GAS VALVE = GAS METER --- T --- = TELEPHONE LINE

---T---= TELEPHONE LINE (RECORD INFORMATION) -- T -- = TELEPHONE LINE (UNDERGROUND LOCATING)

= STORM DRAIN BOX = TRAFFIC SIGNAL BOX

22 CPS CHISELED "+"

23 CPS CHISELED "+"

NUMBER DESCRIPTION NORTHING EASTING ELEV 1 CPS CHISELED "+" 11409.28 9207.41 10.28 3 CPF BM 337-D2E EL=11.733 11369.75 9362.98 11.74 4 CPS CHISELED "+" 11410.81 9382.69 10.62 5 CPF CURB TIE 11400.64 9027.62 9.26 11 CPS PICKER 11197.36 9649.32 12.50 12 CPS CHISELED "+" 11071.71 9558.41 12.50 13 CPS CHISELED "+" 11117.15 9646.01 12.38 14 CPS CHISELED "+" 10885.53 9499.37 11.58 19 CPS CHISELED "+" 11162.82 9060.94 11.47 20 CPS CHISELED "+" 11043.76 9196.68 13.13 21 CPS CHISELED "+" 11036.94 9384.80 12.59

11183.46 9482.11 12.36

11249.01 9246.31 12.30

### CIVIL ABBREVIATIONS AND LEGEND

HIGH DENSITY POLYETHYLENE PIPE

PIPE INVERT ELEVATION

PORTLAND CEMENT CONCRETE

JOINT UTILITY POLE

HIGH POINT

LINEAL FEET

**MOWSTRIP** 

OVERHEAD

LEFT

LIP OF GUTTER

NOT TO SCALE

PLANTER DRAIN

PROPERTY LINE

RIGHT OF WAY

STORM DRAIN

SANITARY SEWER

SCHEDULE

**STANDARD** 

SIDEWALK

UTILITY

WATER

WITHOUT

WATER VALVE

WITH

TELEPHONE

TOP OF CURB

TRENCH DRAIN

TELEPHONE POLE

TOP OF SEAT WALL

VITRIFIED CLAY PIPE

UNDERGROUND

POWER POLE

POST INDICATOR VALVE

POLYVINYL CHLORIDE

STORM DRAIN MANHOLE

SANITARY SEWER MANHOLE

TRENCH DRAIN CATCH BASIN

TOP OF RAMP ELEVATION

TOP OF WALK ELEVATION

UNLESS OTHERWISE NOTED

TOP OF RETAINING WALL

SUBGRADE ELEVATION

PUBLIC UTILITY EASEMENT

REINFORCED CONCRETE PIPE

MANHOLE RIM ELEVATION (SOLID COVER)

REDUCED PRESSURE BACKFLOW PREVENTER

CATV

COMM

CONC.

CONST.

CO

CR

DWG

**ESMT** 

**HDPE** 

SCH

STD

W/

W/O

NOTE: NOT ALL SYMBOLS MAY BE USED ON THESE PLANS.

<u>LEGEND</u> **ABBREVIATIONS** NOTE: NOT ALL ABBREVIATIONS MAY BE USED ON THESE PLANS. PROPOSED GRADING & DRAINAGE SYMBOLS: AGGREGATE BASE ASPHALTIC CONCRETE 8" SD STORM DRAIN LINE AREA DRAIN (SIZE AND FLOW SHOWN) ASSESSOR'S PARCEL NUMBER AIR RELEASE VALVE STORM DRAIN MANHOLE AGGREGATE SUB-BASE BLOW-OFF VALVE (SDMH) **BUTTERFLY VALVE** BACK OF WALK ——— CATCH BASIN (CB) **CENTERLINE** CATCH BASIN ——— DROP INLET (DI) CLASS CORRUGATED METAL PIPE —— AREA DRAIN (AD) CABLE TELEVISION **CLEANOUT** PLANTER DRAIN (PD) OR COMMUNICATION FLOOR DRAIN (FD) CONCRETE CONSTRUCT STORM DRAIN CLEANOUT CURB RETURN CONCRETE SURFACE ELEVATION DOUBLE CHECK VALVE DOUBLE DETECTOR CHECK VALVE FINISHED FLOOR ELEVATION DECOMPOSED GRANITE DROP INLET BUILDING PAD ELEVATION PAD = 99.33DIAMETER DUCTILE IRON PIPE CONCRETE SIDEWALK DRAWING DOWNSPOUT GRADED DIRECTION FOR ELECTRIC DRAINAGE FLOW EDGE OF PAVEMENT EASEMENT  $\longrightarrow$ ---- SWALE **EXISTING** FIRE SERVICE LINE FIRE DEPARTMENT CONNECTION FLOWLINE TREE TO BE REMOVED SANITARY SEWER FORCE MAIN FINISHED FLOOR ELEVATION RETAINING WALL FIRE HYDRANT GRATE ELEVATION PROPOSED SANITARY SEWER SYMBOLS: GRADE ELEVATION 6" SS SANITARY SEWER LINE GATE VALVE HOSE BIBB HEADER BOARD

(SIZE AND FLOW SHOWN) SANITARY SEWER MANHOLE (SSMH) SEWER CLEANOUT FLUSHER BRANCH PROPOSED WATER SYMBOLS:

8" RW RECLAIMED WATER LINE & SIZE 8" IRR IRRIGATION SERVICE LINE & SIZE 8" NP NON POTABLE WATER LINE & SIZE 8" SP FIRE SPRINKLER SERVICE LINE & SIZE <del>────</del> GATE VALVE ———M——— WATER METER

→ → → FH FIRE HYDRANT ASSEMBLY FIRE DEPARTMENT CONNECTION DETECTOR CHECK VALVE DOUBLE DETECTOR CHECK VALVE REDUCED PRESSURE BACKFLOW PREVENTER BUTTERFLY VALVE AIR RELEASE VALVE + SIZE BLOW-OFF VALVE + SIZE POST INDICATOR VALVE

### **DEMOLITION GENERAL NOTES**

- IN THE EVENT THAT ANY UNUSUAL CONDITIONS NOT COVERED BY THE GEOTECHNICAL INVESTIGATION REPORT OR ARE ENCOUNTERED DURING GRADING OPERATIONS THE GEOTECHNICAL ENGINEER AND THE ARCHITECT SHALL BE IMMEDIATELY NOTIFIED FOR DIRECTIONS.
- 2. NO BURNING OR BLASTING SHALL BE PERMITTED.
- ADDITIONAL DEMOLITION INFORMATION MAY BE SHOWN ON THE GRADING, DRAINAGE, AND UTILITY PLANS, AND THOSE PLANS PREPARED BY OTHER DISCIPLINES FOR THIS PROJECT.
- 4. ALL DEMOLISHED ITEMS SHALL BE DISPOSED OF OFFSITE AT A SUITABLE, LEGAL, DUMP SITE OR OTHER FACILITY.
- 5. ALL DISPOSED OF MATERIALS SHALL BE RECYCLED IF POSSIBLE
- 6. THE TYPES, LOCATIONS, SIZES AND/OR DEPTHS OF EXISTING UNDERGROUND UTILITIES AS SHOWN IN THESE PLANS WERE OBTAINED FROM SOURCES OF VARYING RELIABILITY. THE CONTRACTOR IS CAUTIONED THAT ONLY ACTUAL EXCAVATION WILL REVEAL THE TYPES, EXTENT, SIZES, LOCATIONS, AND DEPTHS OF SUCH UNDERGROUND UTILITIES. A REASONABLE EFFORT HAS BEEN MADE TO LOCATE AND DELINEATE ALL KNOWN UNDERGROUND UTILITIES. HOWEVER, WARREN CONSULTING ENGINEERS CAN ASSUME NO RESPONSIBILITY FOR THE COMPLETENESS OR ACCURACY OF ITS DELINEATION OF SUCH UNDERGROUND UTILITIES, NOR FOR THE EXISTENCE OF OTHER BURIED OBJECTS OR UTILITIES WHICH MAY BE ENCOUNTERED BUT WHICH ARE NOT SHOWN ON THESE DRAWINGS. THE CONTRACTOR OR ANY SUBCONTRACTOR FOR THIS CONTRACT SHALL NOTIFY THE DISTRICT TWO (2) WORKING DAYS IN ADVANCE OF PERFORMING ANY EXCAVATION WORK IN ORDER TO VERIFY TO THE GREATEST EXTENT POSSIBLE THE EXISTING UTILITY LINES, CONFLICTS AND PROPOSED UTILITY CONNECTION POINTS.
- 7. THE SCHOOL DISTRICT SHALL HAVE SALVAGE RIGHTS TO ANY DEMOLISHED ITEMS SHOWN HEREON. THE CONTRACTOR SHALL GIVE THE DISTRICT NOTICE 7 DAYS PRIOR TO THE START OF DEMOLITION. THE DISTRICT SHALL MOVE ANY RETAINED ITEMS OUT OF THE CONTRACTORS WORK AREA, UNLESS ANOTHER ARRANGEMENT IS MADE WITH THE CONTRACTOR. ANY REMAINING ITEMS BECOME THE PROPERTY OF THE CONTRACTOR AND SHALL BE REMOVED FROM THE SITE. ANY ITEMS NOT SHOWN FOR REMOVAL SHALL REMAIN AND SHALL BE PROTECTED FROM DAMAGE DURING CONSTRUCTION TO A REASONABLE
- 8. EXISTING UTILITY STRUCTURES IN AREAS OF NEW PAVING SHALL BE REMOVED AND REPLACED WITH NEW BOX/COVER AT NEW GRADE UNLESS SPECIFICALLY NOTED OTHERWISE.
- 9. ITEMS OUTSIDE THE LIMITS OF DEMOLITION SHALL REMAIN AND BE PROTECTED FROM DAMAGE DURING CONSTRUCTION.
- 10. EXISTING UTILITY STRUCTURES AND PIPING NOT SHOWN ON DEMOLITION PLAN TO BE REMOVED SHALL REMAIN AND BE PROTECTED.

### UTILITY VERIFICATION NOTE

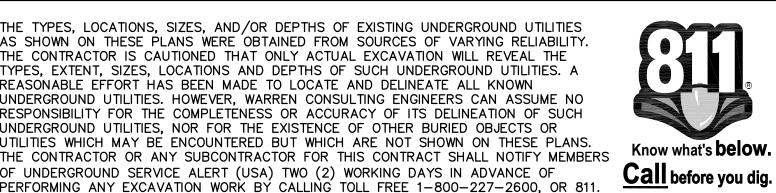
PRIOR TO THE START OF CONSTRUCTION, VERIFY AND POTHOLE ALL UTILITY POINTS OF CONNECTION FOR LOCATION DEPTH. AND SIZE, IF CONFLICT IS FOUND, CONTACT THE ENGINEER IMMEDIATELY FOR

### IRRIGATION DEMOLITION NOTE

WITHIN LANDSCAPE AREAS TO BE DEMOLISHED THERE MAY BE EXISTING IRRIGATION LINES NOT SHOWN ON THIS PLAN. CONTRACTOR SHALL REMOVE LATERAL LINES AND HEADS ENCOUNTERED. MAIN LINES AND CONTROL WIRES MAY ONLY BE REMOVED PROVIDED THAT ROUTING IS KNOWN AND REMOVAL WILL NOT DEACTIVATE AN IRRIGATION SYSTEMS INTENDED TO REMAIN. IF CONFLICT IS FOUND, CONTACT THE ENGINEER FOR DIRECTION.

### **GENERAL NOTES:**

 THE TYPES, LOCATIONS, SIZES, AND/OR DEPTHS OF EXISTING UNDERGROUND UTILITIES AS SHOWN ON THESE PLANS WERE OBTAINED FROM SOURCES OF VARYING RELIABILITY THE CONTRACTOR IS CAUTIONED THAT ONLY ACTUAL EXCAVATION WILL REVEAL THE TYPES, EXTENT, SIZES, LOCATIONS AND DEPTHS OF SUCH UNDERGROUND UTILITIES. A REASONABLE EFFORT HAS BEEN MADE TO LOCATE AND DELINEATE ALL KNOWN UNDERGROUND UTILITIES. HOWEVER. WARREN CONSULTING ENGINEERS CAN ASSUME NO RESPONSIBILITY FOR THE COMPLETENESS OR ACCURACY OF ITS DELINEATION OF SUCH UNDERGROUND UTILITIES, NOR FOR THE EXISTENCE OF OTHER BURIED OBJECTS OR UTILITIES WHICH MAY BE ENCOUNTERED BUT WHICH ARE NOT SHOWN ON THESE PLANS. THE CONTRACTOR OR ANY SUBCONTRACTOR FOR THIS CONTRACT SHALL NOTIFY MEMBERS OF UNDERGROUND SERVICE ALERT (USA) TWO (2) WORKING DAYS IN ADVANCE OF



. WARREN CONSULTING ENGINEERS, INC. (WCE) ASSUMES NO RESPONSIBILITY FOR ERRORS IN PHYSICAL LOCATION OF IMPROVEMENTS, HORIZONTAL OR VERTICAL, IF STAKED BY OTHERS. IN ADDITION, ANY SUCH ERRORS IN PHYSICAL LOCATION MAY AFFECT THE INTENDED DESIGN OF SUCH IMPROVEMENTS AND WCE CANNOT BE HELD RESPONSIBLE FOR SUCH CONDITIONS WHICH ARE A RESULT OF ERRORS IN SURVEYING, OR IMPROPER CONSTRUCTION.

- 3. IF SUBSURFACE CULTURAL RESOURCES, REMAINS, AND/OR ARTIFACTS ARE UNCOVERED DURING PROJECT CONSTRUCTION, ALL WORK IN THE VICINITY SHALL BE STOPPED UNTIL SUCH ITEMS CAN BE ASSESSED BY AN APPROPRIATE MEMBER OF THE COUNTY ENVIRONMENTAL IMPACT SECTION STAFF.
- 4. CONTRACTOR AGREES THAT HE/SHE SHALL ASSUME SOLE AND COMPLETE RESPONSIBILITY FOR JOB SITE CONDITIONS DURING THE COURSE OF CONSTRUCTION OF THIS PROJECT, INCLUDING SAFETY OF ALL PERSONS AND PROPERTY: THAT THIS REQUIREMENT SHALL APPLY CONTINUOUSLY AND SHALL NOT BE LIMITED TO NORMAL WORKING HOURS: AND THAT THE CONTRACTOR SHALL DEFEND, INDEMNIFY AND HOLD THE OWNER AND ENGINEER HARMLESS FROM ANY AND ALL LIABILITY, REAL OR ALLEGED, IN CONNECTION WITH THE PERFORMANCE OF WORK ON THIS PROJECT, EXCEPTING FOR LIABILITY ARISING FROM THE SOLE NEGLIGENCE OF THE OWNER OR ENGINEER.
- 5. THE CONTRACTOR SHALL OBTAIN AN EXCAVATION PERMIT FROM THE STATE OF CALIFORNIA DEPARTMENT OF INDUSTRIAL SAFETY FOR ALL EXCAVATIONS OF 5 FEET OR MORE IN DEPTH.
- 6. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO MAKE ALL NECESSARY PRE-BID AND PRE-CONSTRUCTION SITE INSPECTION, AND/OR OBSERVATIONS ON THE SITE TO PRE-DETERMINE ALL HIS/HER MEANS AND METHODS NECESSARY TO COMPLETE THE IMPROVEMENTS SHOWN ON THESE PLANS AND PER THE PROJECT SPECIFICATIONS. IT IS THE CONTRACTORS RESPONSIBILITY TO DETERMINE, AND INCLUDE IN HIS/HER CONTRACT, ALL MEANS AND METHODS NECESSARY TO PERFORM A COMPLETE AND ACCEPTABLE JOB.
- '. WHERE IMPROVEMENTS LIE WITHIN AN EXISTING DEVELOPED AREA, CONTRACTOR SHALL USE CAUTION WHEN ACCESSING THE SITE THROUGH THESE EXISTING IMPROVEMENTS. IT IS THE CONTRACTORS RESPONSIBILITY TO PROTECT ANY SUCH EXISTING IMPROVEMENTS OUTSIDE THE PROJECT BOUNDARY. OR EXISTING IMPROVEMENTS WITHIN THE BOUNDARY WHICH ARE TO REMAIN. PROPER PRECAUTIONS SHALL BE PROVIDED AND MAINTAINED THROUGHOUT CONSTRUCTION. ANY DAMAGE SHALL BE REPAIRED OR REPLACED TO THE SATISFACTION OF THE
- 8. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO KEEP DETAILED RECORDS OF MINOR CHANGES OR ADJUSTMENTS MADE DURING CONSTRUCTION (WHICH WERE NOT FORMALLY ISSUED). UPON PROJECT COMPLETION, THESE RECORDS AND/OR INFORMATION SHALL BE PROVIDED TO THE OWNER AND WARREN CONSULTING ENGINEERS, INC. UNLESS AN OFFICIAL "AS-BUILT" SET OF PLANS IS A REQUIREMENT OF THE CONTRACT. IF AS-BUILT PLANS ARE A REQUIREMENT OF THE CONTRACT, REFER TO SPECIFICATIONS FOR AS-BUILT DELIVERABLE REQUIREMENTS.
- 9. IN VEHICULAR PATHWAYS, EXISTING ASPHALTIC AND/OR CONCRETE SURFACES SHALL BE CUT TO A NEAT AND STRAIGHT LINE, PARALLEL OR PERPENDICULAR TO THE VEHICULAR TRAVELED PATH. THIS IS TYPICALLY THE ROADWAY CENTERLINE, BUT MAY VARY. THAT SAWCUT EDGE SHALL BE PROTECTED FROM DAMAGE DURING CONSTRUCTION SO A CLEAN EDGE REMAINS FOR PATCH BACK.. IF EDGE IS DAMAGED, A NEW SAW CUT WILL BE REQUIRED. THE EXPOSED EDGE SHALL BE "TACKED" WITH EMULSION PRIOR TO PAVING.
- 10. NO BURNING OR BLASTING SHALL BE ALLOWED ONSITE UNLESS SPECIFICALLY ADDRESSED ON PLANS, OR SPECIFICALLY APPROVED AND COORDINATED WITH THE ARCHITECT, ENGINEER, AND LOCAL AGENCY OR OTHER ADMINISTRATIVE AUTHORITY.
- 11. SUBGRADE AND RESULTING FINISHED GRADE SHALL BE CONSTRUCTED SMOOTH AND UNIFORM BETWEEN SPOT ELEVATIONS, CONTOURS OR OTHER STRUCTURE ELEVATIONS SHOWN ON GRADING OR OTHER PLANS. NO MOUNDS,
- RUTS, DEPRESSIONS OR OTHER GRADING DEFICIENCIES WILL BE ALLOWED UNLESS SPECIFICALLY SHOWN ON PLANS. 12. ON NEW WATER SYSTEMS, SERVICE LATERALS SHALL BE MADE USING APPROPRIATE "TEE" AND "WYE" FITTINGS.
- SADDLE TAPS WILL ONLY BE ALLOWED WHEN MAKING CONNECTIONS TO EXISTING WATER MAINS. 13. CURING COMPOUND SHALL BE APPLIED IN A CONTINUOUS SOLID WET FLOWING COAT. ANY "SPOTTY" APPLICATIONS

SHALL BE RECOATED IMMEDIATELY. APPLICATION SHALL BE INSPECTED BY PROJECT INSPECTOR DURING

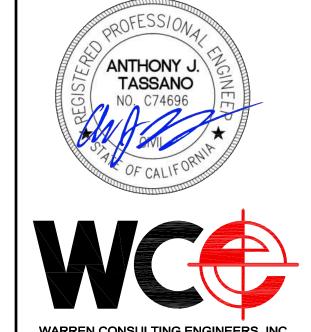
- APPLICATION. 14. EMBEDMENT OF FEATURES IN CONCRETE PAVING, CURBS, OR WALLS, SUCH AS SQUARE OR ROUND TUBING, POSTS, OR COLUMNS, STEEL BOLTED PLATES, OR OTHER STRUCTURES, SHALL REQUIRE ADDITIONAL SCORE OR EXPANSION JOINTS TO PREVENT UNCONTROLLED CRACKING. THOSE ADDITIONAL JOINTS MAY OR MAY NOT BE SPECIFICALLY
- SHOWN ON PLANS BUT SHALL BE PROVIDED BY THE CONTRACTOR. 15. EMBEDMENT OF FEATURES IN CONCRETE PAVING, CURBS, OR WALLS, SUCH AS SQUARE OR ROUND TUBING, POSTS, OR COLUMNS, STEEL BOLTED PLATES, OR OTHER STRUCTURES, SHALL REQUIRE A MINOR ADJUSTMENT OF REBAR WITHIN CONCRETE TO ALLOW FOR SUCH STRUCTURE. THAT REBAR ADJUSTMENT MAY NOT BE SPECIFICALLY SHOWN
- 16. NO MORE THAN 1 GALLON OF WATER PER YARD OF CONCRETE CAN BE ADDED TO THE TRUCK AFTER ARRIVAL TO PROJECT SITE. THE ADDITION OF WATER CAN ONLY BE ADDED UNDER THE SUPERVISION OF THE CONCRETE INSPECTOR OR LABORATORY TECHNICIAN.
- 17. WHEN PUMPING CONCRETE FOR PLACEMENT, ABSOLUTELY NO WATER IS TO BE ADDED TO PUMP HOPPER. ANY WATER ADDED TO HOPPER WILL BE REASON FOR CONCRETE REJECTION AT THE CONTRACTORS EXPENSE.
- 18. ALL CONTRACTION/CONSTRUCTION JOINTS "CJ" SHALL BE 1/4 THE SLAB THICKNESS DEEP, BUT NO LESS THAN 1" FOR CONTROLLING OF CRACKING. CONTRACTOR SHALL EXERCISE CAUTION WHEN FINAL TROWELING OF CONCRETE SO AS NOT TO FILL IN THESE JOINTS WITH CONCRETE CREAM. ANY CRACKS OUTSIDE OF JOINTS WHICH WERE CONSTRUCTED LESS THAN 1" DEEP, SHALL BE CAUSE FOR CONCRETE SLAB(S) TO BE REMOVED AND REPLACE AT
- 19. ANY SCREED BOARDS SET WITHIN CONCRETE SLABS SHALL BE AN "OVERHEAD SCREED" SO THERE IS NO INTERFERENCE WITH THE PLACEMENT AND ALIGNMENT OF SLAB REINFORCING.
- 20. 3-1/2" FELT JOINTS WILL NOT BE ACCEPTED. PROVIDE A FULL 4" FELT JOINT FOR 4" SLAB CONSTRUCTION, AND A 6" FELT JOINT FOR A 6" SLAB SLAB CONSTRUCTION.
- 21. SHOULD ANY SHRINKAGE CRACKS OCCUR OUTSIDE OF EITHER THE EXPANSION JOINTS OR CRACK CONTROL JOINTS, THEN THE CONCRETE SLAB SHALL BE SAWCUT AT THE NEAREST JOINTS ON EACH SIDE OF THE CRACK AND THE CONCRETE SECTION SHALL BE, REMOVED AND REPLACED. NEW CONCRETE SHALL BE DOWELED INTO EXISTING CONCRETE PER DRAWING DETAIL.
- 22. ALL AREAS DISTURBED BY GRADING OPERATIONS WHETHER SHOWN ON THE DRAWINGS OR NOT SHALL BE HYDRO SEEDED UNLESS OTHERWISE NOTED. HYDRO SEEDING SHALL CONFORM TO LOCAL CITY/COUNTY STANDARDS.
- 23. REPAIR OR PATCHING OF GALVANIZED METALS, SUCH AS AFTER WELDING GALVANIZED COMPONENTS, SHALL BE MADE USING A ZINC COMPOSITION "HOT STICK" APPLICATION PER ASTM A 780-01. GALVANIZING PAINTS WILL NOT

### **GENERAL PAVING SURFACE NOTES:**

- 1. PROVIDE EQUIVALENT OF MEDIUM BROOM FINISH AT SLOPES UP TO 5.99%, TYPICAL. PROVIDE EQUIVALENT OF HEAVY BROOM FINISH AT SLOPES 6% AND GREATER. REFER TO SPECIFICATIONS.
- 2. ALL NEW PEDESTRIAN WALKWAYS (NON-RAMP) SHALL BE SLOPED NO GREATER THAN 2.0%, AND NO LESS THAN 0.75% IN ANY DIRECTION, UNLESS SPECIFICALLY LABELED OTHERWISE. ALL CONCRETE SHALL MEET THE FOLLOWING SLOPE REQUIREMENTS:
- NO GREATER THAN 5% SLOPE IN THE DIRECTION OF TRAVEL. - NO GREATER THAN 2% SLOPE CROSSING THE DIRECTION OF TRAVEL.
- NO GREATER THAN 2% SLOPE IN ANY DIRECTION IN COURTYARD OR PLAZA AREAS.

### CIVIL SHEET INDEX

- CO.1 CIVIL GENERAL NOTES AND ABBREVIATIONS
- C1.1 DEMOLITION PLAN
- C2.1 GRADING AND PAVING PLAN



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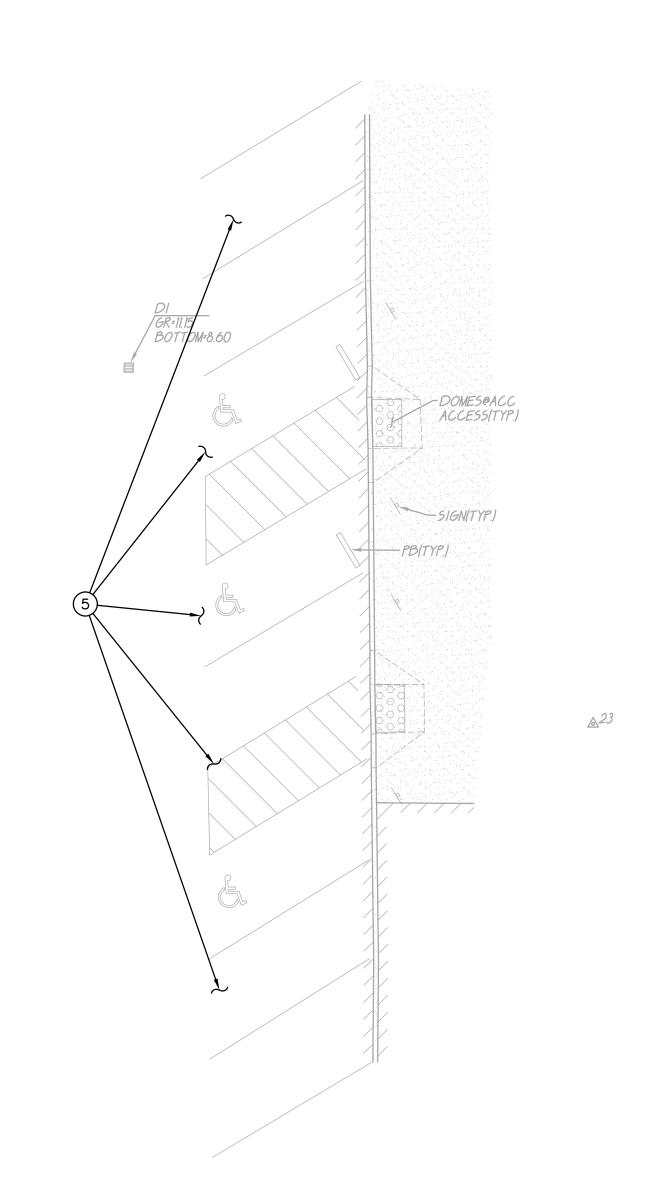
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**CIVIL GENERAL NOTES AND ABBREVIATIONS** 

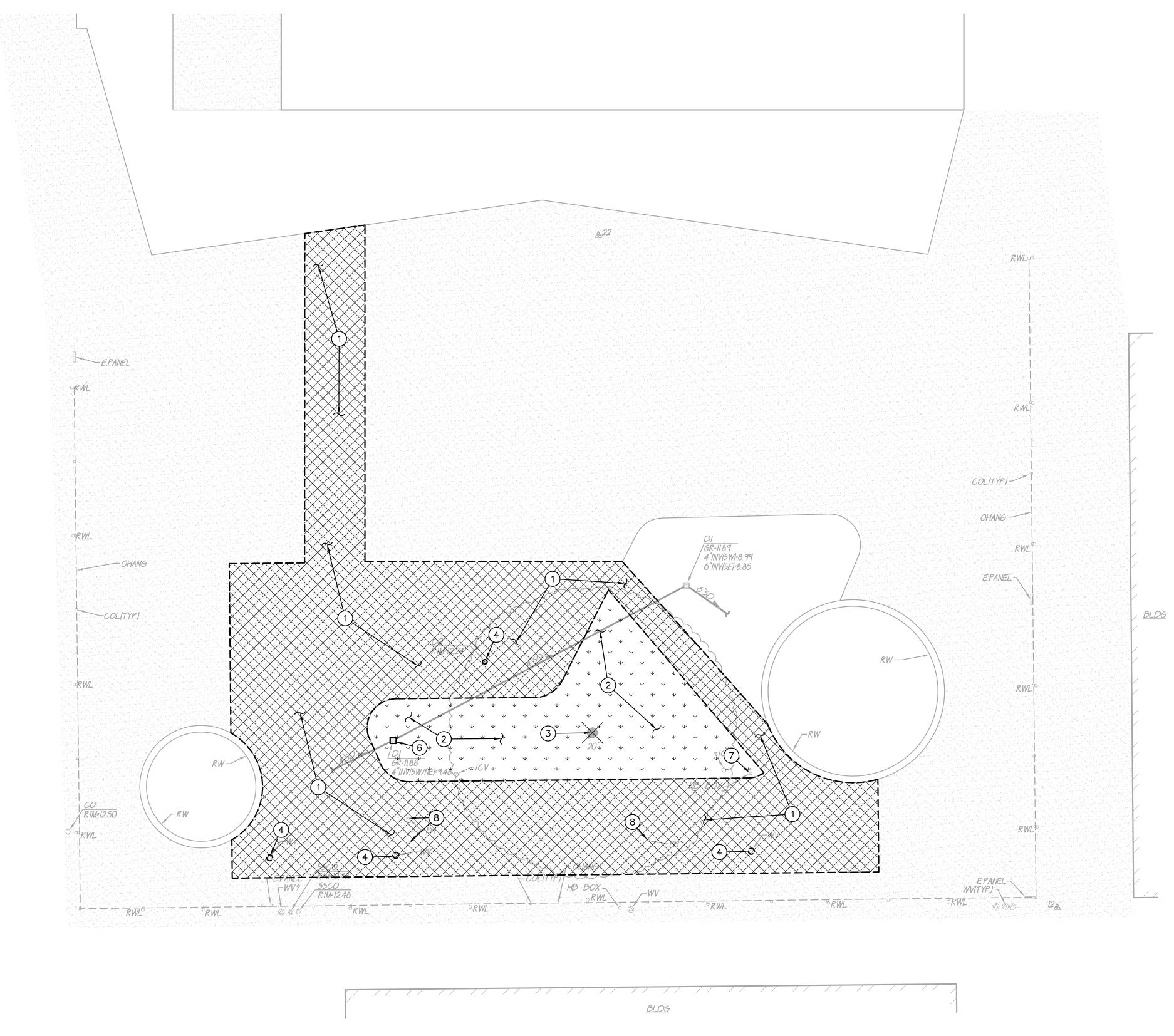
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**DEMOLITION PLAN - ACCESSIBLE PARKING** 

SCALE: 1"=10'



DEMOLITION PLAN - SHADE STRUCTURE

DEMOLITION NOTES

SAWCUT, REMOVE AND DISPOSE OF EXISTING CONCRETE PAVING AND ASSOCIATED AGGREGATE BASE. SAWCUT SHALL BE A NEAT STRAIGHT LINE, MAINTAIN CLEAN, STRAIGHT CUT EDGE UNTIL NEW PAVING IS PLACED.

REMOVE AND DISPOSE OF EXISTING LANDSCAPING, TURF AND ASSOCIATED IRRIGATION PIPING/SPRINKLERS WITHIN AREAS OF WORK. CUT AND CAP ANY MAINLINES NEAR WHERE THEY ENTER THE BOUNDARY OF THE PROJECT. MARK ALL CAPPED LINES WITH AN IRRIGATION VALVE BOX. ALL EXISTING IRRIGATION AREAS OUTSIDE THE PROJECT WORK AREA SHALL BE PRESERVED AND OPERATIONAL. INTEGRITY SHALL BE MAINTAINED WITH PROPER SPRINKLER COVERAGE TO TURF AREAS TO REMAIN.

3. REMOVE AND DISPOSE OF EXISTING TREE, TRUNK AND ASSOCIATED ROOTS.

4. REMOVE EXISTING UTILITY BOX AND/OR FRAME AND COVER AND PROVIDE NEW. NEW BOX SHALL BE SIMILAR IN SIZE, BUT WITH TRAFFIC RATING AND SLIP RESISTANT COVER.

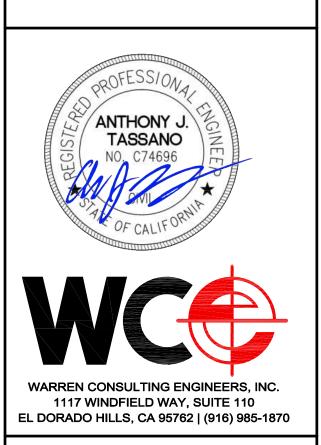
5. BLACK OUT EXISTING STRIPING.

6. REMOVE AND DISPOSE OF EXISTING DROP INLET

7. REMOVE AND DISPOSE OF EXISTING HOSE BIBB.

8. REMOVE AND DISPOSE OF EXISTING POST HOLE AND ASSOCIATED FOOTING.





ADE STRUCTURE AT ROSA PAR DLE SCHOOL

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DEMOLITION PLAN

GRAPHIC SCALE

10' 0 5' 10' 20'

(IN FEET) I inch = 10 feet

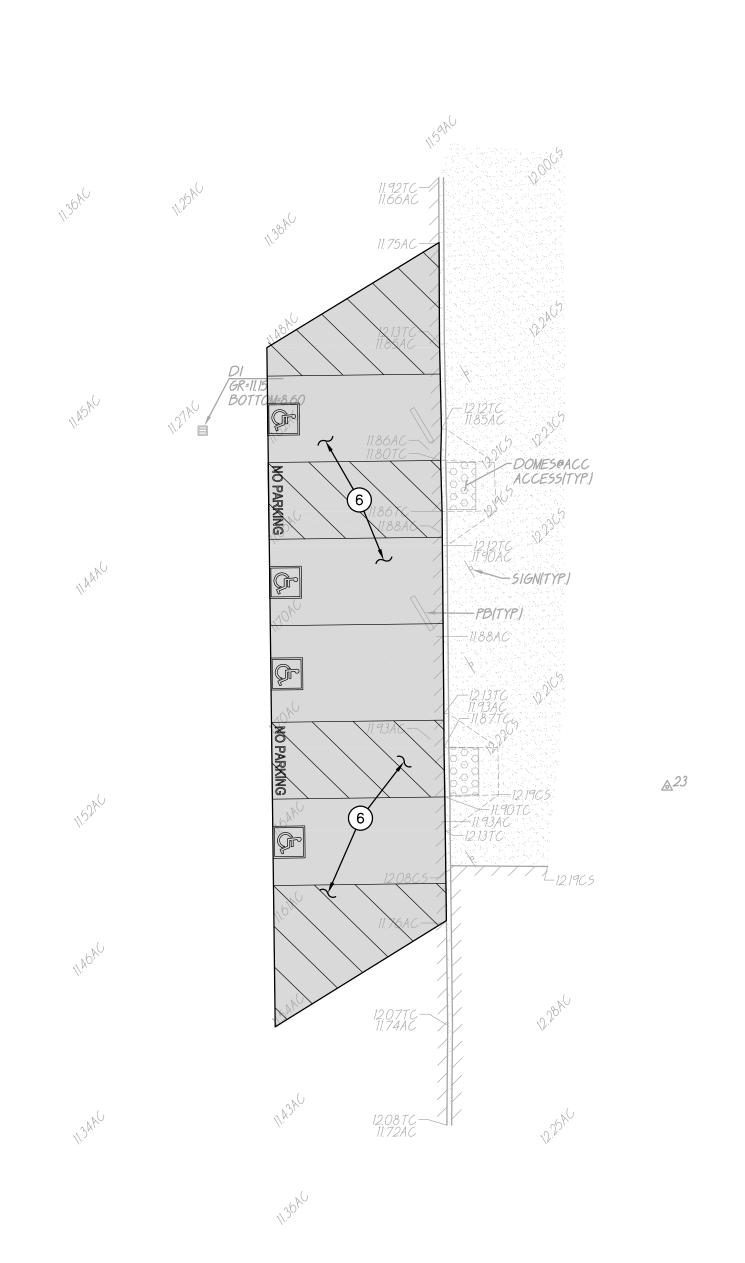
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DATE: 3/21/2022

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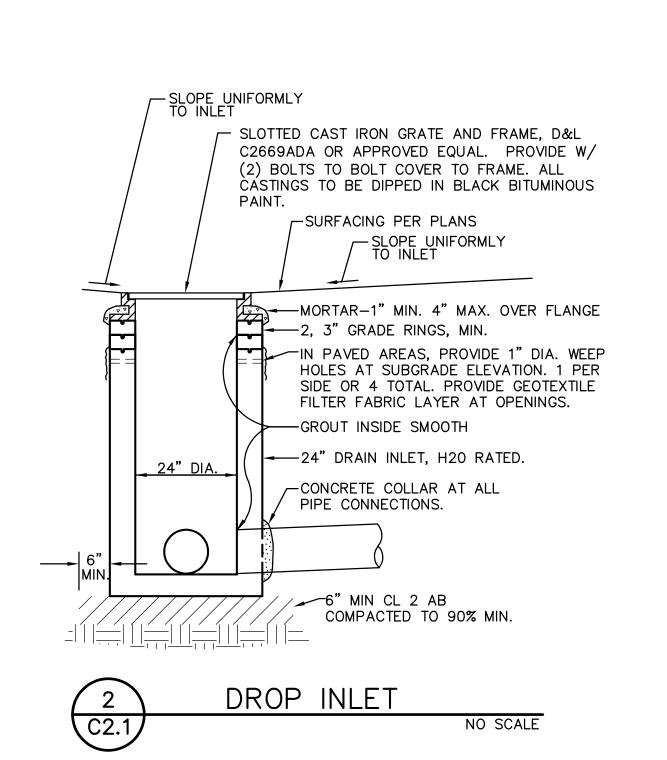
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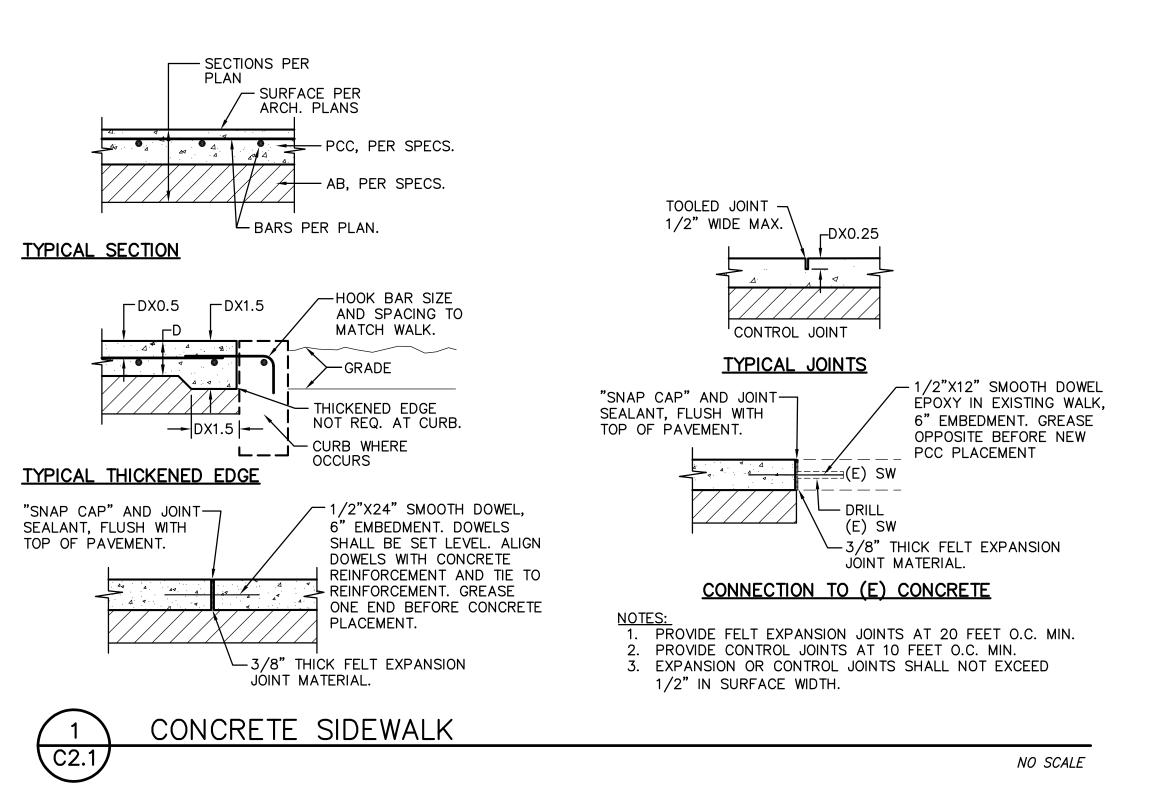


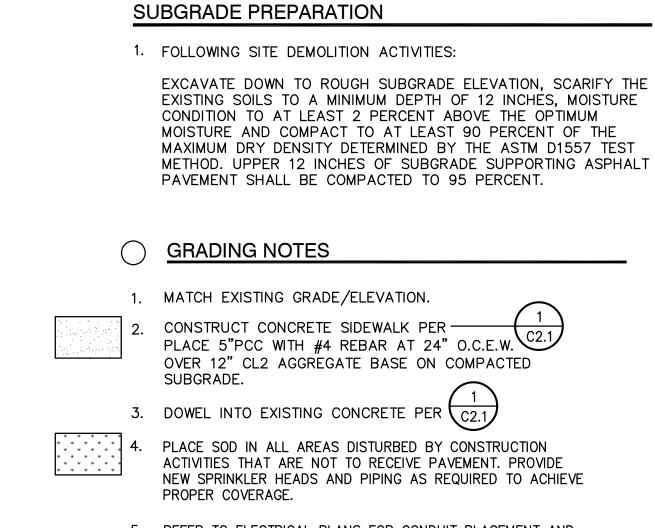
12.35±TW  $O(10^{100})$  GR=12.10 -13.55TRW 12.36C5 12.45±TW 1 BLDG

GRADING AND PAVING PLAN - ACCESSIBLE PARKING

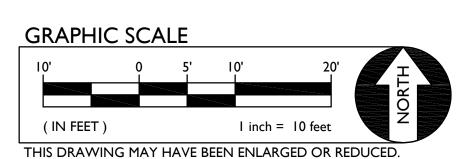
GRADING AND PAVING PLAN - SHADE STRUCTURE







5. REFER TO ELECTRICAL PLANS FOR CONDUIT PLACEMENT AND CRACK FILL AND PLACE TWO (2) APPLICATIONS OF SEAL COAT PRIOR TO STRIPING. 7. CONSTRUCT DROP INLET PER —— CONNECT INLET TO EXITING STORM DRAIN PIPE. C2.1



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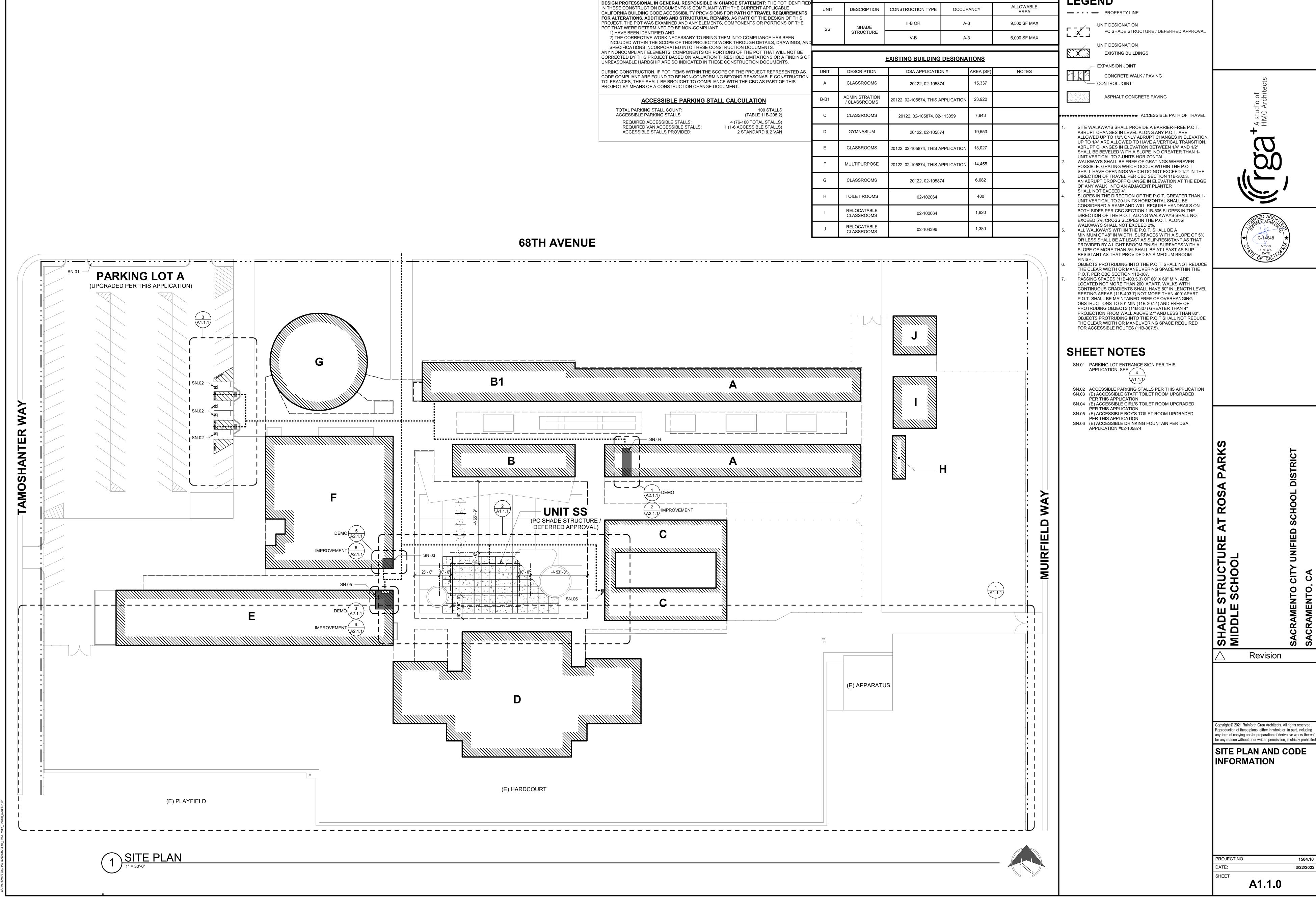
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> **GRADING AND PAVING PLAN**

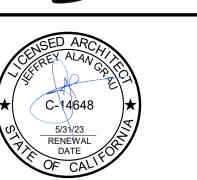
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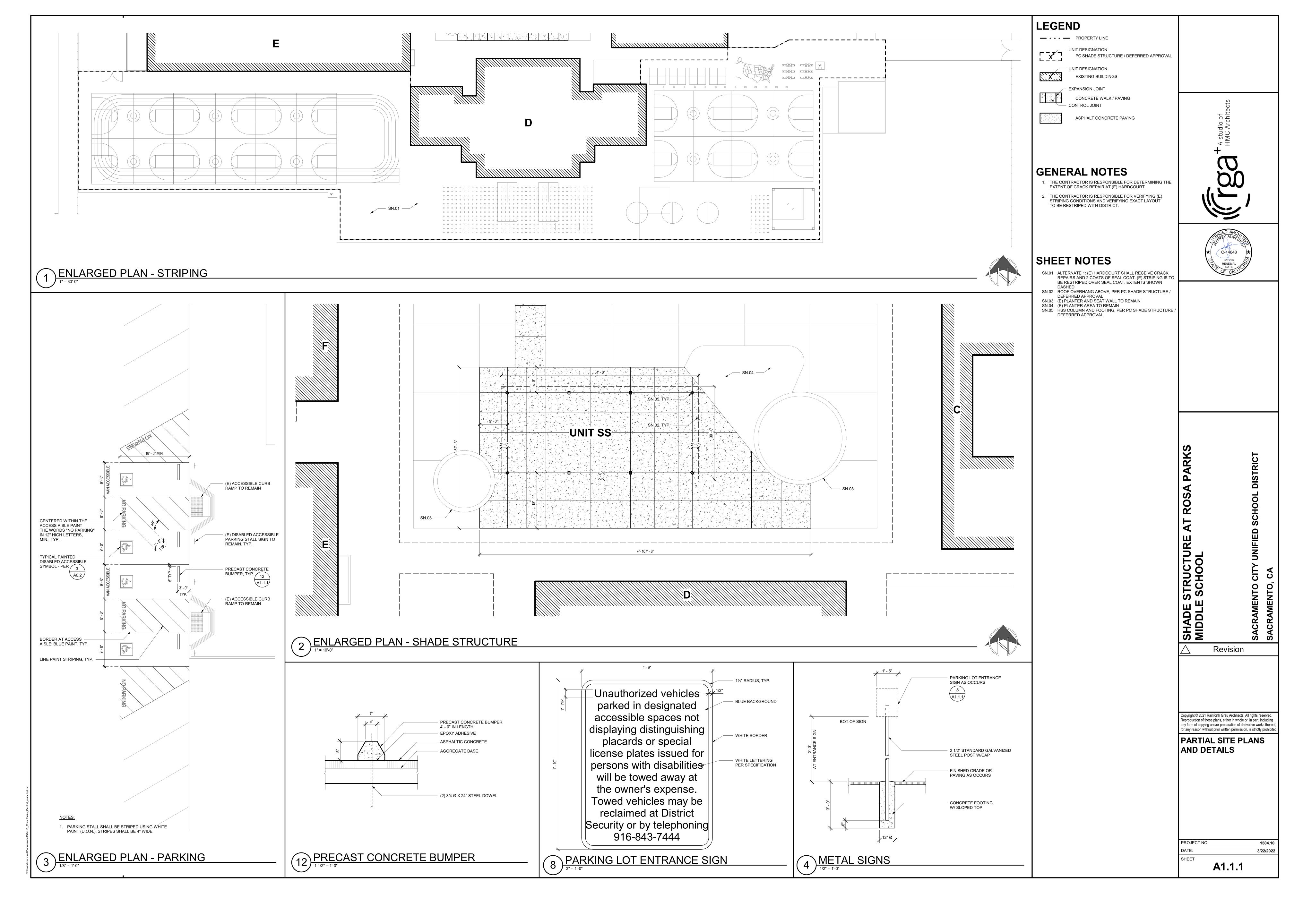


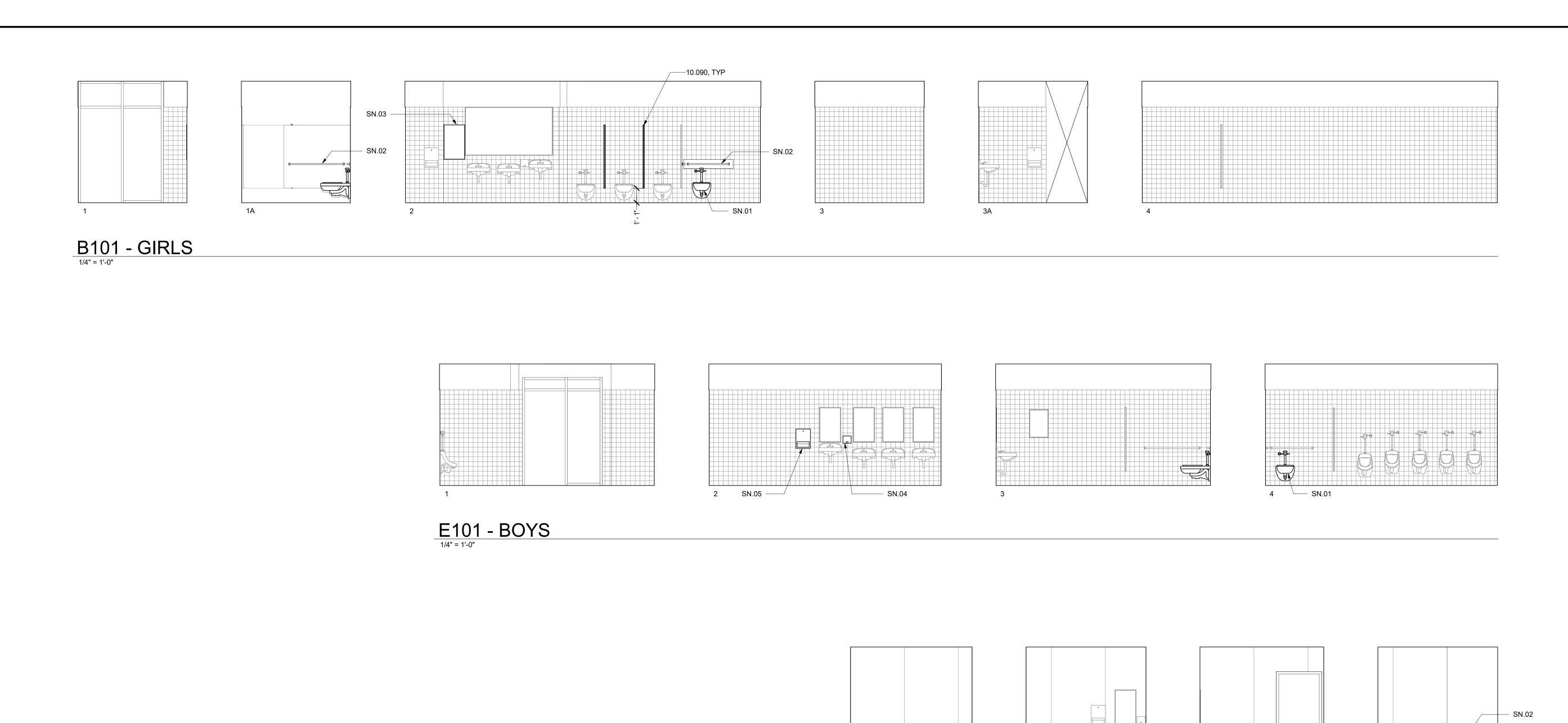
**EXISTING PATH OF TRAVEL (POT): ARCHITECT STATEMENT** 

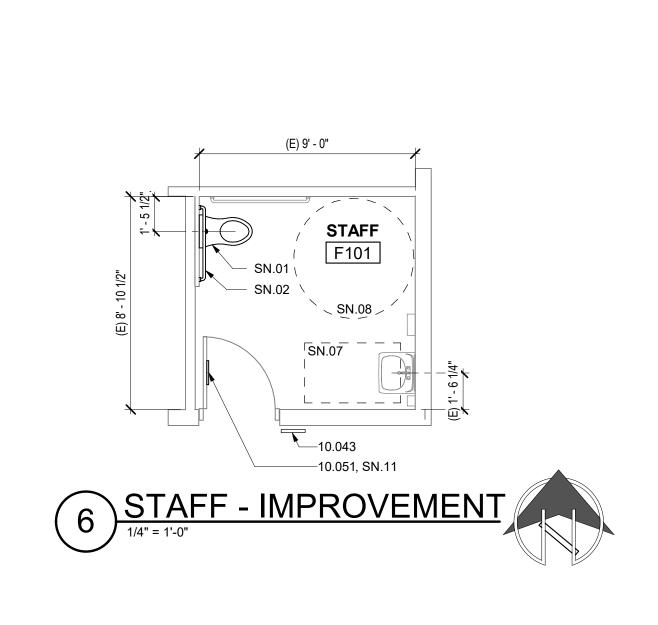
**LEGEND** 

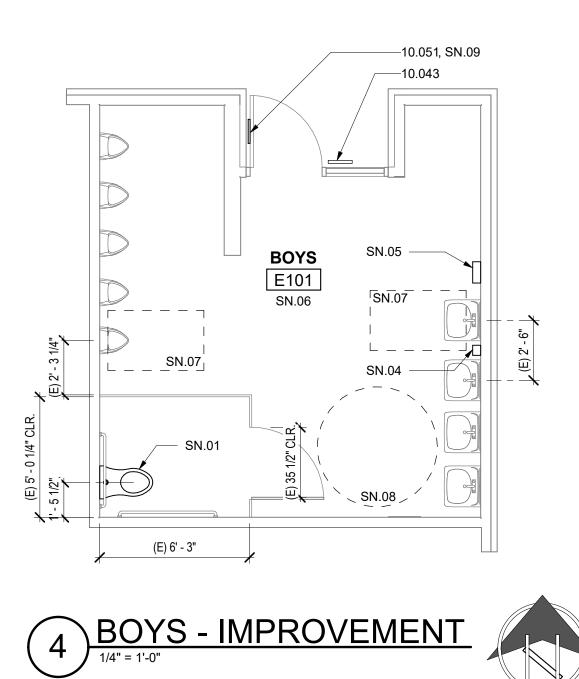
PROPOSED SHADE STRUCTURE

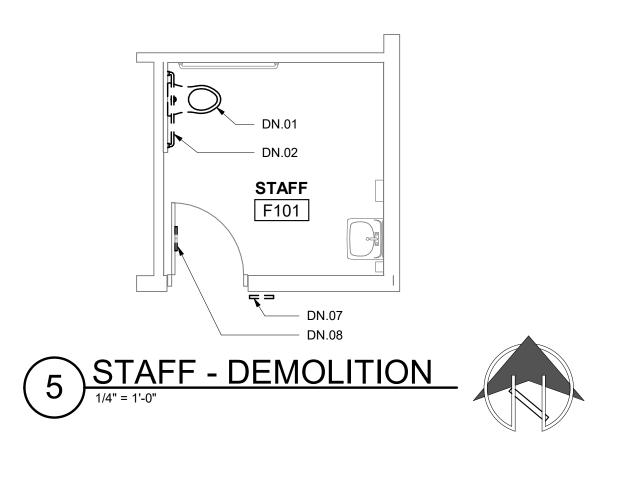


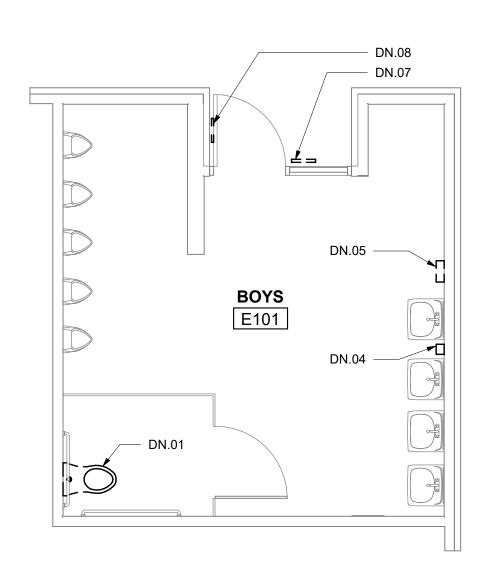




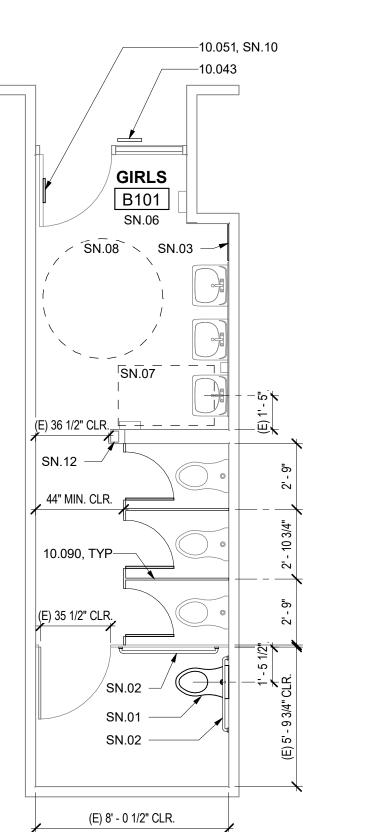






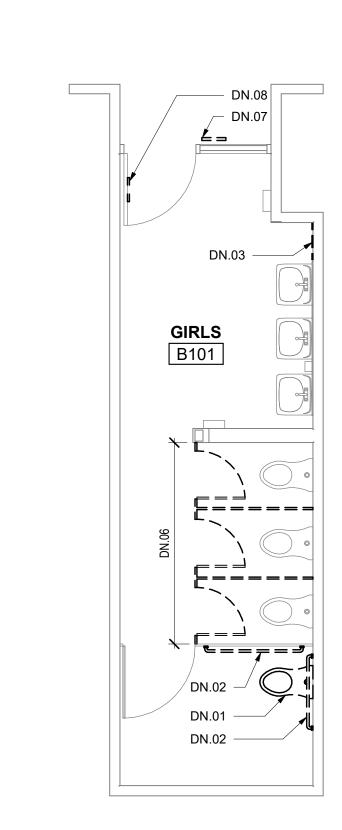






F101 - STAFF







### **LEGEND**

INTERIOR ELEV.

CONSECUTIVE NUMBERING CONVENTION FOR INTERIOR **ELEVATIONS AND ROOM** FINISHES.

### **GENERAL NOTES**

CONTRACTOR UNDER THIS CONTRACT.

- FOR MOUNTING HEIGHTS, LOCATIONS, AND DETAILS, INCLUDING THOSE FOR DISABLED ACCESSIBITY, REFER TO SHEET A0.2 PROTECT ALL ADJACENT SURFACES, ITEMS AND FINISHES NOT
- NOTED TO BE DEMOLISHED. EQUIPMENT/FIXTURES NOTED AS "SALVAGED FOR REINSTALLATION" WILL BE REMOVED AND STORED BY THE CONTRACTOR PRIOR TO START OF DEMOLITION. THESE
- REMOVE ALL ITEMS SCHEDULED TO BE REMOVED, INCLUDING MOUNTING HARDWARE.

EQUIPMENT/FIXTURES SHALL BE REINSTALLED BY THE

DEMO AND REPAIR WALL FINISH AS NECESSARY TO PERFORM FIXTURE AND EQUIPMENT WORK AS NOTED. ADJACENT FINISHES TO BE VERIFIED BY CONTRACTOR.

### **DEMOLITION NOTES**

- DN.01 REMOVE (E) WALL-MOUNTED WATER CLOSET AND SALVAGE FOR REINSTALLATION DN.02 REMOVE (E) GRAB BARS AND SALVAGE FOR REINSTALLATION
- DN.03 REMOVE (E) MIRROR AND SALVAGE FOR REINSTALLATION DN.04 REMOVE (E) SOAP DISPENSER AND SALVAGE FOR
- REINSTALLATION DN.05 REMOVE (E) PAPER TOWEL DISPENSER AND SALVAGE
- FOR REINSTALLATION DN.06 REMOVE (E) TOILET PARTITION AND TOILET PARTITION DOOR
- DN.07 REMOVE (E) TOILET ROOM I.D. SIGN

### DN.08 REMOVE (E) TOILET ROOM DOOR SYMBOL

### SHEET NOTES

- SN.01 REINSTALL (E) SALVAGED WALL-MOUNTED WATER CLOSET TO COMPLY WITH A0.2. ADJUST (E) WATER CARRIER AS REQUIRED FOR RECONNECTION TO WATER CLOSET.
- RECONNECT TO (E) WATER LINE, WASTE LINE AND VENT. SN.02 REINSTALL (E) SALVAGED GRAB BARS TO COMPLY WITH A0.2 SN.03 REINSTALL (E) SALVAGED MIRROR TO COMPLY WITH A0.2
- SN.04 REINSTALL (E) SALVAGED SOAP DISPENSER TO COMPLY WITH A0.2
- SN.05 REINSTALL (E) SALVAGED PAPER TOWEL DISPENSER TO COMPLY WITH A0.2
- SN.06 WRAP ALL EXPOSED PIPES WITH INSULATION AT LAVATORIES SN.07 30" X 48" CLEAR SPACE
- SN.08 60" DIA. TURNING CIRCLE
- SN.09 SIGN TO READ "BOYS"
  SN.10 SIGN TO READ "GIRLS"
  SN.11 SIGN TO READ "STAFF"
  SN.12 (E) STRUCTURAL COLUMN

### KEYNOTES

10.043 SIGNAGE: TOILET ROOM IDENTIFICATION 10.051 SIGNAGE: TOILET ROOM DOOR SYMBOL 10.090 COMPOSITE TOILET COMPARTMENT

Revision

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TOILET ROOM **DEMOLITION AND** IMPROVEMENT PLANS AND INTERIOR ELEVATIONS

UNITS A, E & F

A2.1.1

### ABBREVIATION LIST AMPERE ALTERNATING CURRENT AIR CONDITIONING ARC ENERGY REDUCTION AMP FRAME ABOVE FINISHED FLOOR AMPERES INTERRUPTING CAPACITY AMP TRIP SETTING AMERICAN WIRE GAUGE BARE COPPER BELOW FINISHED CEILING BREAKER BUILDING **BOOSTER POWER SUPPLY** CONDUIT CIRCUIT BREAKER CONTRACTOR FURNISHED. CONTRACTOR INSTALLED CIRCUIT CEILING CONDUIT ONLY, WITH PULL LINE CONT CONTINUOUS METALLIC COLD WATER PIPE DEMOLISH DIRECT CURRENT DISCONNECT DISTRIBUTION PANEL EXISTING EACH WITH **EVENING LIGHT** ELECTRIC EMERGENCY ELECTRICAL METALLIC TUBING END OF LINE DEVICE **EQUIPMENT** EXISTING RELOCATED ELECTRICAL WATER COOLER ELECTRIC WATER HEATER FIRE ALARM CONTROL PANEL FAEP FIRE ALARM EXTENDER PANEL FATC FIRE ALARM TERMINAL CABINET FURNISHED BY OTHERS **FLUOR** FLUORESCENT GROUND FAULT CIRCUIT INTERRUPT GENERAL LIGHTING ZONE METALLIC GAS PIPE GYPSUM HIGH INTENSITY DISCHARGE HORSE POWER HEIGHT HERTZ INTERMEDIATE METALLIC CONDUIT SHORT CIRCUIT CURRENT (RMS SYMMETRICAL) ISOLATED J-B0X JUNCTION BOX THOUSAND CIRCULAR MILLS KCMIL KILO VOLT AMP KILOWATT LIGHTING CONTROL PANEL LOW VOLTAGE THOUSAND CIRCULAR MILLS MECHANICAL MAIN DISTRIBUTION PANEL METAL HALIDE MISCELLANEOUS MAIN LUGS ONLY MAIN POINT OF ENTRY MAIN SWITCHBOARD NOT IN CONTRACT NOT IN ELECTRICAL SECTION OF THESE PLANS & SPECS. NIGHT LIGHT NUMBER NOT TO SCALE ON CENTER OFCI OWNER FURNISHED, CONTRTRACTOR INSTALLED OFOI OWNER FURNISHED, OWNER INSTALLED PULL BOX PROVISION FOR FUTURE BREAKER W/ PFB MOUNTING HARDWARE PRIMARY DAYLIT ZONE PROVISION FOR FUTURE CURRENT TRANSFORMER PHASE PLYWOOD PLYWD PANEL PNLPAIR POLYVINYL CHLORIDE CONDUIT RELOCATE / RELOCATED REQUIRED ROOM RIGID METAL CONDUIT REMOVE AND REPLACE SECONDARY DAYLIT ZONE SKYLIGHT DAYLIT ZONE SPEC SPECIFICATION SIGNAL TERMINAL CABINET SQUARE SWITCH TELEPHONE TELECOMMUNICATIONS GROUNDING TELECOMMUNICATIONS MAIN GROUNDING BUSBAR TELEPHONE TERMINAL BOARD TYPICAL UNDERGROUND UNLESS OTHERWISE NOTED UON VOLTS WEATHERPROOF WEIGHT WATT TRANSFORMER

### **GENERAL NOTES**

- 1. PLANS ARE NOT FOR CONSTRUCTION UNTIL APPROVED BY THE AUTHORITY HAVING JURISDICTION. THE CONTRACTOR SHALL NOT ORDER ANY MATERIALS OR INSTALL ANY EQUIPMENT, PIPING, ETC. UNTIL PLANS ARE APPROVED BY THE AUTHORITY HAVING JURISDICTION.
- 2. ALL WORK SHALL BE DONE AT SUCH TIME AND IN SUCH MANNER AS PRESCRIBED BY THE SCHOOL'S REPRESENTATIVE.
- 3. PROTECT EXISTING EQUIPMENT AND FURNISHINGS FROM ANY DAMAGE DUE TO DUST, MOISTURE OR CONTACT WITH WORK CREW OR MATERIALS.
- 4. THE SCHOOL SHALL BE NOTIFIED AT LEAST FORTY-EIGHT (48) HOURS IN ADVANCE OF ANY POWER SHUTDOWN OF EXISTING PANELS OR SERVICE. SCHEDULE OF SHUTDOWNS SHALL BE AT CONVENIENCE OF THE SCHOOL. THE SCHOOL MAY, AT THEIR OPTION, HAVE A REPRESENTATIVE PRESENT DURING SHUTDOWN. ALL WORK REQUIRING SHUTDOWNS OF EXISTING PANELS OR SERVICE SHALL BE DONE BETWEEN 12:00 AM MIDNIGHT AND 6:00AM WEEKDAYS OR ON SATURDAY AND SUNDAY. REQUIRED SHUTDOWNS SHALL BE KEPT TO A MINIMUM.
- 5. ADEQUATELY STRAP AND SUPPORT ALL CONDUIT WORK PER CEC. IN GENERAL, SUPPORT ALL CONDUIT WITHIN THREE FEET (3') OF OUTLET BOX, CABINET OR PANEL AND MAXIMUM TEN FEET (10') ON CENTER THEREAFTER.
- 6. CORE BORE SHALL BE 1" DIAMETER LARGER THAN EACH CONDUIT. SPACE CONDUIT HOLES 3" APART. SEAL AROUND CONDUIT WITH NON-SHRINK, NON-METALLIC GROUT.
- 7. ALL CONDUCTORS INSTALLED IN PANELBOARDS SHALL BE TRAINED, LACED, AND INSTALLED WITH PHASE TAPE ON ALL CONDUCTORS.
- 8. LABEL DEVICES (I.E. RECEPTACLES, ETC.) ON EACH COVER PLATE IDENTIFYING CIRCUIT AND PANEL DEVICE IS CONNECTED TO.
- 9. CLEAN ALL EXTERIOR AND INTERIOR SURFACES OF PANELS AND ALL MATERIAL AND METAL SHAVINGS FROM PANEL AND CABINET INTERIORS. ALL OPENINGS SHALL BE SEALED AND APPLY TOUCH-UP SPRAY PAINT WHERE NEEDED.
- 10. FIELD COORDINATE DEVICE LOCATIONS PRIOR TO ROUGH-IN.
- 11. CONTRACTOR WILL PROVIDE WARNING LABELS NOTING THE POTENTIAL FOR ELECTRIC ARC FLASH HAZARDS PER CEC 110.16. PROVIDE LABELS ON EQUIPMENT SUCH AS SWITCHBOARDS, SWITCHGEAR, PANELBOARDS, INDUSTRIAL CONTROL PANELS, METER SOCKET ENCLOSURES, MOTOR CONTROL CENTERS, MOTOR STARTER / CONTACTOR PANELS, DISCONNECTS, ETC.. PROVIDE WARNING LABELS BY BRADY, MODEL NO. 101517, OR EQUAL, ON ALL
- 12. INSTALLATION SHALL COMPLY WITH CEC 210.4 EACH MULTIWIRE BRANCH CIRCUIT SHALL BE PROVIDED WITH A MEANS THAT WILL SIMULTANEOUSLY DISCONNECT ALL UNGROUNDED CONDUCTORS AT THE POINT WHERE THE BRANCH CIRCUIT ORIGINATES. THEREFORE ANY CIRCUIT SHARING A COMMON NEUTRAL SHALL BE CAPABLE OF SIMULTANEOUS DISCONNECT OR DEDICATED NEUTRALS SHALL BE INSTALLED.
- 13. SUPPORT ENCLOSURES, BOXES AND CONDUIT INSTALLATIONS PER CEC 314.23 (A) THROUGH (H).
- 14. SEAL CONDUIT OPENINGS THROUGH WALLS AND CEILINGS. INSTALL ESCUTCHEON PLATES AT BUILDING INTERIOR. WHERE EQUIPMENT IS INSTALLED ON THE EXTERIOR WALL, STUB CONDUITS THROUGH WALL AND SEAL CONDUIT OPENINGS, THEN INSTALL EXTERIOR EQUIPMENT. ALSO, SEAL AROUND THE PERIMETER EDGE OF THE EQUIPMENT ENCLOSURE BETWEEN THE ENCLOSURE AND BUILDING.
- 15. CONDUITS INSTALLED ON ROOF AND BUILDING EXTERIOR SHALL BE RIGID GALV. STEEL (HEAVY WALL) WITH THREADED FITTINGS. CONDUIT AND WALL TO BE PAINTED OUT TO MATCH EXTERIOR FINISH.
- 16. SPLICES AND TERMINALS SHALL BE COMPRESSION TYPE OF SEAMLESS PURE COPPER, TIN PLATED, LONG BARREL (TERMINALS WITH TWO-HOLE PAD AND INSPECTION WINDOW WITH NEMA DRILLING), AS MANUFACTURED BY BURNDY TYPE YS, YAZ-2N OR EQUAL. CLEAN ALL SURFACES AND INSTALL WITH OXIDE INHIBITING COMPOUND, BURNDY PENETROX-E OR EQUAL. APPLY COMPOUND BETWEEN BUS AND LUG PAD AND BETWEEN CONDUCTOR AND LUG BARREL. INSTALL COMPRESSION CONNECTORS WITH 360° CIRCUMFERENTIAL COMPRESSION DYE, BURNDY HYPRESS OR EQUAL. THE INDENTER OR OTHER TYPE TOOLS WILL NOT BE ACCEPTABLE.
- 17. INSTALL 'MECHANICALLY FASTENED PHENOLIC NAMEPLATE WITH WHITE LETTERING ON BLACK BACKGROUND ON ALL EQUIPMENT, INCLUDING PULL BOXES, WITH DESCRIPTION INDICATED ON DRAWINGS. NAMEPLATES SHALL READ EXACTLY AS DESCRIBED ON THE DRAWINGS. IN GENERAL NAMEPLATE LETTERING SIZE SHALL BE 3/16" HIGH FOR ALL NAMEPLATES SERVING FEEDER AND BRANCH CIRCUIT BREAKERS. ON MAIN SERVICE PANEL. DISTRIBUTION PANELS AND ALL OTHER NAMEPLATES LETTERING SHALL BE 1/4" HIGH.
- 17.1. ALL SWITCHBOARDS, SWITCHGEAR, PANELBOARDS, VFD'S, MOTORS, JUNCTION BOXES, PULL BOXES, DISCONNECT SWITCHES, ETC., SHALL BE MARKED TO INDICATE EACH DEVICE OR EQUIPMENT WHERE THE POWER ORIGINATES PER CEC 408.4, FIELD IDENTIFICATION REQUIRED, (B) SOURCE OF SUPPLY.
- 18. COORDINATE EQUIPMENT LOCATIONS, CONTROL AND POWER WIRING REQUIREMENTS AND CONNECT POINTS WITH ALL APPLICABLE DISCIPLINES.
- 19. PROVIDE AND INSTALL FUSES PER UNIT NAMEPLATE DATA ON THE EQUIPMENT PROVIDED.
- 20. A LAMINATED COPY OF THE FINAL RECORD ONE LINE DIAGRAM SHALL BE PLACED IN ELEC ROOM.
- 21. PROVIDE WIRING DEVICES AND COVER PLATES IN COLOR(S) SELECTED BY ARCHITECT. THE COLOR OF THE WIRING DEVICE AND COVER PLATE SHALL BE THE SAME UNLESS SPECIFICALLY NOTED OTHERWISE.
- 22. RECEPTACLE WEATHERPROOF COVERS SHALL BE LISTED "EXTRA DUTY", LOCAKBLE, METAL, IN-USE TYPE.
- 23. REINSTALL EXISTING ELECTRICAL INSTALLATIONS DISTURBED. CERTAIN EXISTING ELECTRICAL INSTALLATIONS MAY BE LOCATED IN WALLS. CEILINGS OR FLOORS THAT ARE TO BE REMOVED AND ARE ESSENTIAL FOR THE OPERATION OF OTHER REMAINING INSTALLATIONS. WHERE THIS CONDITIONS OCCURS, PROVIDE A NEW EXTENSION OF ORIGINAL CIRCUITS, RACEWAYS, EQUIPMENT AND OUTLETS TO RETAIN SERVICE CONTINUITY. INSTALLATIONS SHALL BE CONCEALED IN FINISHED AREAS.
- 24. FOR ROOF PENETRATIONS, REFER TO ARCHITECTURAL PLANS FOR INSTALLATION REQUIREMENTS.
- 25. FOR WALL PENETRATION INSTALLATIONS, REFER TO ARCHITECTURAL PLANS FOR REQUIREMENTS.
- 26. PROVIDE "LOCK-ON" DEVICE FOR ALL CIRCUIT BREAKERS ON EMERGENCY DEDICATED CIRCUITS.
- 27. DRAWINGS ARE TO BE CONSIDERED DIAGRAMMATIC. CONTRACTOR SHALL ACCEPT RESPONSIBILITY IN FAMILIARIZING THEMSELVES WITH ARCHITECTURAL AND STRUCTURAL CONDITIONS ALONG WITH INHERENT SPACE LIMITATIONS. WITH THAT UNDERSTANDING SHALL PROVIDE ALL ITEMS OF LABOR, MATERIALS AND TOOLS REQUIRED TO PROVIDE A COMPLETE INSTALLATION.
- 28. MAINTAIN A MINIMUM OF 12" SEPARATION BETWEEN ANY CONDUIT AND (E) UTILITY CONDUIT.
- 29. FOR INTERSECTING TRENCHED CONDUIT, MAINTAIN OR EXCEED THE MINIMUM CONDUIT DEPTH REQUIREMENTS.

### MEP COMPONENT ANCHORAGE NOTE

ALL MECHANICAL, PLUMBING AND ELECTRICAL COMPONENTS SHALL BE ANCHORED AND INSTALLED PER THE DETAILS ON THE DSA APPROVED CONSTRUCTION DOCUMENTS. THE FOLLOWING COMPONENTS SHALL BE ANCHORED AND BRACED TO MEET THE FORCE AND DISPLACEMENT REQUIREMENTS PRESCRIBED IN THE 2019 CBC SECTIONS 1617A.1.18 THROUGH 1617A.1.26 AND ASCE 7-16 CHAPTERS 13, 26 AND 30:

- ALL PERMANENT EQUIPMENT AND COMPONENTS. TEMPORARY, MOVEABLE OR MOBILE EQUIPMENT THAT IS PERMANENTLY ATTACHED (E.G. HARD WIRED) TO THE BUILDING UTILITY SERVICES SUCH AS ELECTRICITY, GAS OR WATER. "PERMANENTLY ATTACHED" SHALL INCLUDE ALL ELECTRICAL CONNECTIONS EXCEPT PLUGS FOR 110/20 VOLT RECEPTACLES HAVING A FLEXIBLE CABLE.
- 3. TEMPORARY, MOVEABLE OR MOBILE EQUIPMENT WHICH IS HEAVIER THAN 400 POUNDS OR HAS A CENTER OF MASS LOCATED 4 FEET OR MORE ABOVE THE ADJACENT FLOOR OR ROOF LEVEL THAT DIRECTLY SUPPORTS THE COMPONENT IS REQUIRED TO BE RESTRAINED IN A MANNER APPROVED BY DSA.
- THE FOLLOWING MECHANICAL AND ELECTRICAL COMPONENTS SHALL BE POSITIVELY ATTACHED TO THE STRUCTURE, BUT NEED NOT DEMONSTRATE DESIGN COMPLIANCE WITH THE REFERENCES NOTED ABOVE. THESE COMPONENTS SHALL HAVE FLEXIBLE CONNECTIONS PROVIDED BETWEEN THE COMPONENT AND ASSOCIATED DUCTWORK, PIPING, AND CONDUIT. FLEXIBLE CONNECTIONS MUST ALLOW MOVEMENT IN BOTH TRANSVERSE AND LONGITUDINAL DIRECTIONS:
- A. COMPONENTS WEIGHING LESS THAN 400 POUNDS AND HAVING A CENTER OF MASS LOCATED 4 FEET OR LESS ABOVE THE ADJACENT FLOOR OR ROOF LEVEL THAT DIRECTLY SUPPORTS THE COMPONENT.
- B. COMPONENTS WEIGHING LESS THAN 20 POUNDS, OR IN THE CASE OF DISTRIBUTED SYSTEMS, LESS THAN 5 POUNDS PER FOOT, WHICH ARE SUSPENDED FROM A ROOF OR FLOOR OR HUNG FROM A WALL.
- THE ANCHORAGE OF ALL MECHANICAL, ELECTRICAL AND PLUMBING COMPONENTS SHALL BE SUBJECT TO THE APPROVAL OF THE DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE CHARGE OR STRUCTURAL ENGINEER DELEGATED RESPONSIBILITY AND ACCEPTANCE BY DSA. THE PROJECT INSPECTOR WILL VERIFY THAT ALL COMPONENTS AND

EQUIPMENT HAVE BEEN ANCHORED IN ACCORDANCE WITH THE ABOVE REQUIREMENTS.

### PIPING, DUCTWORK, AND ELECTRICAL DISTRIBUTION SYSTEM BRACING NOTE

PIPING, DUCTWORK, AND ELECTRICAL DISTRIBUTION SYSTEMS SHALL BE BRACED TO COMPLY WITH THE FORCES AND DISPLACEMENTS PRESCRIBED IN ASCE 7-16 SECTION 13.3 AS DEFINED IN ASCE 7-16 SECTIONS 13.6.5, 13.6.6, 13.6.7, 13.6.8 AND 2019 CBC, SECTIONS 1617A.1.24, 1617A.1.25 AND 1617A.1.26.

THE METHOD OF SHOWING BRACING AND ATTACHMENTS TO THE STRUCTURE FOR THE IDENTIFIED DISTRIBUTION SYSTEM ARE AS NOTED BELOW. WHEN BRACING AND ATTACHMENTS ARE BASED ON A PREAPPROVED INSTALLATION GUIDE (E.G., OSHPD OPM FOR 2013 CBC OR LATER), COPIES OF THE BRACING SYSTEM INSTALLATION GUIDE OR MANUAL SHALL BE AVAILABLE ON THE JOBSITE PRIOR TO THE START OF AND DURING THE HANGING AND BRACING OF THE DISTRIBUTION SYSTEMS. THE STRUCTURAL ENGINEER OF RECORD SHALL VERIFY THE ADEQUACY OF THE STRUCTURE TO SUPPORT THE HANGER AND BRACE LOADS.

MECHANICAL PIPING (MP), MECHANICAL DUCTS (MD), PLUMBING PIPING (PP), ELECTRICAL DISTRIBUTION SYSTEMS (E): MP ☐ MD ☐ PP ☐ E ■ OPTION 1: DETAILED ON THE APPROVED DRAWINGS WITH PROJECT SPECIFIC NOTES AND DETAILS.

MP ☐ MD ☐ PP ☐ E ☐ OPTION 2: SHALL COMPLY WITH THE APPLICABLE OSHPD PRE-APPROVAL (OPM #)

### SYMBOLS LIST

- F' FUSED DISCONNECT SWITCH
- ➡ DUPLEX CONVENIENCE OUTLET
- DOUBLE DUPLEX CONVENIENCE OUTLET GROUND FAULT CIRCUIT INTERRUPTER DUPLEX OUTLET
- GROUND FAULT CIRCUIT INTERRUPTER DOUBLE DUPLEX OUTLET
- SPECIAL OUTLET TO MATCH CAP PROVIDED WITH MACHINE
- FLUSH FLOOR BOX OR "POKE-THRU" UNIT EQUIPPED WITH FLUSH OR PEDESTAL DUPLEX RECEPTACLE AND VOICE/DATA OUTLETS
- AS NOTED, OR REFER TO SCHEDULE ON DRAWINGS.
- PLUGMOLD/WIREMOLD RECEPTACLE SYSTEM
- △ TRANSFORMER
- JUNCTION BOX, SIZE AS REQUIRED BY CODE
- FLEX CONNECTION TO FIXTURE
- PANELBOARD, RECESSED MOUNTED
- PANELBOARD, SURFACE MOUNTED MAIN SWITCHBOARD
- TERMINAL CABINET, RECESSED MOUNTED
- ☐ TERMINAL CABINET, SURFACE MOUNTED → HOMERUN TO PANELBOARD OR RESPECTIVE TERMINAL
- III CONDUIT RUN CONCEALED IN CEILING OR WALL, SEE SYMBOLS LIST NOTES
- ---- CONDUIT RUN UNDERGROUND OR UNDER FLOOR
- —EM— EMERGENCY SYSTEM CONDUIT AND WIRES
- INSULATED GREEN GROUND CONDUCTOR ——>>— INSULATED ISOLATED GROUND CONDUCTOR, GREEN WITH TRACER STRIPE
- -----O CONDUIT RISER — - — EXISTING EQUIPMENT, LIGHTING, DEVICES, CONDUIT, WIRING, ETC., ARE SHOWN
- LIGHT. NEW OR RELOCATED EQUIPMENT, LIGHTING, DEVICES, CONDUIT, WIRING, ETC., ARE SHOWN DARK. X X EXISTING ELECTRICAL EQUIPMENT TO BE REMOVED
- WIREMOLD SURFACE RACEWAY(S) WITH OUTLETS AS SHOWN OR NOTED,
- SEE SURFACE RACEWAY SCHEDULE (1) 1> SYMBOLS REFERRING TO KEYED NOTES ON SAME SHEET
- MECHANICAL EQUIPMENT BY OTHERS, CONNECTED BY ELECTRICAL CONTRACTOR
- DETAIL DESIGNATION, "A" SIGNIFIES DETAIL, "E-1" SIGNIFIES SHEET NUMBER

(1)1-1/2"C  $\leftarrow$  INDICATES SIZE OF CONDUIT = ONE AND ONE HALF INCH CONDUIT — NUMBER WITHIN PARENTHESIS INDICATES QUANTITY OF CONDUITS

### SYMBOLS LIST NOTES:

- 1. MOUNT SWITCH BOXES AT +48" TO TOP OF BOX UNLESS OTHERWISE NOTED.
- 2. MOUNT OUTLET BOXES AT +15" TO BOTTOM OF BOX UNLESS OTHERWISE NOTED.
- 3. "A" ADJACENT TO OUTLET INDICATES OUTLET BOX TO BE MOUNTED ABOVE COUNTER. COORDINATE WITH COUNTER HEIGHT AND DEPTH PRIOR TO ROUGH IN. MOUNT OUTLET ABOVE COUNTERS AT: 3.1. +46" MAX TO TOP OF BOX WHERE BOX IS INSTALLED OVER BASE CABINET.

3.2. +44" MAX TO TOP OF BOX WITH OPEN COUNTERS WITH FORWARD APPROACH.

- 4. OUTLET BOXES SHALL BE:
- 4.1. WALL MOUNTED -4" SQ.  $\times 2-1/8$ " DEEP MINIMUM 4.2. CEILING MOUNTED -4" SQ. OR 4" OCT.  $\times 2-1/8$ " DEEP MINIMUM
- 5. OUTLET BOXES REQUIRING 1-1/4", 1-1/2" OR 2" CONDUITS SHALL BE 4-11/16" x 3-1/4" DEEP MINIMUM.
- 6. FLUSH MOUNTED OUTLET BOXES SHALL UTILIZE TRIM RINGS. COORDINATE TRIM RING DEPTH WITH WALL FINISH PRIOR TO ROUGH-IN.
- 7. NO CROSSBARS ON CONDUIT RUN INDICATES MINIMUM 1" CONDUIT, TWO #10 CU CONDUCTORS PLUS 1#10 CU GND. CROSSBARS INDICATE NUMBER OF #10 CU CONDUCTORS IN CONDUIT. CONDUCTOR SIZES OTHER THAN #10 NOTED ON DRAWINGS. INCREASE CONDUIT SIZE AS REQUIRED TO ACCOMMODATE C.E.C. WIRE FILL REQUIREMENTS. INCLUDE ADDITIONAL BOND WIRE IN ALL PVC AND FLEXIBLE CONDUIT. LONG CROSSBAR INDICATES NEUTRAL CONDUCTOR, SHORT CROSSBARS INDICATE PHASE CONDUCTORS.
- 8. INCREASE BRANCH CIRCUIT CU CONDUCTOR SIZES AS REQUIRED BY THE 120V BRANCH CIRCUIT VOLT DROP CONDUCTOR LENGTH CHART BELOW. USE CONDUCTOR LENGTHS AS FIELD MEASURED, BASED UPON MEASURED FIELD ROUTING LENGTHS. INCREASE MINIMUM CONDUIT SIZE AS REQUIRED TO ACCOMMODATE A MAXIMUM 40% CONDUCTOR FILL OF THE BRANCH CIRCUIT CONDUCTORS. WHERE NECESSARY, PROVIDE A JUNCTION BOX AT ACCESSIBLE CEILING SPACE TO CONVERT THE LAST 15 FEET OF CONDUCTORS TO #10 AWG TO ACCOMMODATE TERMINATION OF CONDUCTORS AT WIRING DEVICES, LIGHTING FIXTURES, CIRCUIT BREAKER, ETC.
- 9. INSTALL CU GROUND CONDUCTOR IN ALL BRANCH CIRCUITS FOR LIGHT FIXTURES AND POWER DEVICES.

### 120V BRANCH CIRCUIT VOLT DROP CONDUCTOR LENGTH CHART

OLI DH	OP CC	ו טטעמי	OK LEN	IGIH C	HAKI							
LOAD IN	LENGTH OF CONDUCTOR											
VOLT		WIRE	SIZE IN (G	AUGE)								
AMPERES	#12	<b>#</b> 10	#8	#6	#4							
1200VA	74	121	183	284	434							
1560VA	57	93	141	218	334							
1800VA	49	81	122	189	289							
1920VA	46	76	115	178	271							
2340VA	Х	62	94	146	223							
2880VA	Х	51	76	118	181							
3000VA	Х	48	73	114	174							
3900VA	Х	Χ	56	87	134							

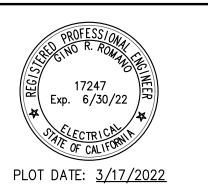
- 1. THIS CHART IS FOR COPPER CONDUCTORS ONLY.
- THIS CHART ASSUMES AN 80% POWER FACTOR AND STEEL RACEWAYS. 3. 2019 CALIFORNIA ENERGY CODE, 130.5(c) ALLOWS A MAXIMUM COMBINED VOLTAGE DROP OF 5%. THIS CHART ASSUMES A MAXIMUM DROP OF 3% FOR FEEDERS. THIS CHART PROVIDES THE MAXIMUM LENGTH OF CONDUCTORS FOR LESS THAN 2% VOLTAGE DROP ON A BRANCH
- CIRCUIT AT GIVEN VA LOAD. 4. USE WIRE SIZE FROM THIS CHART UNLESS LARGER CONDUCTOR SIZES ARE NOTED ON THE

4800VA X X 46 71 108

5. FOR VA VALUES NOT SHOWN USE NEXT HIGHEST VALUE FROM THE CHART







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4 5

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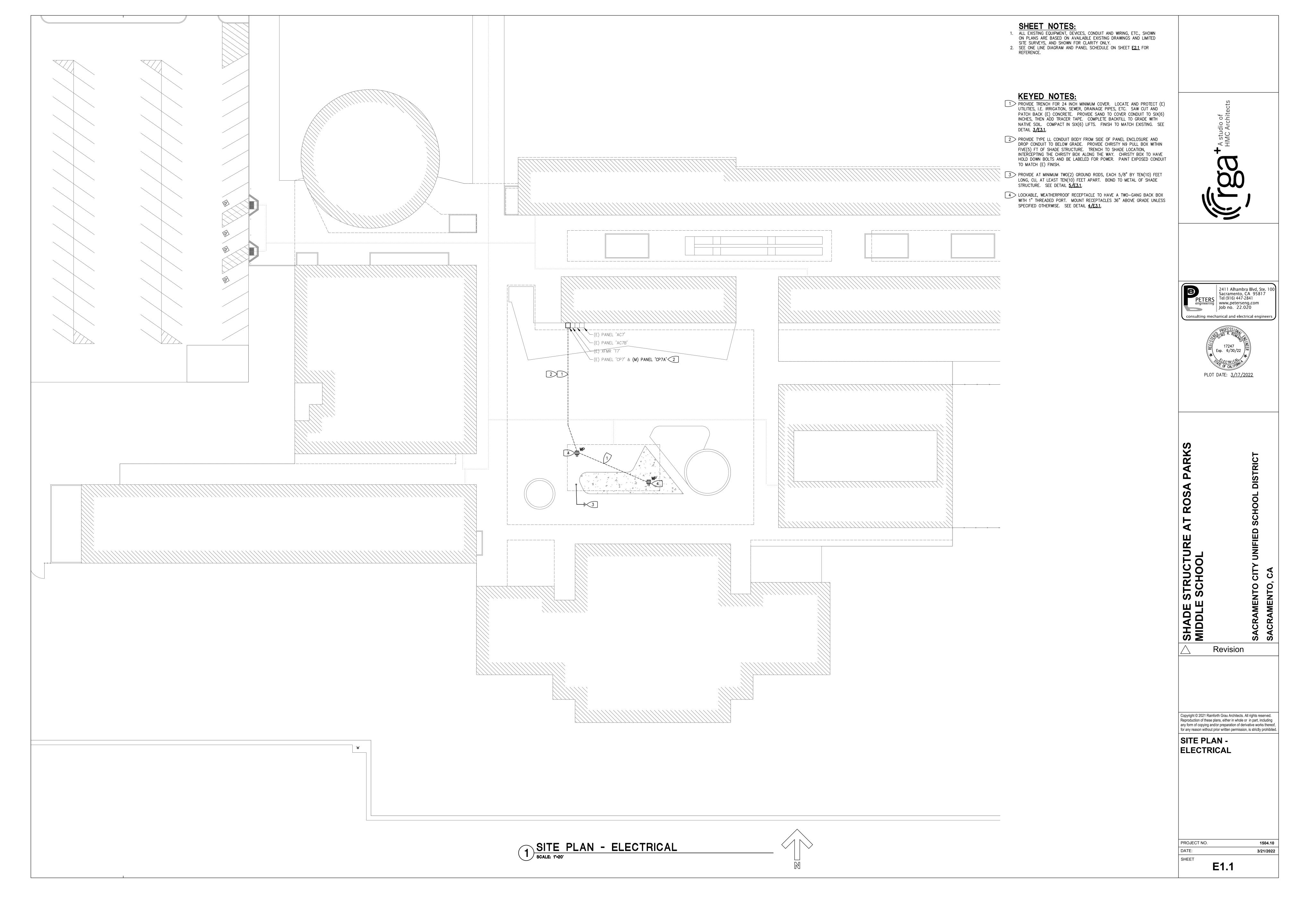
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Revision

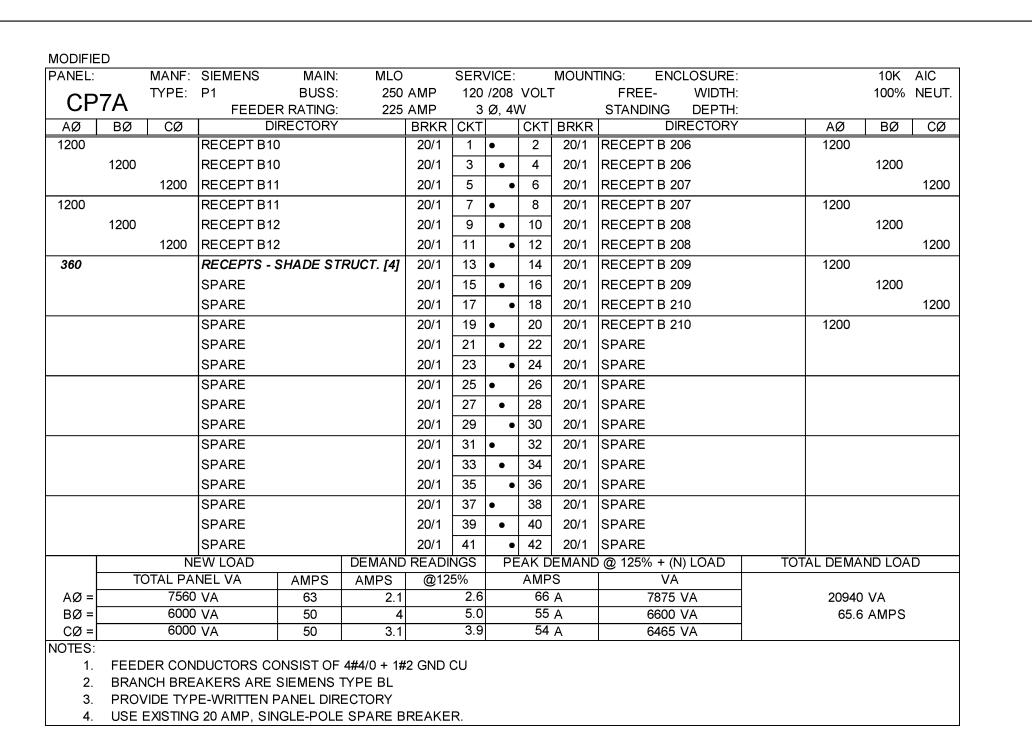
SYMBOLS, NOTES

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PROJECT NO.	1504.10
DATE:	3/21/2022
SHEET FO 1	



		V	oltage	e Drop	Calcu	latio	ns C	opp	er		
lob Name:	Rosa Parks	Elementar	y School -	Shade Struc	cture					Job #:	22.020
Date:	3/10/2022										
	VOLTAGE:	120	PHASE:	1		POWER	FACTOR:	80%	CONDUIT:	S	teel
FEEDER	AMPS AT	KVA	VOLTS	DISTANCE	DISTANCE	WIRES/	LOAD/	WIRE	WIRE	VOLTS	PERCENT
NUMBER	LOAD	TOTAL	AT LOAD	FEET	TOTAL	PHASE	WIRE	SIZE	FACTOR	DROP	VOLT DROP
RECEPT-1	3.0	0.4	119.10	100	100	1	3.00	10	1995	0.60	0.75%
RECEPT-2	1.5	0.2	118.93	58	158	1	1.50	10	1995	0.77	0.89%



\_ \_ \_ \_ \_ \_ \_ \_ \_ \_ \_ \_ \_ \_ \_ \_

RD/3 2500AF

2500AT

(E) SMUD METER #2517737

\(\frac{1}{2} = \frac{1}{2} =

SHEET NOTES:

1. ALL EXISTING EQUIPMENT, DEVICES, CONDUIT AND WIRING, ETC., SHOWN ON PLANS ARE BASED ON AVAILABLE EXISTING DRAWINGS AND LIMITED SITE SURVEYS, AND SHOWN FOR CLARITY ONLY.

**KEYED NOTES:** 1 MODIFIED PANEL SERVES EQUIPMENT BEING ADDED IN THIS PROJECT. SEE PANEL SCHEDULE ON THIS SHEET FOR REFERENCE.

CUT-HAM POWER-R-LINE SWBD

(E)PANEL 'AC7'

(E) XFMR 'T7' 75KVA 480: 208/120V 150°C, 4.77%Z

800A BUS, 208/120V, 3ø, 4W, 65KA SCCR

(E)PANEL 'CP7'

250A BUS, 208/120V, 3ø, 4W, 22KA SCCR

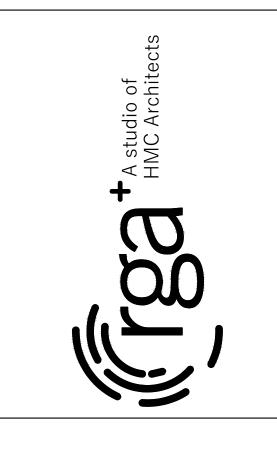
SIEMENS

SIEMENS

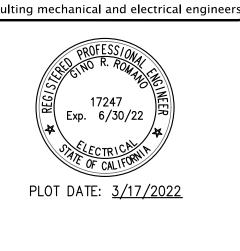
(E)MAIN SWITCHBOARD MSB 2500A BUS, 480/277V, 3ø, 4W, 65KA SCCR 

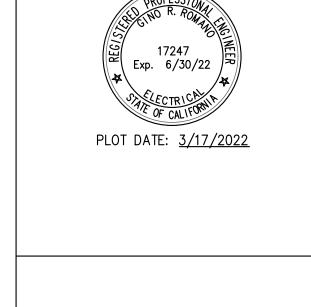
125/3

HMDL/3 ' 800AF . 800AT









STRUCTURE SCHOOL Revision

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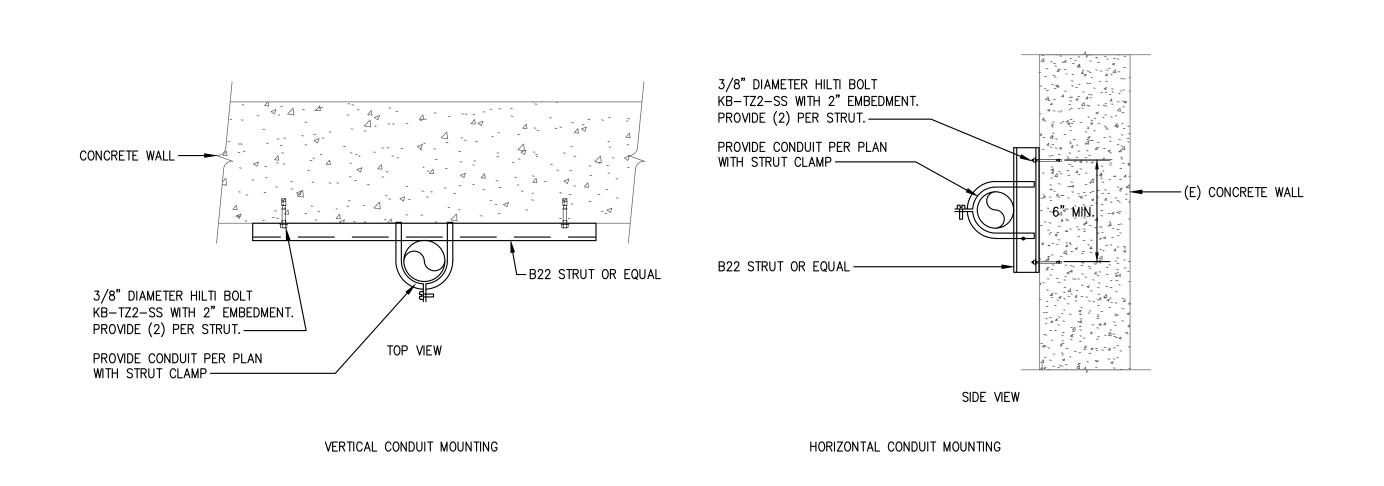
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ONE LINE DIAGRAM

<b></b>	SHEET F2 1	3/21/2022
		3/21/2022
		SHEET

(E)SMUD

TRANSFORMER ———



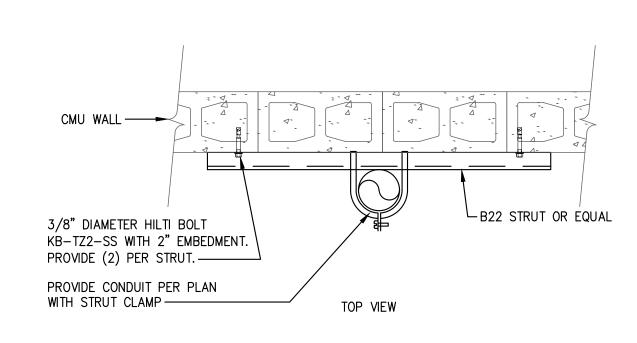
NOTES:

1. CONDUIT SHALL BE SUPPORTED AT INTERVALS NOT EXCEEDING TEN(10)
FEET AND NOT MORE THAN THREE(3) FEET FROM THE OUTLET AND AT
ANY POINT WHERE IT CHANGES DIRECTION.

2. PERFORATED STRAP AND PLUMBER'S TAPE SHALL NOT BE PERMITTED.

3. MAXIMUM CONDUIT AND CONDUCTOR WEIGHT IS 1.83LBS PER LINEAR FOOT.

### 7 CONDUIT MOUNTING DETAIL - CONCRETE WALLS SCALE: NONE



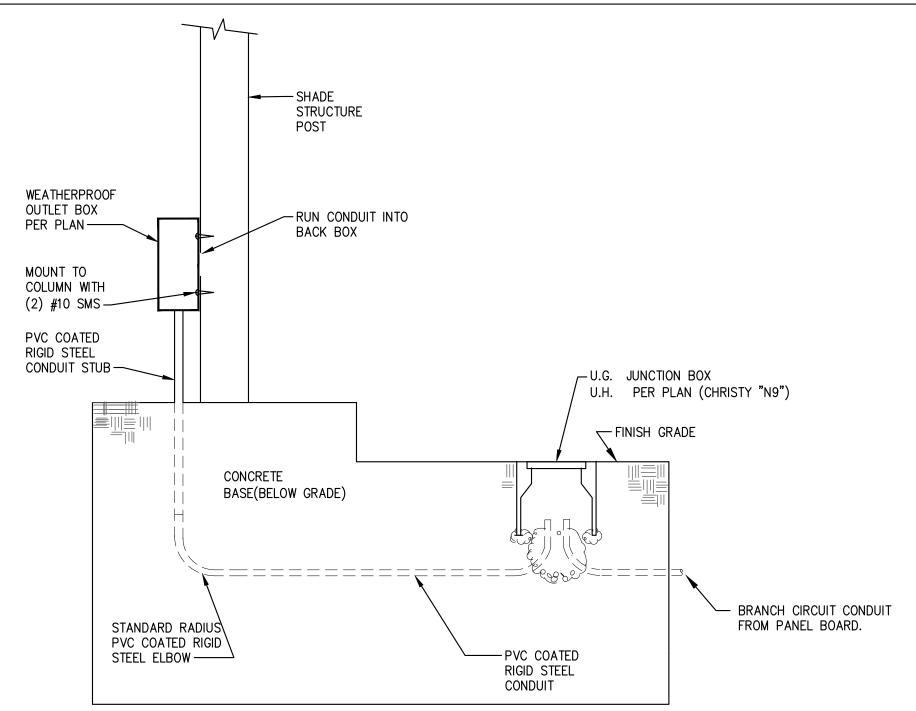
NOTES:

1. CONDUIT SHALL BE SUPPORTED AT INTERVALS NOT EXCEEDING TEN(10)
FEET AND NOT MORE THAN THREE(3) FEET FROM THE OUTLET AND AT
ANY POINT WHERE IT CHANGES DIRECTION.

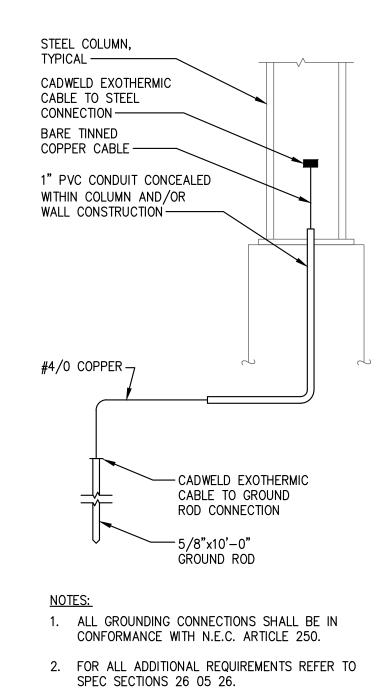
2. PERFORATED STRAP AND PLUMBER'S TAPE SHALL NOT BE PERMITTED.

3. MAXIMUM CONDUIT AND CONDUCTOR WEIGHT IS 1.83LBS PER LINEAR FOOT.

### 8 CONDUIT MOUNTING DETAIL - CMU WALLS SCALE: NONE

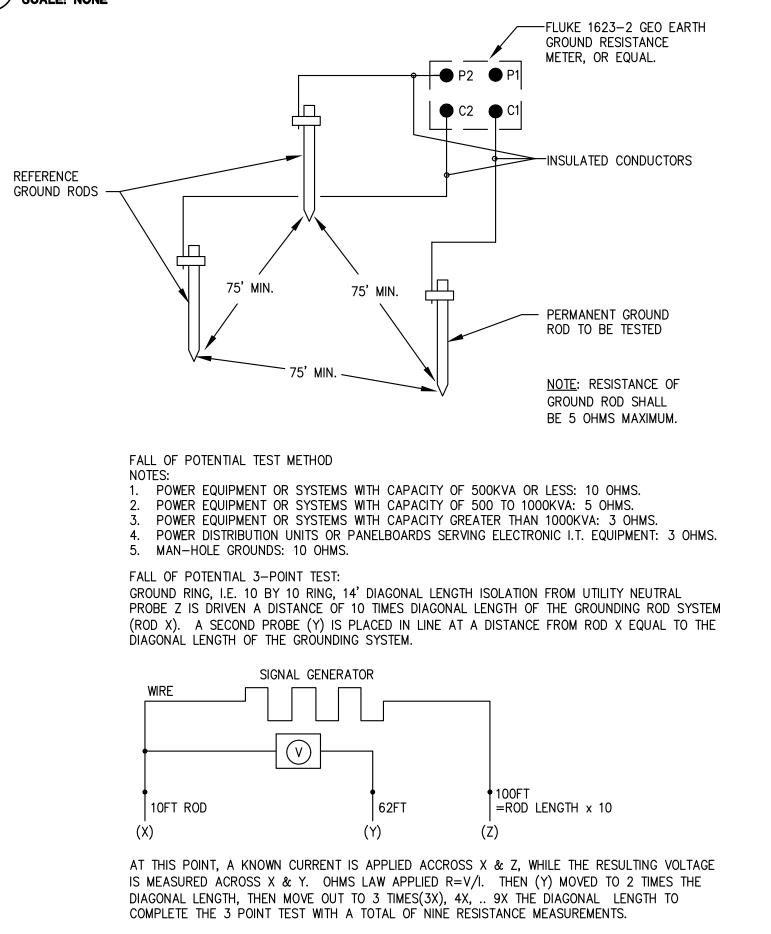


### 4 CONDUIT STUB IN POST DETAIL SCALE: NONE

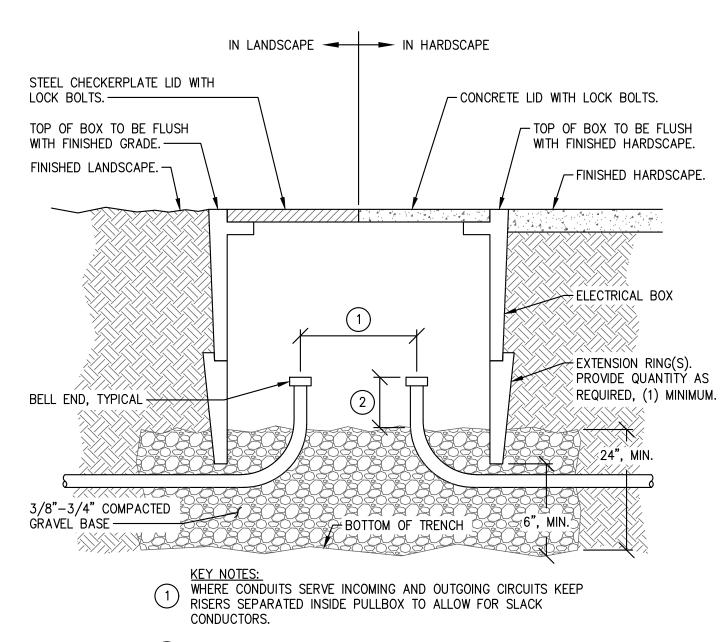


### TYPICAL STEEL COLUMN

### 8 REBAR GROUNDING DETAIL SCALE: NONE

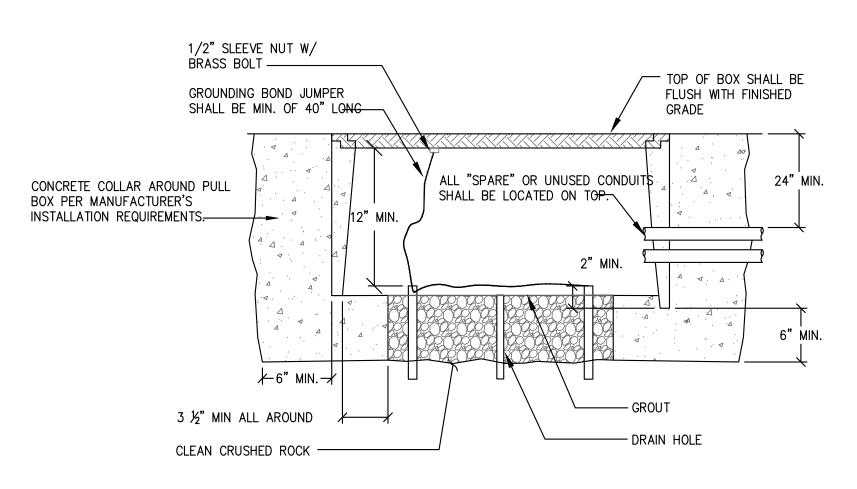


### 6 METHOD OF TESTING GROUND RODS DETAIL SCALE: NONE



### TOPS OF CONDUITS MUST NOT EXTEND INTO PULLBOX MORE THAN 1/3 OF THE TOTAL AVAILABLE INSIDE BOX HEIGHT. IN ORDER TO ALLOW ADEQUATE SPACE FOR CABLE SLACK.

### 1 NON-TRAFFIC RATED PULL BOX SCALE: NONE

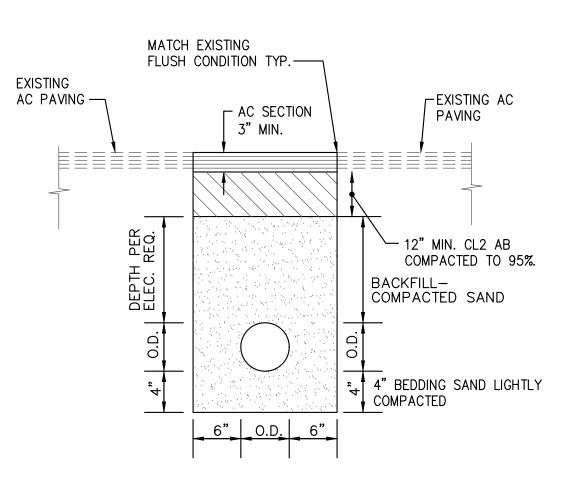


NOTES:

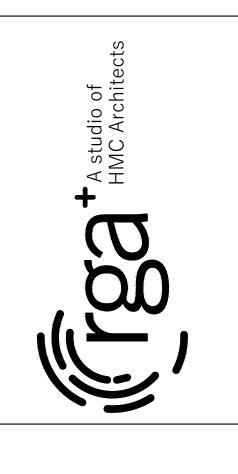
1. HANDHOLES SHALL BE PROVIDE WITH A MINIMUM OF (4) GALVANIZED PULLING PLATES IN BOTTOM OF PULLBOX.

- 2. PULLBOXES SHALL BE PROVIDED WITH CAST IN PLACE VERTICAL CABLE RACKS. ALL CABLES SHALL BE NEATLY BUNDLED, ORGANIZED AND SUPPORTED BY CABLE RACKS.
- 3. WHERE ADDITIONAL CONDUIT ENTRIES ARE REQUIRED BEYOND QUANTITY OF TERMINATORS SHOWN. CONTRACTOR SHALL FIELD CORE DRILL AS REQUIRED. WHERE 4" TERMINATORS ARE PROVIDED CONTRACTOR SHALL PROVIDE CONDUIT REDUCERS TO MATCH SITE CONDUIT SIZE REQUIREMENTS.
- 4. FOR ALTERNATE STYLE PULLBOXES CONTRACTOR SHALL FIELD CORE DRILL ALL CONDUIT ENTRIES 2" DIA
- 5. CONTRACTOR SHALL PROVIDE THE MANUFACTURER'S INSTALLATION INSTRUCTIONS FOR TRAFFIC RATING REQUIREMENTS AS PART OF THE SUBMITTALS.

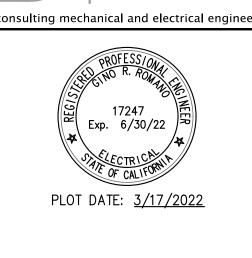
### 2 TRAFFIC RATED PULL BOX SCALE: NONE



3 TYPICAL TRENCH DETAIL
SCALE: NONE







### STRUCTURE AT ROSA PARKS E SCHOOL

SHADE STRUCTURE

MIDDLE SCHOOL

uoisi
ouisi

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DETAILS

PROJECT NO. 1504.10

DATE: 3/21/2022

SHEET

E3.1

SOCF DEAD LOAD (SUPERIMPOSED ON FRAME)   5 PSF	
ROOF LIVE LOAD         20 P           ROOF DEAD LOAD (SUPERIMPOSED ON FRAME)         5 PSF           ROOF PANEL DEAD LOAD         M=1.1 PSF, G=1.2           COLLATERAL DEAD LOAD         M=3.9 PSF, G=3.1           ROOF SNOW LOAD.         20 P           SROUND SNOW LOAD. Pg         20 P           ROOF SNOW LOAD: SLOPED, Pg         20 P           SITE APPLICATION DSA REVIEWER SHALL VERIFY THE STRUCTURE BE LOCATED AT LEAST 20 FEET FROM A SNOW LOAD LOPE FACTOR, Cg         11.6           SNOW EXPOSURE FACTOR, Cg         11.6           SNOW LOAD INPORTANCE FACTOR, Ig         1.1           HERRINA FACTOR, Cg         1.1           SASIC WIND SPEED (3 SECOND GUST), V <sub>cl</sub> 10.0           RISK CATEGORY         0           SASIC WIND SPEED (3 SECOND GUST), V <sub>cl</sub> 10.0           RISK CATEGORY         0           EXPOSURE CATEGORY         0           CACTORS, Kg, Kg, Sc         0.855, 1           Rg DOWER PASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED         CASE A (1.1 / -1.2) G           Cow, PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED         CASE A (0.6 / -0.9)           COMPONENTS & CLADDING - Cg (1 PRESSURE/SUCTION) CLEAR / OBSTRUCTED         CASE A (0.6 / -0.9)           COMPER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED         CASE A (0.6 /	VALUES
SOSP EAD LOAD (SUPERIMPOSED ON FRAME)   5 PSF	DOE
ROOF PANEL DEAD LOAD	_
COLLATERAL DEAD LOAD   ROOF SNOW LOAD	
ROOF SNOW LOAD   ROOF SNOW LOAD   ROOF SNOW LOAD   ROOF SNOW LOAD   ROOF SNOW LOAD SLOPED, P.   20 P   RISK CATEGORY   ■   ■   ROOF SNOW LOAD SLOPED, P.   20 P   STITE APPLICATION DSA REVIEWER SHALL VERIFY THE STRUCTURE BE LOCATED AT LEAST 20 FEET FROM A SNOW LOAD SLOPE FACTOR, C.   1.0   1	
I   ROBERT   ROBER	,
ROOF SNOW LOAD: SLOPED, P <sub>8</sub> STE APPLICATION DSA REVIEWER SHALL VERIFY THE STRUCTURE BE LOCATED AT LEAST 20 FEET FROM A SNOW LOAD SLOPE FACTOR, C <sub>9</sub> 1.6  SNOW LOAD SLOPE FACTOR, C <sub>9</sub> 1.7  SNOW LOAD SLOPE FACTOR, C <sub>9</sub> 1.8  SNOW LOAD IMPORTANCE FACTOR, I <sub>8</sub> 1.9  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 100 M  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 100 M  SISK CATEGORY  1.8  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 100 M  SISK CATEGORY  1.8  CATORS: K <sub>2</sub> , K <sub>2</sub> , K <sub>3</sub> 1.9  1.9  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.0  SASIC	PSF
SITE APPLICATION DSA REVIEWER SHALL VERIFY THE STRUCTURE BE LOCATED AT LEAST 20 FEET FROM A SNOW LOAD SLOPE FACTOR, C <sub>0</sub> 1.0.  SNOW EXPOSURE FACTOR, C <sub>0</sub> 1.0.  SNOW EXPOSURE FACTOR, C <sub>0</sub> 1.1.  SNOW EXPOSURE FACTOR, C <sub>0</sub> 1.1.  SNOW EXPOSURE FACTOR, C <sub>0</sub> 1.2.  SNOW EXPOSURE FACTOR, C <sub>0</sub> 1.3.  SNOW EXPOSURE FACTOR, C <sub>0</sub> 1.4.  SNOW EXPOSURE FACTOR, C <sub>0</sub> 1.5.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.6.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.7.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.8.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.9.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.  SASIC WIND SPEED (3 SECOND GUST), V <sub>olt</sub> 1.0.	1
SNOW EXPOSURE FACTOR, C <sub>e</sub> 1.1. SNOW LOAD IMPORTANCE FACTOR, I <sub>e</sub> 1.2. THERMAL FACTOR, C <sub>1</sub> 3.  WIND DESIGN  BASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.  BASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.  BASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 1.  1.  1.  1.  1.  1.  1.  1.  1.  1	PSF
SNOW EXPOSURE FACTOR, C <sub>0</sub> 1.1.  SNOW LOAD IMPORTANCE FACTOR, I <sub>k</sub> 1.1.  THERMAL FACTOR, C <sub>1</sub> BASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 100 M  RISK CATEGORY  CACTORS: K <sub>0</sub> , K <sub>th</sub> , K <sub>d</sub> Question (3 M  Question (4 M  Question	ADJACENT STRUCTURE
SENOW LOAD IMPORTANCE FACTOR, I <sub>1</sub> THERMAL FACTOR, C <sub>1</sub> MIND DESIGN  BASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> 100 M  RISK CATEGORY  RISK CATEGORY  CEXPOSURE CATEGORY  COACTION  A <sub>1</sub> = 0.00256 K, K <sub>1</sub> , K <sub>2</sub> K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>4</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>2</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>4</sub> , K <sub>3</sub> B <sub>4</sub> = 0.00256 K, K <sub>4</sub> B <sub>5</sub> = 0.00256 K, K <sub>4</sub> B <sub>4</sub> = 0.00256 K, K <sub>4</sub> B <sub>5</sub> = 0.00256 K, K <sub>4</sub> B <sub>4</sub> =	.0
THERMAL FACTOR, C, WIND DESIGN  BASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> . 100 M RISK CATEGORY   II  EXPOSURE CATEGORY   II  EXPOSURE CATEGORY   C  FACTORS, K <sub>x</sub> , K <sub>x</sub> , K <sub>x</sub>   0.85, 1  g <sub>x</sub> = 0.00256 K, K <sub>x</sub> , K <sub>x</sub>   0.85, 1  g <sub>x</sub> = 0.00256 K, K <sub>x</sub> , K <sub>x</sub>   0.85, 1  g <sub>x</sub> = 0.00256 K, K <sub>x</sub>   0.80 F ANGLE 18.43 - CLEAR / OBSTRUCTED   CASE A (1.1 /-1.2)    C <sub>ML</sub> PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED   CASE A (0.17 /-1.09)    C <sub>ML</sub> PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED   CASE A (-0.17 /-1.09)    C <sub>ML</sub> PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED   CASE A (-0.17 /-1.09)    COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED   CONE 3 - (0.29 /-  ZONE 2 - (1.77 /-  ZONE 1 - (1.15 /-  SEISMIC DESIGN   STEEL - ORDINARY CA  ANALYSIS PROCEDURE   STEEL - ORDINARY CA  SEISMIC IMPORTANCE FACTOR, Ie   EQUIVALENT LA  SEISMIC SITE CLASS   D  MCE <sub>R</sub> SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>3</sub>   0.9  SHORT PERIOD SITE COEFFICIENT, F <sub>a</sub>   1.2  LONG PERIOD COEFFICIENT, F <sub>a</sub>   1.7  FUNDAMENTAL PERIOD OF THE STRUCTURE, T   0.15  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> - USED   2.08 * 0.70  TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3)   1.0  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub>   1.0  SEISMIC DESIGN CATEGORY   1.2  RESPONSE MODIFICATION FACTOR, R   1.2  OVERSTRENGTH FACTOR, Ω   1.2  RESDUNDANCY FACTOR, Ω   1.2  RESD	.0
THERMAL FACTOR, C, WIND DESIGN  BASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> . 100 M RISK CATEGORY   II  EXPOSURE CATEGORY   II  EXPOSURE CATEGORY   C  FACTORS, K <sub>x</sub> , K <sub>x</sub> , K <sub>x</sub>   0.85, 1  g <sub>x</sub> = 0.00256 K, K <sub>x</sub> , K <sub>x</sub>   0.85, 1  g <sub>x</sub> = 0.00256 K, K <sub>x</sub> , K <sub>x</sub>   0.85, 1  g <sub>x</sub> = 0.00256 K, K <sub>x</sub>   0.80 F ANGLE 18.43 - CLEAR / OBSTRUCTED   CASE A (1.1 /-1.2)    C <sub>ML</sub> PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED   CASE A (0.17 /-1.09)    C <sub>ML</sub> PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED   CASE A (-0.17 /-1.09)    C <sub>ML</sub> PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED   CASE A (-0.17 /-1.09)    COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED   CONE 3 - (0.29 /-  ZONE 2 - (1.77 /-  ZONE 1 - (1.15 /-  SEISMIC DESIGN   STEEL - ORDINARY CA  ANALYSIS PROCEDURE   STEEL - ORDINARY CA  SEISMIC IMPORTANCE FACTOR, Ie   EQUIVALENT LA  SEISMIC SITE CLASS   D  MCE <sub>R</sub> SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>3</sub>   0.9  SHORT PERIOD SITE COEFFICIENT, F <sub>a</sub>   1.2  LONG PERIOD COEFFICIENT, F <sub>a</sub>   1.7  FUNDAMENTAL PERIOD OF THE STRUCTURE, T   0.15  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> - USED   2.08 * 0.70  TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3)   1.0  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub>   1.0  SEISMIC DESIGN CATEGORY   1.2  RESPONSE MODIFICATION FACTOR, R   1.2  OVERSTRENGTH FACTOR, Ω   1.2  RESDUNDANCY FACTOR, Ω   1.2  RESD	.0
WIND DESIGN           BASIC WIND SPEED (3 SECOND GUST), V <sub>ol</sub> R         100 M           RISK CATEGORY         0.85, 1           EXPOSURE CATEGORY         0.85, 1           10 <sub>a</sub> = 0.00256 K <sub>x</sub> K <sub>xt</sub> V <sub>x</sub> FOR ALL EAVE HEIGHTS (8', 10' & 12')         18.50           0.2 <sub>MV</sub> PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED         CASE A (1.1 / -1.2)           0.8 <sub>MV</sub> PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED         CASE A (-0.17 / -1.09)           0.2 <sub>MV</sub> PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED         CASE A (-0.6 / -0.9)           0.00MPONENTS & CLADDING - C <sub>N</sub> (PRESSURE/SUCTION) CLEAR / OBSTRUCTED         CASE A (-0.6 / -0.9)           0.00MPONENTS & CLADDING - C <sub>N</sub> (PRESSURE/SUCTION) CLEAR / OBSTRUCTED         ZONE 3 - (2.29 / -2.20	2
BASIC WIND SPEED (3 SECOND GUST), V <sub>ult</sub> RISK CATEGORY  RISK CATEGORY  CATEGORY  CASEA (-0.85, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1,	
RISK CATEGORY  EXPOSURE CATEGORY  C EXPOSURE CATEGORY  C DATE OF THE EXPOSURE CATEGORY  RESPONSE MODIFICATION FACTOR, ρ  RESPONSE MODIFICATION FOR TICK. 20' WIDE, 30' WIDE, 40' WIDE)  1.10 DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  1.11 DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  1.11 DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  1.12 DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  1.14 DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  1.15 DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  1.16 DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  1.17 PENDAMENTAL OF CATEGORY  C DASS A (-0.0 f, 0.15)  C ASE A (-0.1 f, -1.2)	MPH
EXPOSURE CATEGORY FACTORS: K <sub>2</sub> , K <sub>2</sub> , K <sub>3</sub> 0.85, 1  The = 0.00256 K <sub>2</sub> K <sub>2</sub> K <sub>4</sub> V <sub>2</sub> Por All EAVE HEIGHTS (8', 10' & 12') 1.9, = 0.00256 K <sub>2</sub> K <sub>2</sub> K <sub>4</sub> V <sub>2</sub> Por All EAVE HEIGHTS (8', 10' & 12') 1.9, = 0.00256 K <sub>3</sub> K <sub>4</sub> K <sub>4</sub> V <sub>2</sub> Por All EAVE HEIGHTS (8', 10' & 12') 1.9, PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED 1.0, PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED 1.0, PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED 1.0, PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED 1.0, PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED 1.0, PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED 1.0, PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED 1.0, PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED 1.0, PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED 1.0, PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED 1.0, PER SUBJECT OBSTRUCTER, TO 1.1, PER SUBJECT OBSTRUCTED 1.0, PER SUBJECT OBSTRUCTER, TO 1.1, PER SUBJECT OBSTRUCTED 1.0, PER SUBJECT OBSTRUCTURE, TO 1.1, PER SUBJECT OBSTRUCTED 1.0, PER SUBJECT OBSTRUCTURE, TO 1.1, PER SUBJECT OBSTRUCTED	
FACTORS: K <sub>2</sub> , K <sub>2</sub> , K <sub>3</sub> Q <sub>1</sub> = 0.00256 K <sub>2</sub> K <sub>2</sub> , K <sub>3</sub> V <sup>2</sup> FOR ALL EAVE HEIGHTS (8', 10' & 12')  18.50 C <sub>MW</sub> PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED CASE A (1.1 / -1.2) C <sub>ML</sub> PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED CASE A (-0.17 / -1.09) C <sub>M</sub> PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLEAR - (OBSTRUCTED COMPONENTS & CLAD - (OBSTRUCTED COMPONENTS & CLACE COMPONENTS & COMPONENTS	
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	1, 0.85
C <sub>MM</sub> PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED         CASE A (-0.17 /-1.09)           C <sub>NL</sub> PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED         CASE A (-0.17 /-1.09)           C <sub>N</sub> PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED         CASE A (-0.6 /-0.9)           COMPONENTS & CLADDING - C <sub>N</sub> ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED         ZONE 3 - (2.29 /-2           ZONE 2 - (1.77 /-1         ZONE 1 - (1.15 /-1           SEISMIC DESIGN         ZONE 1 - (1.15 /-1           LATERAL FORCE RESISTING SYSTEM         STEEL - ORDINARY C/A           ANALYSIS PROCEDURE         EQUIVALENT LA           SEISMIC IMORTANCE FACTOR, Ie         11.0           SEISMIC SITE CLASS         D           MCE <sub>R</sub> SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>s</sub> 2.6           MCE <sub>R</sub> SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>s</sub> 2.6           MCE <sub>R</sub> SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>s</sub> 1.2           LONG PERIOD COEFFICIENT, F <sub>s</sub> 1.2           LONG PERIOD COEFFICIENT, F <sub>s</sub> 1.2           DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> - USED         2.08 * 0.70           DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> - USED         2.08 * 0.70           DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> 1.0           DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-	
$C_{NL} \ PER \ ASCE FIGURE 27.4-5 \ ROOF \ ANGLE 18.43 - CLEAR / OBSTRUCTED                                    $	
$ \begin{array}{c} C_N  \text{PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED} & CASE A  (-0.6  /  -0.9) \\ \hline COMPONENTS & CLADDING - C_N  (  \text{PRESSURE/SUCTION})  \text{CLEAR / OBSTRUCTED}} & ZONE  3 -  (2.29  /  \cdot  20  \text{Ne}  2 -  (1.77  /  -  20  /  -  20  \text{NE}  2 -  (1.28  /  -  20  ,  -  20  \text{NE}  2 $	· · · · · · · · · · · · · · · · · · ·
COMPONENTS & CLADDING - $C_N$ ( PRESSURE/SUCTION) CLEAR / OBSTRUCTED  ZONE 3 - $(2.29)$ / $(2.29)$ / $(2.20)$	
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	· · · · · · · · · · · · · · · · · · ·
$ \begin{array}{c} \text{ZONE 1 - } 1.15 I - \\ \text{SEISMIC DESIGN} \\ \\ \text{LATERAL FORCE RESISTING SYSTEM} \\ \text{STEEL - ORDINARY CAMALYSIS PROCEDURE} \\ \text{SESIMIC IMORTANCE FACTOR, Ie} \\ \text{SEISMIC SITE CLASS} \\ \text{MCE}_R \text{ SPECTRAL RESPONSE ACCELERATION @ 0.2 \text{ s, S}_S } \\ \text{2.6} \\ \text{MCE}_R \text{ SPECTRAL RESPONSE ACCELERATION @ 0.2 \text{ s, S}_1 } \\ \text{3.1} \\ \text{3.2} \\ \text{4.1} \\ \text{5.2} \\ \text{5.2} \\ \text{5.3} \\ \text{5.4} \\ \text{5.2} \\ \text{6.4} \\ \text{6.2} \\ \text{6.2} \\ \text{6.2} \\ \text{6.2} \\ \text{6.2} \\ \text{6.2} \\ \text{6.3} \\ \text{6.2} \\ \text{6.2} \\ \text{6.3} \\ \text{6.2} \\ \text{6.2} \\ \text{6.3} \\ \text{6.2} \\ \text{6.3} \\ \text{6.2} \\ \text{6.3} \\ \text{6.3} \\ \text{6.4} \\ \text{6.2} \\ \text{6.2} \\ \text{6.2} \\ \text{6.2} \\ \text{6.3} \\ \text{6.2} \\ \text{6.3} \\ \text{6.3} \\ \text{6.4} \\ \text{6.2} \\ \text{6.2} \\ \text{6.3} \\ \text{6.2} \\ \text{6.3} \\ \text{6.3} \\ \text{6.3} \\ \text{6.3} \\ \text{6.4} \\ \text{6.4} \\ \text{6.2} \\ \text{6.2} \\ \text{6.4} \\ \text{6.2} \\ \text{6.4} \\ 6.4$	
LATERAL FORCE RESISTING SYSTEM  ANALYSIS PROCEDURE  EQUIVALENT LA SESIMIC IMORTANCE FACTOR, Ie  SEISMIC SITE CLASS  D $MCE_R$ SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>S</sub> $C.6$ $MCE_R$ SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>1</sub> $C.6$ SHORT PERIOD SITE COEFFICIENT, F <sub>a</sub> $C.6$ LONG PERIOD COEFFICIENT, F <sub>v</sub> $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $C.6$ DESIGN CATEGORY  EQUIVALENT LA  DESIGN CATEGORY  EQUIVALENT LA  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $C.6$	-1.05) / (0.5 / -1.5)
ANALYSIS PROCEDURE  SESIMIC IMORTANCE FACTOR, le  1.6  SEISMIC SITE CLASS $MCE_R$ SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>S</sub> 2.6 $MCE_R$ SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>S</sub> 3.7  SHORT PERIOD SITE COEFFICIENT, F <sub>a</sub> 1.7  FUNDAMENTAL PERIOD OF THE STRUCTURE, T  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> 2.0  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> - USED  TO DETERMINE CS (WITH CAP PER ASCE-7 12.8.1.3)  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> 1.0  SEISMIC DESIGN CATEGORY  E  RESPONSE MODIFICATION FACTOR, R  1.2  OVERSTRENGTH FACTOR, $\Omega$ 1.2  REDUNDANCY FACTOR, $\rho$ 1.4  HORIZONTAL OR VERTICAL IRREGULARITIES  SEISMIC RESPONSE COEFFICIENT, CS (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  1.2.73 PSF, 13.41	ANTILEVER COLUMN
SEISMIC SITE CLASS $MCE_R$ SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>S</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>S</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S <sub>1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> - USED $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S <sub>DS</sub> - USED $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S <sub>D2</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S <sub>D2</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S <sub>D2</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S <sub>D2</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S <sub>D2</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S <sub>D2</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S <sub>D2</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S <sub>D2</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S <sub>D2</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S <sub>D2</sub> $MCE_R$ SPECTRAL RESPONSE ACCELERATION AT S	
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	.0
MCE <sub>R</sub> SPECTRAL RESPONSE ACCELERATION @ $0.2 \text{ s}$ , $S_1$ SHORT PERIOD SITE COEFFICIENT, $F_a$ LONG PERIOD COEFFICIENT, $F_v$ 1.7  FUNDAMENTAL PERIOD OF THE STRUCTURE, T  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ 2.0  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ - USED  TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3)  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, $S_{D1}$ 1.0  SEISMIC DESIGN CATEGORY  ERSPONSE MODIFICATION FACTOR, R  OVERSTRENGTH FACTOR, $D$ 1.2  REDUNDANCY FACTOR, $D$ 1.4  HORIZONTAL OR VERTICAL IRREGULARITIES  SEISMIC RESPONSE COEFFICIENT, Cs ( $D_{CO}$ WIDE, $D_{CO}$ W	)
SHORT PERIOD SITE COEFFICIENT, $F_a$ LONG PERIOD COEFFICIENT, $F_v$ 1.7  FUNDAMENTAL PERIOD OF THE STRUCTURE, T  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ 2.0  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ - USED  TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3)  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, $S_{D1}$ 1.0  SEISMIC DESIGN CATEGORY  ERSPONSE MODIFICATION FACTOR, R  OVERSTRENGTH FACTOR, $\Omega$ 1.2  REDUNDANCY FACTOR, $\rho$ HORIZONTAL OR VERTICAL IRREGULARITIES  NON  SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  12.73 PSF, 13.41	60
LONG PERIOD COEFFICIENT, $F_V$ FUNDAMENTAL PERIOD OF THE STRUCTURE, T  0.15.  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ 2.0  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ - USED  TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3)  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, $S_{D1}$ 1.0  SEISMIC DESIGN CATEGORY  ERSPONSE MODIFICATION FACTOR, R  1.2  OVERSTRENGTH FACTOR, $\Omega$ 1.2  REDUNDANCY FACTOR, $\rho$ HORIZONTAL OR VERTICAL IRREGULARITIES  NON  SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  12.73 PSF, 13.41	90
FUNDAMENTAL PERIOD OF THE STRUCTURE, T 0.15.  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ 2.0  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ - USED 2.08 * 0.70  TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3)  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, $S_{D1}$ 1.0  SEISMIC DESIGN CATEGORY E RESPONSE MODIFICATION FACTOR, R 1.2  OVERSTRENGTH FACTOR, $\Omega$ 1.2  REDUNDANCY FACTOR, $\rho$ 1.0  HORIZONTAL OR VERTICAL IRREGULARITIES NON SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE) 1.16  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE) 12.73 PSF, 13.41	20
FUNDAMENTAL PERIOD OF THE STRUCTURE, T 0.15.  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ 2.0  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ - USED 2.08 * 0.70  TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3)  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, $S_{D1}$ 1.0  SEISMIC DESIGN CATEGORY E RESPONSE MODIFICATION FACTOR, R 1.2  OVERSTRENGTH FACTOR, $\Omega$ 1.2  REDUNDANCY FACTOR, $\rho$ 1.0  HORIZONTAL OR VERTICAL IRREGULARITIES NON SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE) 1.16  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE) 12.73 PSF, 13.41	70
DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ 2.0  DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ - USED 2.08 * 0.70  TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3)  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, $S_{D1}$ 1.0  SEISMIC DESIGN CATEGORY  RESPONSE MODIFICATION FACTOR, R 1.2  OVERSTRENGTH FACTOR, $\Omega$ 1.2  REDUNDANCY FACTOR, $\rho$ 1.0  HORIZONTAL OR VERTICAL IRREGULARITIES NON  SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE) 1.16  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE) 12.73 PSF, 13.41	52 s
DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ - USED  TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3)  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, $S_{D1}$ SEISMIC DESIGN CATEGORY  RESPONSE MODIFICATION FACTOR, R  OVERSTRENGTH FACTOR, $\Omega$ REDUNDANCY FACTOR, $\rho$ HORIZONTAL OR VERTICAL IRREGULARITIES  SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  12.73 PSF, 13.41	
TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3)  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> SEISMIC DESIGN CATEGORY  RESPONSE MODIFICATION FACTOR, R  OVERSTRENGTH FACTOR, Ω  REDUNDANCY FACTOR, ρ  HORIZONTAL OR VERTICAL IRREGULARITIES  SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  12.73 PSF, 13.41	08
TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3)  DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> SEISMIC DESIGN CATEGORY  RESPONSE MODIFICATION FACTOR, R  OVERSTRENGTH FACTOR, Ω  REDUNDANCY FACTOR, ρ  HORIZONTAL OR VERTICAL IRREGULARITIES  SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  12.73 PSF, 13.41	
DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S <sub>D1</sub> SEISMIC DESIGN CATEGORY  RESPONSE MODIFICATION FACTOR, R  OVERSTRENGTH FACTOR, Ω  REDUNDANCY FACTOR, ρ  HORIZONTAL OR VERTICAL IRREGULARITIES  SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  12.73 PSF, 13.41	<b>7</b> 0 = 1.456
SEISMIC DESIGN CATEGORY  RESPONSE MODIFICATION FACTOR, R  1.2  OVERSTRENGTH FACTOR, Ω  REDUNDANCY FACTOR, ρ  HORIZONTAL OR VERTICAL IRREGULARITIES  SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  12.73 PSF, 13.41	
SEISMIC DESIGN CATEGORY  RESPONSE MODIFICATION FACTOR, R  1.2  OVERSTRENGTH FACTOR, Ω  REDUNDANCY FACTOR, ρ  HORIZONTAL OR VERTICAL IRREGULARITIES  SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  12.73 PSF, 13.41	
RESPONSE MODIFICATION FACTOR, R  OVERSTRENGTH FACTOR, $\Omega$ REDUNDANCY FACTOR, $\rho$ HORIZONTAL OR VERTICAL IRREGULARITIES  SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  12.73 PSF, 13.41	
OVERSTRENGTH FACTOR, $\Omega$ 1.2  REDUNDANCY FACTOR, $\rho$ 1.0  HORIZONTAL OR VERTICAL IRREGULARITIES NON SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE) 1.10  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE) 12.73 PSF, 13.41	
REDUNDANCY FACTOR, ρ  HORIZONTAL OR VERTICAL IRREGULARITIES  SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  12.73 PSF, 13.41	
HORIZONTAL OR VERTICAL IRREGULARITIES  SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  12.73 PSF, 13.41	
SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE)  DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE)  1.10  1.273 PSF, 13.41	
ALLOWABLE SOIL BEARING FOR FOUNDATIONS  VARIES - SEE FOUN	· ·
ALLOWABLE SOIL BEARING FOR FOUNDATIONS VARIES - SEE FOUN	
	NDATION CHARTS
FLOOD DESIGN - DESIGN IS ASSUMED TO NOT BE IN FLOOD HAZARD AREA	
IF PROJECT IS LOCATED IN A FLOOD ZONE OTHERTHAN ZONE X, A LETTER STAMPED & SIGNED FROM A SOILS ENGINEER IS REQUIRED TO VALIDATE THE	

ALL DEFLECTIONS SHOWN ALSO INCLUDE THE P-DE	LTA ROTATION PER IR PC-7	DEFLECT	IONS ARE FOR (1) STI	RUCTURE
		SOIL	CLASSES PER CBC TABLE 18	06A.2
MAXIMUM DRIFT $\delta_{max}$ SIDE COLUMNS		Soil Class 5	Soil Class 4	Soil Class 3
20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.40	2.55	2.65
30' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.25	2.35	2.45
40' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT) MINIMUM SEPARATION $(\delta_m = C_d \ \delta_{max})$ $C_d = 1.25$	(INCHES)	2.20	2.25	2.20
20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	3.00	3.19	3.31
30' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.81	2.94	3.06
40' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.75	2.81	2.75
MAXIMUM DRIFT $\delta_{max}$ CORNER COLUMNS		Soil Class 5	Soil Class 4	Soil Class 3
20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.20	2.30	2.40
30' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.30	2.45	2.50
40' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT) MINIMUM SEPARATION $(\delta_m = C_d \ \delta_{max})$ $C_d = 1.25$	(INCHES)	2.40	2.55	2.65
20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.75	2.88	3.00
30' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.88	3.06	3.13
40' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	3.00	3.19	3.31
MAXIMUM DRIFT $\delta_{max}$ END COLUMNS		Soil Class 5	Soil Class 4	Soil Class 3
20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	1.60	1.70	1.75
30' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.00	2.45	2.25
40' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT) MINIMUM SEPARATION $(\delta_m = C_d \ \delta_{max})$ $C_d = 1.25$	(INCHES)	2.50	2.30	2.80
20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.00	2.13	2.19
30' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.50	3.06	2.81
40' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	3.13	2.88	3.50

STRUCTURAL SEPARATION

ARCHITEC TURAL REQUIREMENTS	
DESC RIPTION	DESIGN VAULES
TYPE OF CONSTRUCTION	II-B
OCCUPANCY CLASSIFICATION	A-3
NUMBER OF STORIES	1
FIRE SPRINKLER SYSTEM	NOT BY ICON/WEIGHT NOT INCLUDED IN DESIGN

### RELATED BUILDING CODES AND STANDARDS

### TITLE 24 CODES:

2019 CALIFORNIA ADMINISTRATIVE CODE (CAC).. ..(PART 1, TITLE 24, CCR) 2019 CALIFORNIA BUILDING CODE (CBC), VOLUMES 1, AND 2.(PART 2, TITLE 24,

2019 CALIFORNIA ELECTRICAL CODE. .(PART 3, TITLE 24, CCR) 2019 CALIFORNIA MECHANICAL CODE (CMC). .(PART 4, TITLE 24, CCR) (PART 5, TITLE 24, CCR) 2019 CALIFORNIA PLUMBING CODE (CPC).. 2019 CALIFORNIA ENERGY CODE. .(PART 6, TITLE 24, CCR) 2019 CALIFORNIA FIRE CODE (CFC) .(PART 9, TITLE 24, CCR` 2019 CALIFORNIA GREEN BUILDING STANDARDS CODE.....(PART 11, TITLE 24, CCR) 2019 CALIFORNIA REFERENCE STANDARDS CODE.. ..(PART 12, TITLE 24, CCR)

REFERENCE CODE SECTIONS FOR APPLICABLE STANDARDS: 2019 CBC, CHAPTER 35

### 2019 CFC, CHAPTER 80 SCOPE OF WORK NARRATIVE

THESE DRAWINGS ILLUSTRATE THE FABRICATION AND INSTALLATION REQUIREMENTS FOR A FREE-STANDING PREFABRIC ATED STEEL SHADE STRUCTURE. THE ENTIRE STRUCTURAL SYSTEM IS COMPRISED OF HOLLOW STRUCTURAL STEEL MEMBERS SUPPORTED BY CONCRETE FOUNDATIONS. THE FLEXIBILITY INCLUDED HEREIN

ALLOWS THE STRUCTURE TO COMPLY WITH A WIDE VARIETY OF PROJECT SITES AND LOADING REQUIREMENTS.

- 1. GENERAL NOTES AND TYPICAL DETAILS SHALL APPLY TO ALL PARTS OF THE JOB EXCEPT WHERE THEY MAY CONFLICT WITH DETAILS AND NOTES ON OTHER SHEETS. WHERE CONDITIONS ARE NOT SPECIFICALLY INDICATED BUT ARE OF SIMILAR CHARACTER TO DETAILS SHOWN, SIMILAR DETAILS OF CONSTRUCTION SHALL BE USED SUBJECT TO REVIEW BY THE STRUCTURAL ENGINEER FOR THIS PROJECT
- 2. WORK SHALL CONFORM TO THE REQUIREMENTS, AS AMENDED TO DATE, OF THE LATEST ADOPTED EDITION OF THE CBC, C.A.C. TITLE 24, AND ALL OTHER LOCAL, STATE AND FEDERAL REGULATIONS
- 3. OMISSIONS OR CONFLICTS BETWEEN THE VARIOUS ELEMENTS OF THE WORKING DRAWINGS AND/OR SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER FOR THIS PROJECT PRIOR TO PROCEEDING
- 4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING THE WORK OF ALL TRADES AND SHALL CHECK ALL DIMENSIONS, ALL DISCREPANCIES SHALL BE CALLED TO THE ATTENTION OF THE STRUCTURAL ENGINEER FOR THIS PROJECT AND BE RESOLVED BEFORE PROCEEDING WITH THE WORK.
- 5. THESE CONSTRUCTION DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE AND DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND SHALL BE SOLELY RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES, INCLUDING, BUT NOT LIMITED TO, BRACING, TEMPORARY SUPPORTS, AND SHORING. OBSERVATION VISIT TO THE SITE BY FIELD REPRESENTATIVES OF THE ARCHITECT/ENGINEER SHALL NOT INCLUDE INSPECTIONS OF THE PROTECTIVE MEASURES OR THE CONSTRUCTION PROCEDURES. ANY SUPPORT SERVICES PERFORMED BY THE ARCHITECT/ENGINEER DURING THE CONSTRUCTION SHALL BE DISTINGUISHED FROM CONSTRUCTION AND DETAILED INSPECTION SERVICES WHICH ARE FURNISHED BY OTHERS. THESE SUPPORT SERVICES PERFORMED BY THE ARCHITECT/ENGINEER, WHETHER OF MATERIAL OR WORK, ARE FOR THE PURPOSE OF ASSISTING IN QUALITY
- CONTROL AND IN ACHIEVING CONFORMANCE WITH CONTRACT DOCUMENTS, BUT DO NOT GUARANTEE CONSTRUCTION. 6. ASTM DESIGNATIONS AND ALL STANDARDS REFER TO THE LATEST AMENDMENTS.
- 7. CONFORM TO APPLICABLE CAL/OSHA CONSTRUCTION SAFETY REGULATIONS FOR ALL WORK PERFORMED DURING CONSTRUCTION. JOB SITE SAFETY IS STRICTLY THE RESPONSIBILITY OF THE CONTRACTOR AND NOT THE ARCHITECT/ENGINEER OR OWNER.
- 8. THE ENGINEER AND THEIR CONSULTANTS SHALL HAVE NO RESPONSIBILITY FOR THE DISCOVERY, HANDLING, REMOVAL OR DISPOSAL OF HAZARDOUS MATERIALS AT THE PROJECT SITE, INCLUDING BUT NOT LIMITED TO
- ASBESTOS, ASBESTOS PRODUCTS, POLYCHLORINATED BIPHENYL (PCB) OR OTHER TOXIC SUBSTANCES. 9. SHOULD ANY CONDITIONS DEVELOP NOT COVERED BY THE CONTRACT DOCUMENTS, OR IF A CHANGE IN THE SCOPE OF WORK IS PROPOSED, A CONSTRUCTION CHANGE DOCUMENT DETAILING AND SPECIFYING THE REQUIRED
- CHANGE(S) SHALL BE SUBMITTED TO AND APPROVED BY DSA BEFORE PROCEEDING WITH THE WORK. 10. THE SCHOOL DISTRICT INSPECTOR ON RECORD SHALL INSPECT AND APPROVE THE ERECTED FRAME PRIOR TO ROOF
- 11. SEE REQUIREMENTS FOR LOCATION IN ANY FIRE HAZARD SEVERITY ZONE FOR WILDLAND URBAN INTERFACE AREAS (WUI) AS SPECIFIED IN THE APPLICABLE VERSION OF THE CALIFORNIA BUILDING CODE. PROVIDE PROTECTION AND DETAILS OF ALL AREAS COMPLYING WITH THE WUI REQUIREMENTS.
- 12. LOCATING THIS STRUCTURE CLOSER THAN 20 FEET TO OTHER STRUCTURES MAY AFFECT THE ALLOWABLE AREA
- FOR THE EXISTING CONSTRUCTION PER THE APPLICABLE VERSION OF THE CALIFORNIA BUILDING CODE. 13. VIEWS AND DETAILS ARE NOT DRAWN TO SCALE (UNLESS NOTED OTHERWISE). DO NOT SCALE THESE DRAWINGS.

### STRUCTURAL AND MISCELLANEOUS STEEL:

- 1. ALL STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE AMERICAN INSTITUE OF STEEL CONSTRUCTION (AISC) SPECIFICATION MANUAL REFERENCED BY THE LATEST EDITION OF THE CALIFORNIA BUILDING CODE.
- 2. PIPE SECTIONS SHALL CONFORM TO ASTM A53, Fy = 35 KSI, GRADE B OR A501 UNLESS NOTED OTHERWISE. 3. STRUCTURAL TUBING (HSS SHAPES) SHALL CONFORM TO ASTM A-500, GRADE B (OR C), Fy = 46 KSI (MIN).
- 4. IF MATERIAL AVAILABILITY IS LIMITED, MEMBER THICKNESS CAN BE INCREASED BEYOND WHAT IS SHOWN IN THESE DRAWINGS (MAXIMUM INCREASE OF 1/8").
- 5. ALL CHANNELS, ANGLES, AND MISC. STEEL SHALL CONFORM TO ASTM A-36, Fy =36 KSI.
- 6. ALL PLATE STEEL SHALL CONFORM TO ASTM A-572, Fy= 50 KSI.
- 7. ALL COLD FORM STEEL SHALL CONFORM TO ASTM A-653, CS = TYPE B, Fy = 50 KSI.
- 8. STRUCTURAL STEEL AND DECK SHALL BE IDENTIFIED FOR CONFORMITY PER CBC 2202A.1.
- 9. ALL ROOF DECKS SHALL HAVE KYNAR 500 METAL COATING.
- 10.ALL ROOF DECKS SHALL CONFORM TO ASTM A-792, Fy = 50 KSI.

### INSTRUCTIONS FOR ARCHITECTS SUBMITTING THESE PRE-CHECKED DRAWING TO DSA: BEFORE SUBMITTING THESE PRE-CHECKED DRAWINGS FOR YOUR PROJECT, FOLLOW THE

STEP 1: SELECT FRAME DIMENSIONS FOR YOUR PROJECT -GABLE STRUCTURES UP TO 20' WIDE USE THE "RG 20" BASE FRAME -GABLE STRUCTURES UP TO 30' WIDE USE THE "RG 30" BASE FRAME

STEPS BELOW TO PROPERLY DEFINE THE APPROVED OPTIONS:

-GABLE STRUCTURES UP TO 40' WIDE USE THE "RG 40" BASE FRAME -MAXIMUM WIDTH IS 40' (SEE "ARCHITECTURAL VIEWS" SHEET FOR REFERENCE) -THE 24', 44', 64', 84' AND 104' LENGTHS ARE SUGGESTED BECAUSE THEY ARE THE MOST COMMON

(20' BAYS ARE THE MOST ECONOMICAL) -FRAME LENGTHS ASSUME 2' OVERHANGS (UNO BY ARCHITECT - 2' MAX DIMENSION)

STEP 2: SELECT ROOF DECK FOR YOUR PROJECT -"M" REPRESENTS McELROY METAL "MULTI-RIB" ROOF PANEL

-"G" REPRESENTS McELROY METAL "MEGA-RIB" ROOF PANEL -"S" REPRESENTS McELROY METAL "MEDALLION-LOK" 16" STANDING SEAM ROOF PANEL

STEP 3: IDENTIFY THE Ss ACCELERATION (q) FOR YOUR PROJECT -Ss VALUE DETERMINES THE REQUIRED SEISMIC DESIGN FORCES

-Ss VALUE DEPENDS ON THE PROJECTS GEOGRAPHICAL LOCATION (VALUES RANGE FROM 0.00 TO 3.73)

STEP 4: IDENTIFY THE Ss REGION FOR YOUR PROJECT -THE REGIONS ARE DEPENDANT ON THE Ss VALUE DETERMINED IN STEP 3

-THE SS REGION DICTATES THE MAXIMUM DEAD LOAD PERMITTED ON THE FRAME (SEE TABLE TO RIGHT) STEP 5: IDENTIFY THE ROOF DEAD LOAD FOR YOUR PROJECT

-THE ROOF DECK DEAD LOAD WILL ALWAYS BE INCLUDED -THE COLLATERAL LOAD REPRESENTS ADDITIONAL LOAD THAT CAN BE SUPPORTED BY THE FRAME -BE SURE THE TOTAL ROOF DEAD LOAD FOR YOUR PROJECT IS LESS THAN OR EQUAL TO THE MAX DEAD LOAD SHOWN IN STEP 4 FOR YOUR SE VALUE

-Sds value used in calculation is the capped Sds (see design criteria) STEP 6: IDENTIFY THE FOUNDATION REQUIREMENTS FOR YOUR PROJECT -IDENTIFY SOIL CLASS FOR PROJECT SITE PER SITE SPECIFIC SOIL CONDITIONS -USE THIS TO SELECT CORRECT FOUNDATION SIZE ON FOUNDATION SHEET

STEP 7: SELECT MISCELLANEOUS OPTIONS FOR YOUR PROJECT -MAXIMUM CLEAR HEIGHT IS 12'-0"; (SEE "ARCHITECTURAL VIEWS" SHEET FOR REFERENCE) -MARK UP PC DRAWINGS WITH SIZE AND LOCATION OF CUTOUTS BEFORE SUBMITTING TO DSA

STEP 8: SELECT APPLICABLE SHEET INDEX FOR YOUR PROJECT -REFERENCE THE BASE FRAME (STEP 1) AND THE ROOF PANEL TYPE (STEP 2) -IDENTIFY THE APPLICABLE SHEÈT INDEX

STEP 9: INCLUDE APPLICABLE SHEETS WITH YOUR DSA SUBMITTAL

-INCLUDE 'MISC DESIGN OPTIONS' SHEET FOR PROJECTS WITHOUT ELECTRICAL CUTOUTS OR GUTTERS

### NOTICE OF DISCLAIMER FOR STRUCTURAL ENGINEERING RESPONSIBILITY

1. PER TITLE 24, PART 1, SECTION 4-316(e) OF THE CALIFORNIA CODE OF REGULATIONS, THIS NOTICE SHALL

4. STRUCTURAL OBSERVATION OF CONSTRUCTION IS SPECIFICALLY EXCLUDED FROM J.R. MILLER & ASSOCIATES'

- BE GIVEN TO DSA PRIOR TO THE APPROVAL OF PLANS AND SPECIFICATIONS. 2. FOR THE SITE SPECIFIC PROJECT, J. R. MILLER & ASSOCIATES IS NOT THE DESIGN PROFESSIONAL IN
- GENERAL RESPONSIBLE CHARGE. 3. FOR THE SITE SPECIFIC PROJECT, J.R. MILLER & ASSOCIATES' RESPONSIBILITY IS LIMITED TO THE PREPARATION OF THE PLANS AND SPECIFICATIONS FOR THE SHELTERS OF THIS PC ONLY.
- RESPONSIBILITY FOR THE SITE SPECIFIC PROJECT. 5. ALL CONSTRUCTION ACTIVITIES RELATED TO STRUCTURAL ENGINEERING SHALL BE DELEGATED TO A QUALIFIED ENGINEER BY THE DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE CHARGE. THESE ACTIVITIES INCLUDE. BUT ARE NOT LIMITED TO, APPROVAL OF INSPECTOR QUALIFICATIONS, STRUCTURAL OBSERVATION OF
- COMPLETED WORK. 6. J.R. MILLER & ASSOCIATES WILL BE RESPONSIBLE FOR RESPONDING TO QUESTIONS PERTAINING TO THE PLANS AND SPECIFICATIONS FOR THE SHELTERS OF THIS PC WHICH ARISE DURING PLAN REVIEW AND CONSTRUCTION.

CONSTRUCTION, REVIEW OF INSPECTION REPORTS, AND SIGNING OFF OF THE VERIFIED REPORT FOR

- 1. ALL WELDING SHALL COMPLY WITH AWS D1.1 SPECIFICATIONS AND SHALL BE DONE BY AWS QUALIFIED WELDERS
- CERTIFIED FOR THE TYPE OF WELDING TO BE PERFORMED AS REQUIRED BY DSA. 2. ALL WELDING SHALL BE DONE BY GAS METAL ARC PROCESS WITH E70XX ELECTRODES. FLUX CORE ARC WELD SHALL CONFORM TO CHARPY NOTCH TOUGHNESS RATING OF 20 ft-16 @ ( 0° F).
- 3. ALL WELDING SHALL BE DONE IN THE SHOP WITH REQUIRED INSPECTION, PRE-APPROVED BY DSA, TO ENSURE PROPER MATERIAL ID AND WELDING.
- 4. WELD FILLER METAL MANUFACTURER SHALL PROVIDE WRITTEN CERTIFICATION OF COMPLIANCE WITH CODE AND SPECIFIC ATIONS.

- 1. ALL BOLTS SHOWN ON THESE DRAWINGS ARE ASTM F3125 GRADE A325 HIGH STRENGTH BOLTS (UNO), WITH THE NUTS CONFORMING TO ASTM A-563
- 2. HIGH STRENGTH BOLTS SHALL BE VERIFIED AND INSPECTED PER CBC 1705A2.1
- 3. BEFORE ERECTING THE FRAME, VERIFY ALL BOLTS AND NUTS ARE CLEAN OF DEBRIS AND BURRS INCLUDING THE HARDWARE ALREADY FASTENED INSIDE THE MEMBERS. CHASING SOME OF THE BOLTS AND NUTS MAY BE
- 4. HARDENED STEEL WASHERS SHALL CONFORM TO ASTM F-436.
- 5. THE BOLTING INSTALLATION REQUIREMENTS OUTLINED BELOW ARE CRITICAL TO THE STRUCTURE'S DESIGN AND PERFORMANCE. THE INSTALLER IS REQUIRED TO COORDINATE THIS PHASE OF CONSTRUCTION WITH THE SPECIAL BOLTING INSPECTOR AND THE INSPECTOR OF RECORD PRIOR TO THE ERECTION OF THE FRAME. ALL BOLTS SHALL BE INSTALLED AND INSPECTED PER THE APPLICABLE VERSION OF AISC'S "SPECIFICATION FOR STRUCTURAL JOINTS
- USING HIGH-STRENGTH BOLTS", CBC 1705A.2.1; AISC 341-16 J7; AISC 360-16 N5.6. A)PRETENSIONED JOINTS MUST BE INSTALLED AND INSPECTED TO MEET ONE OF THE FOLLOWING REQUIREMENTS:
  - 1. TURN-OF-NUT PRETENSIONING
  - 2. CALIBRATED WRENCH PRETENSIONING 3. DIRECT-TENSION-INDICATOR PRETENSIONING (CONTRACTOR RESPONSIBLE FOR PURCHASE OF

- 1. ALLOWABLE SOIL PRESSURES ASSUME CLASS 5 SOIL CLASSIFICATION PER CBC TABLE 1806A, UNLESS NOTED
- 2. PER CBC SECTION 1803A.2, GEOTECHNICAL REPORTS ARE NOT REQUIRED FOR ONE-STORY LIGHT-STEEL FRAME BUILDINGS OF TYPE II CONSTRUCTION AND 4,000 SQUARE FOOT OR LESS IN FLOOR AREA AND NOT LOCATED WITHIN EARTHQUAKE FAULT ZONESOR SIESMIC HAZARD ZONES AS SHOWN ON THE MOST RECENT MAPS PUBLISHED BY THE CGS. ALLOWABLE FOUNDATION AND LATERAL SOIL PRESSURE VALUES MAY BE DETERMINED FROM TABLE 1806A.2.
- 3. FILL AND BACKFILL SHALL BE COMPACTED TO 95% OF MAX. DENSITY IN ACCORDANCE WITH ASTM TEST METHOD D-1557 OR AS RECOMMENDED BY THE GEO-TECH ENGINEER. FLOODING NOT PERMITTED.
- 4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SHORING, ETC. NECESSARY TO SUPPORT CUT AND/OR FILL BANKS DURING EXCAVATION, AND FORMING AND PLACEMENT OF CONCRETE.
- 5. MINIMUM SETBACK FROM TOE OF SLOPE ON AN ASCENDING SLOPE SHALL BE 15 FEET AND MINIMUM SETBACK FROM TOE OF SLOPE ON A DESCENDING SLOPE SHALL BE 40 FEET
- 6. PER CBC SECTION 1803A.6, GEOHAZARD REPORTS ARE NOT REQUIRED FOR ONE-STORY LIGHT-STEEL FRAME BUILDINGS OF TYPE II CONSTRUCTION AND 4,000 SQUARE FOOT OR LESS IN FLOOR AREA AND NOT LOCATED WITHIN EARTHQUAKE FAULT ZONESOR SIESMIC HAZARD ZONES AS SHOWN ON THE MOST RECENT MAPS PUBLISHED BY THE CGS.
- 7. GEOHAZRD REPORTS ARE TO COMPLY WITH DSA IR A-4 PER IR-7 SECTION 1.8
- 8. SITE SPECIFIC GEOTECHNICAL REPORT IS REQUIRED AT THE TIME OF SITE APPLICATION IS USING OTHER THAN
- 9. LATERAL BEARING HAS BEEN INCREASED PER CBC 1806A.3.4 & HAS BEEN DESIGNED FOR P-DELTA EFFECTS

### 1. MIX DESIGN REQUIREMENTS: (NORMAL WEIGHT CONCRETE)

STRENGTH Pc (28 DAYS)	W/C RATIO (NON—AIR ENTRAINED)	W/C RATIO (AIR ENTRAINED)	SLUMP (±1")	UNIT WEIGHT (NORMAL WEIGHT)
4500 PSI	0.44	0.35	3"	150 PCF

SCHOOL DISTRICT:

- ENTRAINMENT FOR THESE CATEGORIES SHALL BE AS FOLLOWS: F0-0, F1-4.5, F2-6 3. AGGREGATES SHALL CONFORM TO THE ASTM C-33 WITH PROVEN SHRINKAGE CHARACTERISTICS OF LESS THAN 0.005.
- MAX AGGREGATE SIZE = 1".
- 4. CEMENT SHALL CONFORM TO ASTM C-150 (TYPE V) UNLESS NOTED OTHERWISE ON THE DRAWINGS. 5. CONCRETE SHALL BE MAINTAINED IN A MOIST CONDITION FOR A MINIMUM OF FIVE DAYS AFTER PLACEMENT.
- ALTERNATE METHODS WILL BE APPROVED IF SATISFACTORY PERFORMANCE CAN BE ASSURED. 6. CONCRETE SHALL NOT FREE FALL MORE THAN FIVE FEET.
- 7. CONCRETE DURABILITY SHALL BE PER CBC 1904A.1 & ACI 318-14 CHAPTER 19.
- 8. CONCRETE SHALL BE TESTED PER CBC 1903A, TABLE 1705A.3. AND ACI 318-14 SECTION 26.12.

### STEP 10: IDENTIFY PROJECT NAME AND SCHOOL DISTRICT

PROJECT NAME:

			FRAME	DIMENSION	 S	
_			SUG	GESTED		OTHER
STEF	FRAME WIDTH	[] 20'	<b>X</b> 30'	[] 40'		[ ] (40' MAX)
	FRAME LENGTH	[] 44'	<b>⋈</b> 64'	[ ] 84'	Γ ] 104 <sup>'</sup>	[] (NO MAX)

7		ROOF PANEL	
STEP	ROOF PANEL TYPE	<b>⋈</b> м [] G [] S	
3 E		PROJECT SITE — Ss ACCELERATION (g)	
ST		0.642	

	Ss REGION		
		Ss REGIONS	MAX DEAD LOAD
	Х	0 < Ss <= 2.14	5 PSF
		2.14 < Ss <= 2.50	5 PSF
DESCRIPTION		2.50 < Ss <= 2.75	5 PSF
		2.75 < Ss <= 3.00	4 PSF
		Ss > 3.73 MAX	3 PSF
	DESCRIPTION	X	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$

		TOTAL ROOF DEAD LOA	AD
		DEAD LOAD	EXAMPLES
٦. 5	ROOF DECK	<u>1.1</u> PSF	M=1.1PSF; G=1.2PSF;S=1.3PSF (SEE STEP 2)
STE	COLLATERAL	<u>0</u> PSF	LIGHTING, ETC
	TOTAL	<u>1.1</u> PSF	ADD ROOF DECK AND COLLATERAL LOADS (MAX 5 PSF)

1. REINFORCING STEEL SHALL BE DEFORMED STEEL CONFORMING TO THE REQUIREMENTS OF ASTM A-615,

GR 60: (#4 BARS AND LARGER)

GR 40: (#3 BARS)

- DETAILING, FABRICATION, AND ERECTION OF REINFORCING BARS SHALL CONFORM TO THE ACL "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCING CONCRETE STRUCTURES."
- 3. MIN. COVER FOR CAST-IN-PLACE CONCRETE SHALL BE AS FOLLOWS:
- A. CAST AGAINST EARTH ..... B. CAST AGAINST FORM BELOW GRADE .....2"
- C. FORMED SLABS (#11 BAR & SMALLER)......3/4" D. SLABS ON GRADE (FROM TOP OF SLAB)......1"

MINIMUM

MISC ELLANEOUS

- 4. BARS SHALL BE CLEAN OF RUST, GREASE OR OTHER MATERIAL LIKELY TO IMPAIR BOND.
- BENDS SHALL BE MADE COLD. REINFORCING SHALL BE LAP SPLICED PER ACI 318-14 SECTION 25.5.
- 6. PRIOR TO PLACING OF CONCRETE, REINFORCING STEEL AND EMBEDDED ITEMS SHALL BE WELL SECURED IN POSITION.
- 7. WELDING OF REINFORCING IS NOT ALLOWED. 8. REINFORCING STEEL SHALL BE INSPECTED PER CBC 1705A.3.

### POWDER-COAT FINISH SYSTEM:

REINFORCING STEEL:

- ALL BUILDINGS THAT HAVE A POWDER-COATED FINISH SHALL MEET THE FOLLOWING SPECIFICATIONS:
- 1. THE STEEL FRAME SHALL BE SHOT-BLASTED TO A NEAR WHITE CONDITION PER SSPC-10 SPECIFICATIONS. 2. THE STEEL SHALL BE WASHED IN A ZINC PHOSPHATE IN AN MINIMUM EIGHT STAGE ELECTRO DEPOSITION
- 3. IMMEDIATELY FOLLOWING PRE-TREATMENT THE STEEL SHALL BE TOTALLY IMMERSED IN A LIQUID EPOXY
- PRIMER(E-COAT) AND COATED TO A UNIFORM THICKNESS OF A MINIMUM OF 0.7 TO 0.9 MILS. THE E-COATING SHALL PROVIDE A MINIMUM OF 1000 HOURS OF SALT SPRAY CORROSION PROTECTION TO THE STEEL.
- 4. THE STEEL SHALL THEN HAVE A TGIC POLYESTER COLOR COAT APPLIED OVER THE E-COATED SURFACE.
- 5. THE COLOR COAT SHALL THEN HAVE A CLEAR TGIC COATING APPLIED TO SEAL IN THE COLOR COAT AND RESIST ULTRAVIOLET LIGHT, TO HELP PREVENT FADING.
- 6. THE FINISH THICKNESS OF THESE THREE APPLICATIONS SHALL BE A MINIMUM OF 8 TO 12 MILS
- 7. ALL CARBON STEEL MEMBERS (COLUMNS, BEAMS, PLATES, ETC.) NOT POWDER-COATED SHALL BE PAINTED WITH PRIME COAT PER THE "AISC CODE OF STANDARD PRACTICE" AND THE "AISC SPECIFICATION SECTION M3" (UNLESS NOTED

o menuse).	
ABBREVIATIONS:	

ABBREV	ATIONS:		
ACI	AMERICAN CONCRETE INSTITUTE	MPH	MILES PER HOUR
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION	М	MULTI-RIB ROOF PANEL (MCELROY)
ASM	ASSEMBLY (INTERNAL REFERENCE)	NTS	NOT TO SCALE
ASTM	AMERICAN SOCIETY FOR TESTING AND MAT'LS	NO	NUMBER
AWS	AMERICAN WELDING SOCIETY	ОС	ON CENTER
CBC	CALIFORNIA BUILDING CODE	OSHA	OCCUPATIONAL HEALTH AND SAFETY ADMIN
CJP	COMPLETE JOINT PENETRATION	PCF	POUNDS PER CUBIC FOOT
CLR	CLEAR	PJ	PRETENSIONED JOINT
DEG	DEGREE	PLCS	PLACES
DIA	DIAMETER	PLT	PLATE
DIM	DIMENSION	PSF	POUNDS PER SQUARE FOOT
DSA	DIVISION OF THE STATE ARCHITECT	PSI	POUNDS PER SQUARE INCH
EQ	EQUAL	QTY	QUANTITY
FT	FEET	REF	REFERENCE
GA	GAGE	SQ	SQUARE
IN	INCHES	SS	STANDING SEAM ROOF PANEL (MCELROY)
KSI	KIPS PER SQUARE INCH	TYP	TYPIC AL
MAX	MAXIMUM	UNO	UNLESS NOTED OTHERWISE

U.S. GEOLOGIC AL SURVEY

DIV. OF THE STATE ARCHITECT APP: 04-120013 PC REVIEWED FOR SS 🗹 FLS 🗹 ACS 🗹 CG 🗌 DATE: 08/06/2021

DRAWN BY

DATE

REV

REV DATE

ARCHITECTS ENGINEERS

2700 SATURN STIBREA, CA 92821

. 714.524.1870 | F. 714.524.1875

**ANGEL** 

4/2/202

FOUNDATION REQUIREMENTS SOIL CLASS 5 (BEARING)-1500 PSF 🔀 SOIL CLASS 3 (BEARING)-3000 PSF [ SOIL CLASS 4 (BEARING)-2000 PSF [] SOIL CLASS 5 (LATERAL BEARING)-100 PSF | SOIL CLASS 4 (LATERAL BEARING)-150 PSF | SOIL CLASS 3 (LATERAL BEARING)-200 PSF

SHEET INDEX

	MIS	SC ELLANEOUS	
		DESIGN	N OPTIONS
	CLEAR HEIGHT	[]8' 🔀 10' []12'	[ ] ' (12' MAX)
S	ELECTRICAL CUTOUTS	<b>⋈</b> YES	[ ] NO
	GUTTERS	<b>⋈</b> YES	[ ] NO

	BASE FRAME		RG 20			RG 30			RG 40	
	ROOF PANEL TYPE	М	G	S	М	G	S	М	G	S
	SELECT ONE	[]	[]	[]	[]	[]	[X]	[]	[]	[]
	GENERAL NOTES	LS1.0								
	DSA 103 EXAMPLE	LS1.1								
ω	FOUNDATION PLAN	LS2.0	LS2.0	LS2.0	LS3.0	LS3.0	LS3.0	LS4.0	LS4.0	LS4.0
STEP	FRAMING PLAN	LS2.1	LS2.1	LS2.1	LS3.1	LS3.1	LS3.1	LS4.1	LS4.1	LS4.1
	FRAME CONNECTION DETAILS	LS2.1	LS2.1	LS2.1	LS3.1	LS3.1	LS3.1	LS4.2	LS4.2	LS4.2
	ROOFING LAYOUT & DETAILS	LS2.2	LS2.3	LS2.4	LS3.2	LS3.3	LS3.4	LS4.3	LS4.4	LS4.5
	MISC DESIGN OPTIONS	LS5.0								

ISTINCTIVE STEEL SHELTERS

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616.396.0919

800.748.0985

616.396.0944 FX

PRE-CHECK (PC) DOCUMENT Code: 2019 CBC

PRINTED ON:

### CONSTRUCTION NOTES

SHALL COMPLY WITH ALL LOCAL ORDINANCES

- 1. A DSA-CERTIFIED CLASS 3 PROJECT INSPECTOR IS REQUIRED FOR THIS PROJECT.
- 2. CHANGES TO THE APPROVED DRAWINGS AND SPECIFICATIONS SHALL BE MADE BY ADDENDA OR CONSTRUCTION CHANGE DOCUMENT (CCD) APPROVED BY DSA, AS REQUIRED BY SECTION 4-338, PART 1, TITLE 24, CCR.
- $\,$  3. A "DSA CERTIFIED" PROJECT INSPECTOR EMPLOYED BY THE DISTRICT (OWNER) AND APPROVED BY DSA SHALL PROVIDE
- CONTINUOUS INSPECTION OF WORK, THE DUTIES OF THE INSPECTOR ARE DEFINED IN SECTION 4-342, PART 1, TITLE 24, CCR. 4. A DSA ACCEPTED TESTING LABORATORY DIRECTLY EMPLOYED BY THE DISTRICT (OWNER) SHALL CONDUCT ALL THE REQUIRED
- TESTS AND INSPECTIONS FOR THE PROJECT. 5. THE INTENT OF THESE DRAWINGS AND SPECIFICATIONS ARE THAT ALL THE WORK OF THE ALTERATION, REHABILITATION OR RECONSTRUCTION IS TO BE IN ACCORDANCE WITH TITLE 24, CCR. SHOULD ANY EXISTING CONDITIONS SUCH AS DETERIORATION OR NON-COMPLYING CONSTRUCTION BE DISCOVERED WHICH IS NOT COVERED BY THE CONTRACT DOCUMENTS WHEREIN THE
- FINISHED WORK WILL NOT COMPLY WITH TITLE 24, CCR, A CONSTRUCTION CHANGE DOCUMENT (CCD), OR A SEPARATE SET OF PLANS AND SPECIFICATIONS, DETAILING AND SPECIFYING THE REQUIRED WORK SHALL BE SUBMITTED TO AND APPROVED BY DSA BEFORE PROCEEDING WITH THE WORK, (SECTION 4-317(c), PART 1, TITLE 24, CCR) 6. GRADING PLANS, DRAINAGE IMPROVEMENTS, ROAD AND ACCESS REQUIREMENTS AND ENVIRONMENTAL HEALTH CONSIDERATIONS

A separate project application for construction is required.

<b>SA 103-19: LISTING OF</b>	STRUCTURAL TESTS & SPECIAL IN	SPECTIONS, 2019 CBC			S15 & SPECIAL IN	ISPECTIO	NS (SOILS), 2019 CBC	DSA 103-19: LIST Application Number:	School Name:	J & JI LOIAL INJ	School District:	DSA 1 Applicat
pplication Number: 4-000000	School Name: ICON Shelter Systems	School District: PC Submittal	Application Number: 04-000000 DSA File Number:	School Name: ICON Shelter Systems Increment Number:			School District: PC Submittal Date Created:	04-000000 DSA File Number:	ICON Shelter Systems Increment Number:		PC Submittal  Date Created:	Applica 04-0000 DSA File
A File Number:	Increment Number:	Date Created: 2021-07-14 05:50:33					2021-07-14 05:50:33				2021-07-14 05:50:33	
												5 T
	2010	ono.	Geotechnical Reports:	: Project has a geotechnic	cal report, or CDs in	ndicate soil	s special inspection is required by GE	C. Compaction tes	ting.	Test L(	* Under the supervision of the geotechnical engineer. (Refer to specific items identified in the Appendix for exemptions when	re 🗆 a
IMPORTANT: This fo	<b>2019</b> orm is only a summary list of structural test:	CBC and some of the special inspections required for the project.	1. GENERAL:	· ·	Table 1705A.6						soils testing may be conducted under the supervision of a geotechnical engineer or LOR's engineering manager. In such cases, the LOR's form 291 shall satisfy the soil test reporting requirements for the exempt ite	DSA L
Generally, the structural te	ests and special inspections noted on this f	orm are those that will be performed by the Geotechnical Engineer uplete test and inspection program must be performed as detailed	Test or Special Inspe	ection	Type Pe By	erformed C y	ode References and Notes				251 shall satisfy the soil test reporting requirements for the exempt te	□ <b>b</b>
on the DSA approved doci	cuments. The appendix at the bottom of thi	of form identifies work NOT subject to DSA requirements for special for providing inspection of all facets of construction, including but		ared properly prior to placement of	Periodic of		By geotechnical engineer or his or her qualified representative. See Appendix for exemptions.)	4. CAST-IN-PLACE	E DEEP FOUNDATIONS (PIERS):	Table 1705A.8		□ C.
not limited to, special inspe	ections not listed on this form such as struc	tural wood framing, high-load wood diaphragms, cold-formed steel etc., per Title 24, Part 2, Chapter 17A (2019 CBC).	Foundation excavation	excavations for foundations. tions are extended to proper deptl	th			Test or Special In	spection	Type Perfo	rmed   Code References and Notes	d
_		is document are from the CBC, or California Building Code.	and have reached pro  • Materials below foot design bearing capaci	tings are adequate to achieve the				✓ a. Inspect drilling of and accurate record	operations and maintain complete	Continuous G	E*  * By geotechnical engineer or his or her qualified representative.  (See Appendix for exemptions.)	e
TO COLUMNS	ica section and table references round in th	is document are from the CDC, of Camornia building Code.		·				□ <b>b.</b> Verify pier locati	ions, diameters, plumbness, bell cable), lengths and embedment into	Continuous G	E* * By geotechnical engineer or his or her qualified representative.  (See Appendix for exemptions.)	6
1. TYPE		2. PERFORMED BY	2, SOIL COMPACTION Test or Special Inspe		Table 1705A.6	erformed C	ode References and Notes		ible); record concrete or grout		(See Appendix for exemptions.)	
ntinuous – Indicates that a cont	ntinuous special inspection is	GE – Indicates that the special inspection shall be performed by a registered geotechnical engineer or his or her authorized representative.	□ a. Perform classification	ion and testing of fill materials.	By	LOR* *	Under the supervision of the geotechnical engineer.	C. Confirm adequa	te end strata bearing capacity.	Continuous G	E* * By geotechnical engineer or his or her qualified representative. (See Appendix for exemptions.)	
uired		COR – Indicates that the test or special inspection shall be performed by a testing	b. Verify use of proper	er materials, densities and	Continuous	GE* *	By geotechnical engineer or his or her qualified representative. (Refer to be decific items identified in the Appendix for exemptions where soils SI and	d. Concrete piers.		Provide tests and insp	pections per CONCRETE section below.	b
iodic – Indicates that a periodic	ic special inspection is required	laboratory accepted in the DSA Laboratory Evaluation and Acceptance (LEA) Program. See CAC Section 4-335.	during placement of f	es, placement and compaction fill.		te	sting may be conducted under the supervision of a geotechnical ngineer or LOR's engineering manager. In such cases, the LOR's form DSA					□ c.
		PI – Indicates that the special inspection may be performed by a project inspector when specifically approved by DSA.				29 ite	21 shall satisfy the soil SI and test reporting requirements for the exempt ems.)					
t – Indicates that a test is require	ired	SI – Indicates that the special inspection shall be performed by an appropriately						DGS DSA 103-19 (Revised 07.	/16/2020)			DGS DSA
		qualified/approved special inspector.						DIVISION OF THE STATE ARC	HITECT	DEPARTMENT OF GEN Page 3 o		DRNIA DIVISION
OSA 103-19 (Revised 07/16/2020)			DGS DSA 103-19 (Revised 07/16/	,	DEPARTMENT OF C	GENERAL SERVI	CES STATE OF CALIFORNIA					
ON OF THE STATE ARCHITECT	DEPARTMENT OF GI Page 1					2 of 11						
					ESTS & SPECIAL I	INSPECTI	ONS (Concrete), 2019 CBC		TING OF STRUCTURAL TEST -14 Sections 26.12 & 26.13	TS & SPECIAL INS	PECTIONS (Concrete), 2019 CBC	<b>DSA 10</b> Table 170
cation Number:	STRUCTURAL TESTS & SPECIAL IN School Name: ICON Shelter Systems	SPECTIONS (SOILS), 2019 CBC  School District: PC Submittal	Table 1705A.3; ACI 318-7 Application Number: 04-000000	-14 Sections 26.12 & 26.13 School Name: ICON Shelter System:	ns.		School District: PC Submittal	Application Number: 04-000000 DSA File Number:	School Name: ICON Shelter Systems Increment Number:		School District: PC Submittal Date Created:	Applicati 04-00000 DSA File I
ile Number:	Increment Number:	Date Created: 2021-07-14 05:50:33	DSA File Number:	Increment Number:			Date Created: 2021-07-14 05:50:33	DSA FIJE NUMBER:	mcrement wumber:		2021-07-14 05:50:33	
7. CAST-IN-PLACE CONCRE			17. STRUCTURAL  Material Verification and	L STEEL, COLD-FORMED STEEL AN	ND ALUMINUM USED FO	OR STRUCTU	RAL PURPO	☑ b. Test high-stren	ngth bolts, nuts and washers.	Test I	OR Table 1705A.2.1 Item 1c, 2213A.1; RCSC 2014 Section 7.2; DSA IR 17-	
Test or Special Inspection	Ву	rformed Code References and Notes	Test or Special Ins		Туре	Performed	Code References and Notes	Inspection of High-Stre				Te:
rial Verification and Testing: a. Verify use of required desig		SI Table 1705 A.3 Item 5, 1910 A.1.		ation of all materials and:	Periodic		Table 1705A.2.1 Item 3a–3c. 2202A.1; AISI S100-16 Section A3.1 & A3.2, AISI S240-15 Section A3 & A5, AISI S220-15 Sections A4 & A6. * By special		'snug tight") connections.	Periodic	SI Table 1705A.2.1 Item 2a, 1705A.2.6, 2204A.2; AISC 360-16 J3.1, J3.2 M2.5 & N5.6; RCSC 2014 Section 9.1; DSA IR 17-9.	fille
b. Identifiy, sample, and test r	reinforcing steel. Test	LOR 1910A.2; ACI 318-14 Section 26.6.1.2; DSA IR 17-10. (See Appendix for exemptions.)	with requirements  • Material sizes, typ		, mpi		inspector or qualified technician when performed off-site.	d. Pretensioned a	and slip-critical connections.	*	SI Table 1705A.2.1 Items 2b & 2c, 1705A.2.6, 2204A.2; AISC 360-16 J3 J3.2, M2.5 & N5.6; RCSC 2014 Sections 9.2 & 9.3; DSA IR 17-9. * "Continuous" or "Periodic" depends on the tightening method used.	.1, de
<b>c.</b> During concrete placement for strength tests, perform slu	nt, fabricate specimens Test  Jump and air content	LOR Table 1705A.3 Item 6; ACI 318-14 Sections 26.5 & 26.12.	requirements.  D. Test unidentified	ed materials	Test	LOR	2202A.1.					
tests, and determine the temp concrete.			c. Examine seam w	velds of HSS shapes	Periodic	SI	DSA IR 17-3.	19. WELDI		D1.2 for Aluminum;	705A.2.1 Items 4 & 5; AWS D1.1 and AWS D1.8 for structural steel; AWS AWS D1.3 for cold-formed steel; AWS D1.4 for reinforcing steel; DSA IR 17-	□ d. v
<b>d</b> . Test concrete (f'c).	Test	LOR 1905A.1.15; ACI 318-14 Section 26.12.	<u> </u>	ıment steel fabrication per DSA-app uments.	proved Periodic		Not applicable to cold-formed steel light-frame construction, except for russes (1705A.2.4).	   Verification of Materia	ls, Equipment, Welders, etc.:	3 (See Appendix for	exemptions.)	e.i
e. Batch plant inspection:	See Notes	SI Default of 'Continuous' per 1705A.3.3. If approved by DSA, batch plant inspection may be reduced to 'Periodic' subject to requirements in		GTH BOLTS: RCSC 2		l'	NOSES (TOOKILITY).	Test or Special II	nspection	Type Perf By	ormed Code References and Notes	23.
		Section 1705A.3.3.1, or eliminated per 1705A.3.3.2. (See Appendix for exemptions.)		nd Testing of High-Strength Bolts	s, Nuts and Washers:				er material identification markings per listed on the DSA-approved document:		SI DSA IR 17-3.	Te
f. Welding of reinforcing steel	el. Provide special i	rspection per STEEL, Category 19.1(d) & (e) and/or 19.2(g) & (h) below.	Test or Special Ins	·	, i	Ву	Code References and Notes		er material manufacturer's certificate of	Periodic	SI DSA IR 17-3.	✓ a./
			certificates of com	ation markings and manufacturer's npliance conform to ASTM standard ISA-approved documents.			Table 1705A.2.1 Items 1a & 1b, 2202A.1; AISC 360-16 Section A3.3, J3.1, and N3.2; RCSC 2014 Section 1.5 & 2.1; DSA IR 17-8 & DSA IR 17-9.	c. Verify WPS, we	lder qualifications and equipment.	Periodic	SI DSA IR 17-3.	□ b.
												DGS DSA 10
SA 103-19 (Revised 07/16/2020)			DGS DSA 103-19 (Revised 07,	•	DEPARTMENT O	DE GENERAL SEE	VICES STATE OF CALIFORNIA	DGS DSA 103-19 (Revised 0 DIVISION OF THE STATE ARG	·	DEPARTMENT OF GE	NERAL SERVICES STATE OF CALIF	DIVISION O
N OF THE STATE ARCHITECT	DEPARTMENT OF C Page		NIA DIVISION OF THE STATE ANCI	nitect		ge 6 of 11	VICES STATE OF CALIFORNIA			Page 7 o	of 11	
		SPECTIONS (Steel and Aluminum), 2019 CBC	DSA 103-19: LIST Application Number:	TING OF STRUCTURAL T School Name:	TESTS & SPECIAL	INSPECT	ONS(SIGNATURE), 2019 CBC School District:	DSA 103-19: LIST Application Number: 04-000000	OF REQUIRED VERIFIED RE School Name: ICON Shelter Systems	PORTS, CBC 2019	School District: PC Submittal	
tion Number:	103-16, AISC 341-16, AISC 358-16, AISC 360-16; School Name: ICON Shelter Systems	AISI S100-16 School District: PC Submittal	04-000000 DSA File Number:	ICON Shelter System Increment Number			PC Submittal  Date Created: 2021-07-14 05:50:33	DSA File Number:	Increment Number:		Date Created: 2021-07-14 05:50:33	
	Increment Number:	Date Created: 2021-07-14 05:50:33										
								1				
			Name of Architect or Enginee	eer in general responsible charge:				1. Soils Testing and	d Inspection: Geotechnical Verified R	eport Form DSA 293		_
3. ANCHOR BOLTS AND ANG	ICHOR RODS:		Name of Structural Engineer	r (When structural design has been deleg	gated):				ng and Inspection: Laboratory Verified	·		_
est or Special Inspection		rformed Code References and Notes						<sup>3.</sup> DSA 292			independently contracting SI, Special Inspection Verified Report Form	
. Anchor Bolts and Anchor Ro	ods Test	LOR Sample and test anchor bolts and anchor rods not readily identifiable p procedures noted in DSA IR 17-11.	er Signature of Architect or Stru	uctural Engineer:	Date:			High-Strength B 4. Report Form DS		ry Verified Report Form	DSA 291, or, for independently contracting SI, Special Inspection Verified	i 
o. Threaded rod not used for fo	foundation anchorage. Test	LOR Sample and test threaded rods not readily identifiable per procedures noted in DSA IR 17-11.	Note: To facilitate DS	SA electronic mark-ups and idoas	ication stamp application	n. DSA recom	mends against using secured electronic or digital signatures.					
			NOTE. 10 IACIIITATE DS	and Identifi	жилон эсангр application	יי וecom	DSA STAMP	]				
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A 103-19 (Revised 07/16/2020) N OF THE STATE ARCHITECT	DEPARTMENT OF C	ENERAL SERVICES STATE OF CALIFO	DIVISION OF THE STATE ARC		DEPARTMENT C	OF GENERAL SE age 10 of 11	RVICES STATE OF CALIFORNIA		HITECT	DEPARTMENT OF GENE Page 11 o		NIA

Application Number: 04-000000 ICON Shelter Systems Increment Number: DSA File Number: Date Created: 5. RETAINING WALLS: Type Performed Code References and Notes Test or Special Inspection Continuous GE\* 1705A.6.1. \* By geotechnical engineer or his or her qualified representative. (See Section 2 above). a. Placement, compaction and inspection of backfill. □ b. Placement of soil reinforcement and/or drainage Continuous | GE\* | \* By geotechnical engineer or his or her qualified representative Continuous

# By geotechnical engineer or his or her qualified representative See DSA IR 16-3. c. Segmental retaining walls; inspect placement of units, dowels, connectors, etc. d. Concrete retaining walls. Provide tests and inspections per CONCRETE section below. e. Masonry retaining walls. Provide tests and inspections per MASONRY section below. 6. OTHER SOIL Test or Special Inspection Type Performed Code References and Notes a. Soil Improvements Test GE\* Submit a comprehensive report documenting final soil improvements constructed, construction observation and the results of the confirmation testing and analysis to CGS for final acceptance. \* By geotechnical engineer or his or her qualified representative b. Inspection of Soil Improvements Continuous GE\* \* By geotechnical engineer or his or her qualified representative DGS DSA 103-19 (Revised 07/16/2020) INIA DIVISION OF THE STATE ARCHITECT DEPARTMENT OF GENERAL SERVICES STATE OF CALIFORNIA Page 4 of 11 DSA 103-19: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (Concrete), 2019 CBC Table 1705A.3; ACI 318-14 Sections 26.12 & 26.13
Application Number: School Name: PC Submittal ICON Shelter Systems Date Created: 2021-07-14 05:50:33 DSA File Number: Increment Number: 19.1 SHOP WELDING: Type Performed Code References and Notes Test or Special Inspection a. Inspect groove welds, multi-pass fillet welds, single pass | Continuous fillet welds > 5/16", plug and slot welds. applicable); DSA IR 17-3. ✓ b. Inspect single-pass fillet welds ≤ 5/16", floor and roof
 Periodic
 SI
 1705A.2.2, Table 1705A.2.1 Items 5a.5 & 5a.6; AISC 360-16 (and AISC) 341-16 as applicable); DSA IR 17-3. deck welds. c. Inspect welding of stairs and railing systems. Periodic SI 1705A.2.1; AISC 360-16 (and AISC 341-16 as applicable); AWS D1.1 & D1.3; d. Verification of reinforcing steel weldability Periodic SI 1705A.3.1; AWS D1.4; DSA IR 17-3. Verify carbon equivalent reported on other than ASTM A706. Continuous SI Table 1705A.2.1 Item 5b, 1705A.3.1, Table 1705A.3 Item 2, 1903A.8; AWS D1.4; DSA IR 17-3. e. Inspect welding of reinforcing steel. 23. ANCHOR BOLTS AND ANCHOR RODS: Type Performed Code References and Notes Test or Special Inspection ☑ a. Anchor Bolts and Anchor Rods LOR Sample and test anchor bolts and anchor rods not readily identifiable per procedures noted in DSA IR 17-11. LOR Sample and test threaded rods not readily identifiable per procedures noted in DSA IR 17-11. **b.** Threaded rod not used for foundation anchorage.

DEPARTMENT OF GENERAL SERVICES

STATE OF CALIFORNIA

DSA 103-19: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (SOILS), 2019 CBC

DIV. OF THE STATE ARCHITECT APP: 04-120013 PC REVIEWED FOR SS 🗹 FLS 🗹 ACS 🗹 CG 🗌 DATE: 08/06/2021

RH/DSA-PC

ANGEL

4/2/2021

ARCHITECTS ENGINEERS

2700 SATURN ST I BREA, CA 92821

T. 714.524.1870 | F. 714.524.1875 WWW.JRMA.COM

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DRAWN BY

DATE

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REV DATE

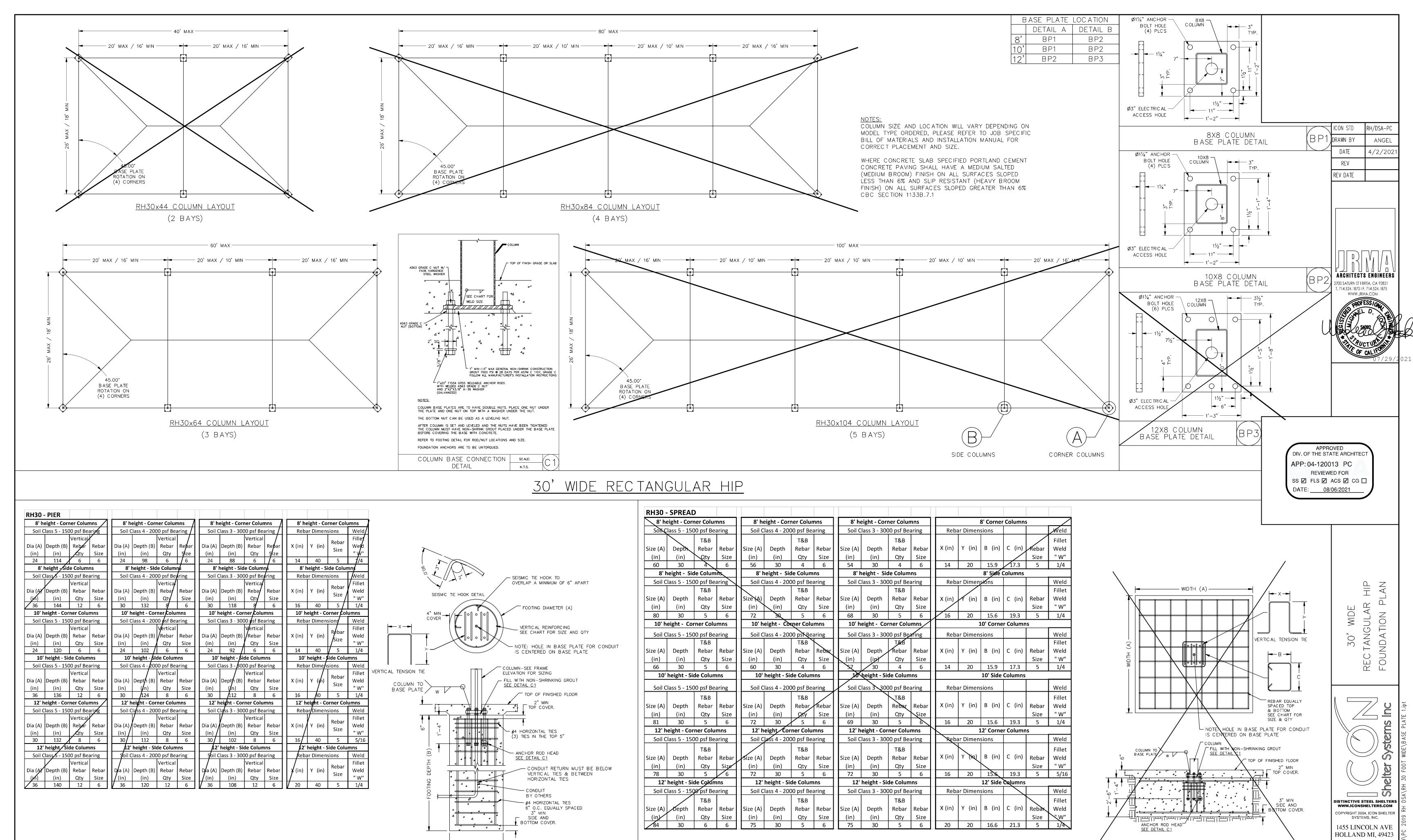
103 DSA

DISTINCTIVE STEEL SHELTERS COPYRIGHT 2004, ICON SHELTER 1455 LINCOLN AVE

HOLLAND MI, 49423

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PRE-CHECK (PC) DOCUMENT Code: 2019 CBC A separate project application for construction is required.



FOOTING DIAMETER (A)

SEE DETAILS BP1. BP2 OR BP3 FOR ANCHOR BOLT PATTERNS

BP1 & BP2 ARE (4) BOLT PATTERN WHILE B3 IS A (6) BOLT

PRE-CHECK (PC) DOCUMENT
Code: 2019 CBC
A separate project application for construction is required.

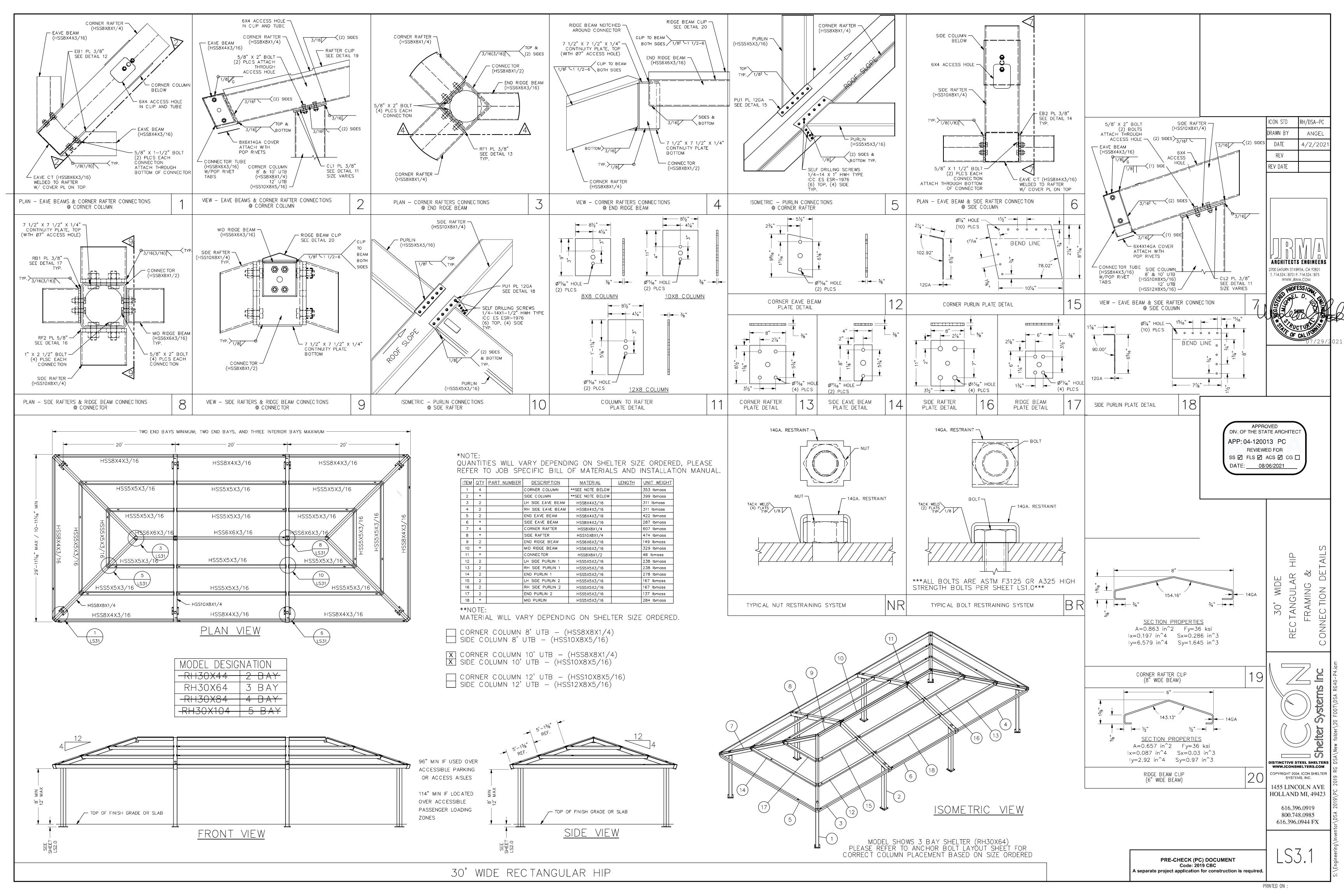
DETAILS BP1. BP2 OR BP3 FOR ANCHOR BOLT PAT**N**ERNS

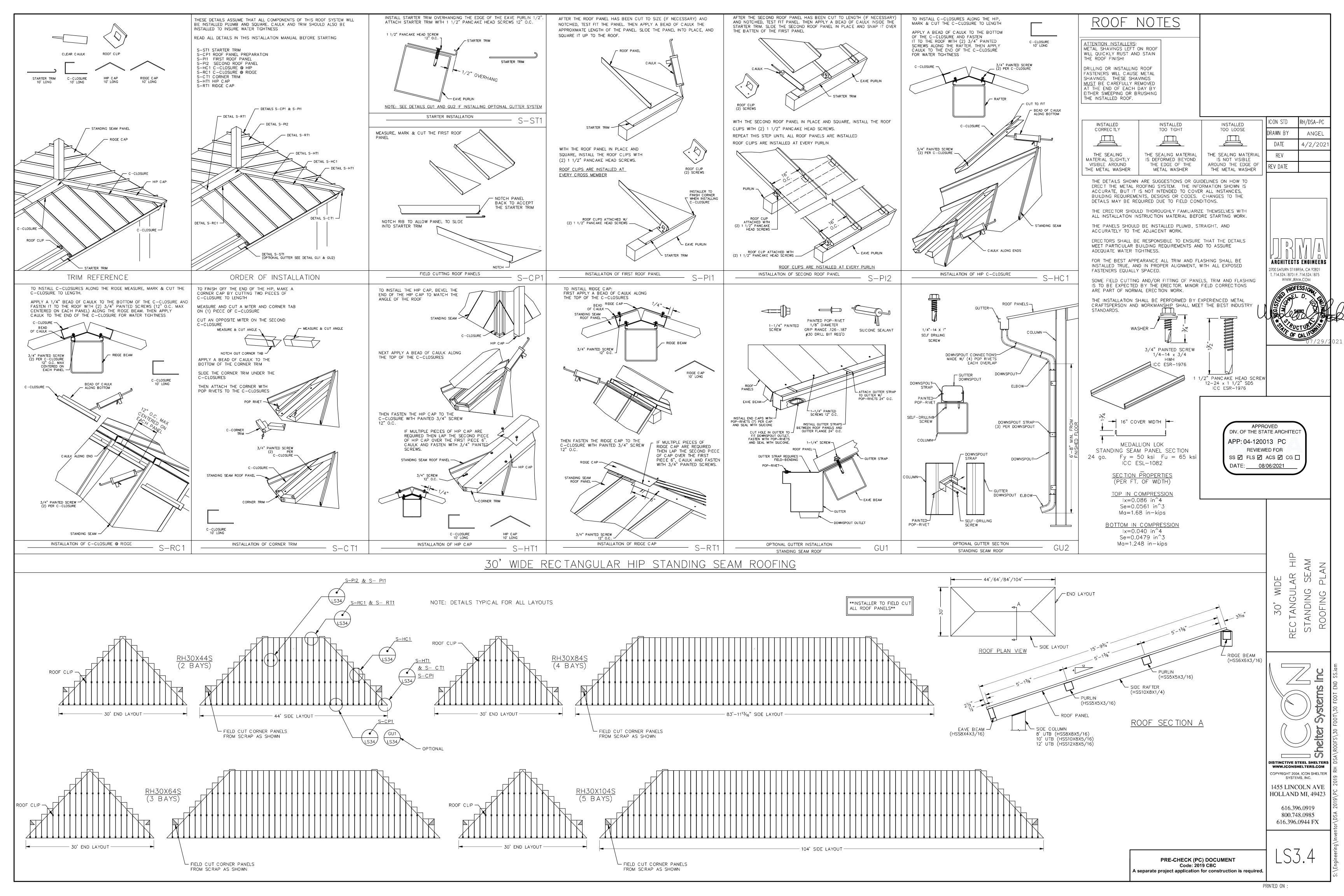
 $t ar{eta}$ P1 & BP2 are (4) bolt pattern while b3 is a (6) b $\delta$ L^

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616.396.0919 800.748.0985

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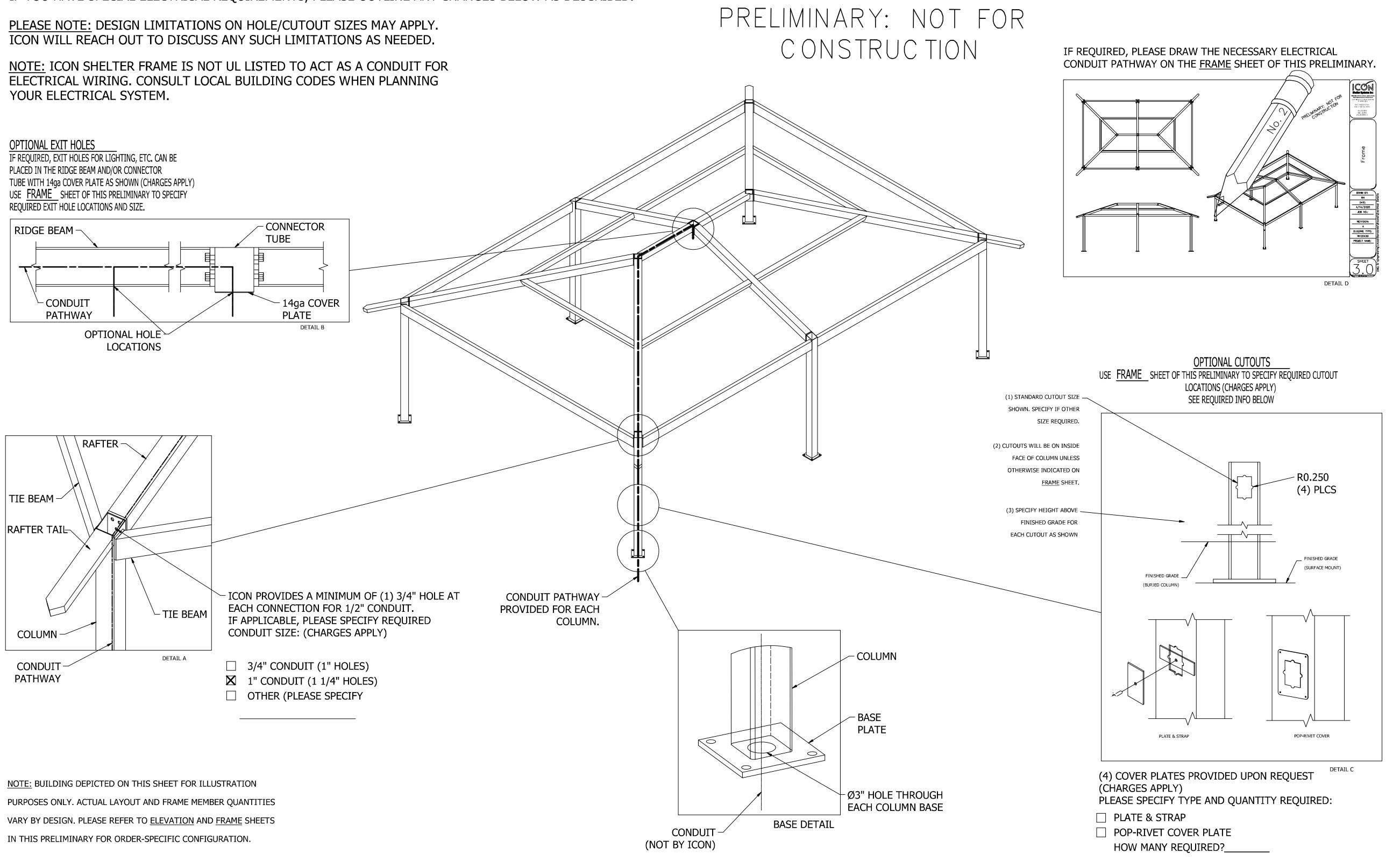


### ELECTRICAL INFORMATION - RECTANGULAR HIP

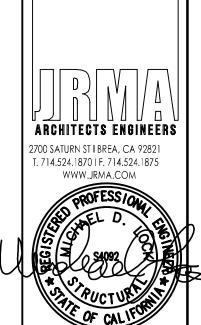
ICON'S STANDARD ELECTRICAL IS DESIGNED TO ACCOMMODATE Ø1/2" CONDUIT WITH A Ø3" INLET HOLE ON THE BOTTOM OF EACH COLUMN. THE CONDUIT PATHWAY RUNS THROUGH THE COLUMN, RAFTER, AND RIDGE BEAM THROUGH ALL BOLTED CONNECTIONS AS SHOWN. IF YOU HAVE SPECIAL ELECTRICAL REQUIREMENTS, PLEASE OUTLINE ANY CHANGES BELOW AS DESCRIBED.

2. ELECTRICAL EXIT HOLES (DETAIL B) 3. ELECTRICAL ACCESS & COVER PLATES (DETAIL C) 4. ELECTRICAL CONDUIT PATHWAY (DETAIL D)

1. CONDUIT HOLE SIZE (DETAIL A)

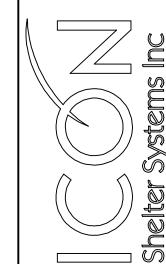


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APPROVED DIV. OF THE STATE ARCHITEC APP: 04-120013 PC SS 🗹 FLS 🗹 ACS 🗹 CG 🗌

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