

### **GENERAL DEMOLITION NOTES:**

- I. CONTRACTOR SHALL COORDINATE UNDERGROUND DEMOLITION REQUIREMENTS
- 2. ALL DEMOLITION WORK SHALL BE DONE IN ACCORDANCE WITH ARCHITECTURAL PHASING SCHEDULE. CONTRACTOR SHALL REFER TO ARCHITECTURAL AND
- 3. (E) PULL BOX NOT SHOWN OR INDENTIFIED ON DRAWINGS TO REMAIN AND SHALL NEED TO BE ADJUSTED TO (N) FINISH GRADE. CONTRACTOR TO PROVIDE AND INCLUDE, IN BID, BOX ADJUSTMENTS. ADJUSTMENTS INCLUDE
- 4. ALL (E) CONDUITS SHOWN ON DRAWINGS ARE DIAGRAMMATIC AND MAY NOT REFLECT EXACT ROUTING. CONTRACTORS TO INCLUDE IN BID PROFESSIONAL UNDERGROUND CONDUIT LOCATOR AS NEEDED FOR HE/SHE TO BE FAMILIAR WITH THE (E) SITE CONDITIONS AND PROVIDE REQUIRED WORK AND ADJUSTMENTS TO EXTEND/RECONNECT POWER CONDUITS AS NOTED IN
- 5. CONTRACTOR SHALL VERIFY ALL EXISTING ELECTRICAL EQUIPMENT NOTED ON DRAWINGS AND REMOVE TO SOURCE. CONTRACTOR SHALL BE RESPONSIBLE FOR FIELD VERIFYING AND LOCATING POWER AND COMMUNICATION SOURCE AND PROPERLY SAFE-OFF ALL ELECTRICAL

### **DEMOLITION SHEET NOTES:**

- (E) BATTING CAGE TO BE DEMOLISHED. REMOVE (E) POWER OUTLETS BACK TO SOURCE. (E) UNDER POWER FOR MISC. EQUIPMENT TO REMAIN
- 2 MAINTAIN (E) POWER CONDUIT & SIGNAL CONDUIT TO REMAIN OPERATIONAL FOR (E) FOOTBALL SCORE BOARD. MAINTAIN & PROTECT MAIN POWER CONDUIT & SIGNAL CONDUIT TO REMAIN OPERATIONAL.
- (3) COORDINATE WITH LANDSCAPE CONTRACTOR TO DISCONNECT AND
- (4) REMOVE ALL ELECTRICAL OUTLETS IN BUILDING BACK TO SOURCE. CONTRACTOR TO LOCATE (E) SOURCE AS NEEDED TO FACILITATE
- REMOVE ALL ASSOCIATED CONDUIT AND CABLES TO (E) SOURCE TO
- (7) DEMOLISH (E) POWER OUTLET AND (E) SIGNAL JACK BACK TO SOURCE TO FACILITATE BACK STOP REMOVAL. (E) F-CONNECTOR TO BE REMOVE

- (10) PROVIDE NEMA-4 6"X6"X6" CAN. RECONNECT EXISTING XFMR AS A RESULT OF BOOSTER PUMP BEING REMOVED. PROVIDE ASSOCIATED UNISTRUT SUCH THAT THE NEMA-4 CAN IS MOUNTED AS A PEDESTAL.





### **CONDUIT SCHEDULE:**

POWER SYSTEMS
(I) 3"C - POWER - DISTRIBUTION
2 (1) 2"C - POWER - SOFTBALL SCOREBOARD
3 (I) 2"C - LIGHTING - SOFTBALL BATTING CAGE
(I) 2"C - POWER - SOFTBALL BATTING CAGE
5 (1) 2"C - POWER - SOFTBALL DUGOUT
6 (I) 2"C - POWER - SOFTBALL BACKSTOP
(1) 2"C - POWER - BASEBALL SCOREBOARD
8 (I) 2"C - POWER - BASEBALL BATTING CAGE
(I) 2"C - LIGHTING - BASEBALL BATTING CAGE
(I) 2"C - POWER - BASEBALL DUGOUT
(I) 2"C - POWER - BASEBALL BACKSTOP
12 (I) 2"C - POWER - ROMTEC PANEL
(3) 2"CO - SPARE
(1) $3"CO - SPARE$

- $\rangle$  IRRIGATION BOOSTER PUMP. CONTRACTOR SHALL INSTALL AND TERMINATE PER EQUIPMENT MANUFACTURER REQUIREMENTS. COORDINATE WITH LANDSCAPE / DISTRICT FOR EXACT LOCATION.
- 2 IRRIGATION CONTROLLER CONTRACTOR SHALL INSTALL AND TERMINATE PER EQUIPMENT MANUFACTURER REQUIREMENTS. COORDINATE WITH LANDSCAPE / DISTRICT FOR EXACT LOCATION.
- 3 LOCATE, INTERCEPT AND EXTEND CONDUIT TO OUTGOING CONDUIT FROM RESTROOM BUILDING THAT IS STUBBED





### **GENERAL NOTES:**

- I. CONTRACTOR SHALL COORDINATE UNDERGROUND REQUIREMENTS WITH ALL OTHER TRADES TO AVOID CONFLICT.
- 2. CONTRACTOR TO SITE SURVEY EXISTING CONDITIONS AND LOCATIONS OF EXISTING UNDERGROUND SYSTEMS, WHERE (N) TRENCHWORK OCCURS PRIOR TO BIDDING. CONTRACTOR SHALL TAKE PROPER PRECAUTIONS TO ENSURE (E) UNDERGROUND SYSTEMS/CONDUIT/PIPES ARE NOT DAMAGED DURING INSTALLATION. CONTRACTOR IS RESPONSIBLE FOR ANY REPAIRS REQUIRED IN THE EVENT THE (E) UNDERGROUND SYSTEMS ARE DAMAGED AS A RESULT OF THE (N) ELECTRICAL TRENCHWORK.
- SEE SINGLE LINE DIAGRAM FOR WIRE SIZES AND CONDUIT REQUIREMENTS.
- 4. CONTRACTOR TO COORDINATE SITE PLAN TO COMBINE ALL UNDERGROUND CONDUIT IN COMMON TRENCH AS NECESSARY.
- 5. ALL EMPTY CONDUIT SHALL BE PROVIDED WITH NYLON PULL CORD AS NOTED IN THE SPECIFICATIONS.
- 6. ALL ELECTRICAL WORK SHALL BE INSTALLED PER 2022 CEC.
- 7. PRIOR TO ALL (N) TRENCHES, CONTRACTOR TO USE ALL (E) ELECTRICAL CONDUITS AND OTHER UTILITIES TO FAMILIARIZE THEMSELVES WITH THE FIELD CONDITIONS AND ADJUST (N) TRENCHES ACCORDINGLY.
- 8. IN-GRADE PULL BOX IDENTIFIED WITH 'P' SHALL HAVE LID LABELED 'ELECTRICAL'. 9. IN-GRADE PULL BOX IDENTIFIED WITH 'S' SHALL HAVE LID LABELED
- 'SIGNAL'. IO. CONTRACTOR SHALL BE RESPONSIBLE FOR ANY SAW CUTTING AND REMOVAL OF EXISTING SURFACES TO FACILITATE UNDERGROUND
- SYSTEMS. THE CONTRACTOR SHALL PATCH AND REPAIR ALL DAMAGED AND CUT SURFACES TO MATCH ADJACENT.
- II. CONTRACTOR SHALL COORDINATE FINAL LOCATION OF ALL IN-GRADE PULL BOX WITH LANDSCAPE ARCHITECT. THE INTENT IS TO VOID RELOCATING PULL BOXES.
- 12. ALL POWER SYSTEM CONDUITS STUB IN "ELECTRICAL" PULL BOX AND ALL COMMUNICATION SYSTEMS CONDUIT IN "SIGNAL" BOXES AS REQUIRED BY CODE.
- 13. ALL PULL BOXES SHALL BE TRAFFIC RATED B2436 UNLESS OTHERWISE NOTED. SEE DETAIL FOR SPECIFICS.
- 14. COORDINATE PULL BOX ORIENTATION WITH LANDSCAPE ARCHITECT TO BE SQUARE WITH SURFACE CURB, CONCRETE WALKWAY, DRAINAGE, ETC.
- 15. IN-GRADE PULL BOX IDENTIFIED WITH 'L' SHALL HAVE LID LABELED 'LIGHTING'.

### **CONDUIT SCHEDULE:**

	POWER SYSTEMS
$\overline{\langle \rangle}$	(1) 2"C - POWER - SOFTBALL SCOREBOARD
$\langle 2 \rangle$	(I) 2"C - LIGHTING - SOFTBALL BATTING CAGE
3	(1) 2"C - POWER - SOFTBALL BATTING CAGE
$\langle 4 \rangle$	(1) 2"C - POWER - SOFTBALL DUGOUT
5	(I) 2"C - POWER - SOFTBALL BACKSTOP
6	(1) 2"C - POWER - BASEBALL SCOREBOARD
$\langle \tau \rangle$	(I) 2"C - POWER - BASEBALL BATTING CAGE
٨	(I) 2"C - LIGHTING - BASEBALL BATTING CAGE
$\langle q \rangle$	(1) 2"C - POWER - BASEBALL DUGOUT
$\langle 0 \rangle$	(I) 2"C - POWER - BASEBALL BACKSTOP
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#### COMMUNICATION SYSTEMS

- 20 (2) 2"CO SIGNAL
- 21) (1) 2"CO SIGNAL



IDENTIFICATION STAMP DIV. OF THE STATE ARCHITEC APP: 02-121752 INC: **REVIEWED FOR** SS 🗹 FLS 🗹 ACS 🗹 DATE: <u>3/19/2024</u> VERDE DESIGN LANDSCAPE ARCHITECTURE **CIVIL ENGINEERING** SPORT PLANNING & DESIGN 1843 Iron Point Rd. Suite 140 Folsom, CA 95630 tel: 916.415.6554 fax: 916.415.6525 www.VerdeDesignInc.com STAMP CONSULTANT American Consulting Engineers Electrical, Inc. 1590 The Alameda Suite 200 San Jose, CA 95126 JOB #EK23098 408/236–2312 Fax: 408/236–2316 SHEET TITLE ELECTRICAL ENLARGED BASEBALL & SOFTBALL SITE PLAN - NEW PROJECT NAME JOHN F. KENNEDY HIGH SCHOOL BASEBALL, SOFTBALL, & TENNIS COURT IMPROVEMENTS PROJECT ADDRESS 6715 GLORIA DRIVE SACRAMENTO, CA 95831 SUBMITTAL DATE 08/20/23 50% SUBMITTAL 10/25/23 100% SUBMITTAL 01/18/24 DSA BACKCHECK NO. REVISIONS DATE CHECKED BY DRAWN BY CN AA/SF DATE ISSUED SCALE AS NOTED 01/18/24 PROJ. NO. 2304200 SHEET NO. E2.1





# **GENERAL NOTES:**

- I. CONTRACTOR SHALL COORDINATE UNDERGROUND REQUIREMENTS WITH ALL OTHER TRADES TO AVOID CONFLICT.
- 2. CONTRACTOR TO SITE SURVEY EXISTING CONDITIONS AND LOCATIONS OF EXISTING UNDERGROUND SYSTEMS, WHERE (N) TRENCHWORK OCCURS PRIOR TO BIDDING. CONTRACTOR SHALL TAKE PROPER PRECAUTIONS TO ENSURE (E) UNDERGROUND SYSTEMS/CONDUIT/PIPES ARE NOT DAMAGED DURING INSTALLATION. CONTRACTOR IS RESPONSIBLE FOR ANY REPAIRS REQUIRED IN THE EVENT THE (E) UNDERGROUND SYSTEMS ARE DAMAGED AS A RESULT OF THE (N) ELECTRICAL TRENCHWORK.
- 3. LIGHTING AND RECEPTACLE CONDUIT SHALL BE IN SAME TRENCH.
- 4. SEE SINGLE LINE DIAGRAM FOR WIRE SIZES AND CONDUIT REQUIREMENTS.
- 5. CONTRACTOR TO COORDINATE SITE PLAN TO COMBINE ALL UNDERGROUND CONDUIT IN COMMON TRENCH AS NECESSARY.
- 6. ALL EMPTY CONDUIT SHALL BE PROVIDED WITH NYLON PULL CORD AS NOTED IN THE SPECIFICATIONS.
- 7. SEE DETAIL I/ET.I AND 7/ET.I FOR TRENCHING REQUIREMENTS.
- CONTRACTOR TO PROVIDE ALL MATERIALS, EQUIPMENT, SPORT FIELD LIGHTS, CONTROL 8 CABINETS, WIRING, CONDUITS, ETC TO SUCCESSFULLY INSTALL NEW SPORTFIELD LIGHTING. 9. ALL ELECTRICAL WORK SHALL BE INSTALLED PER 2022 CEC.
- IO. ALL CONDUITS FOR OUTLETS AND DATA SHALL BE CONCEALED IN WALL. CONTRACTOR SHALL BE RESPONSIBLE TO COORDINATE WITH DUGOUT CONTRACTOR IN ADVANCE TO ENSURE THEY ARE AWARE OF CONDUITS TO BE CONCEALED IN CMU WALL.

### SHEET NOTES:

- PROVIDE AND INSTALL WEATHERPROOF, GFCI, EXTERIOR OUTLET FOR DUGOUT. OUTLET SHALL BE PROVIDED WITH RAIN-TIGHT "WHILE-IN-USE" LOCKABLE COVER PER C.E.C REQUIREMENTS. OUTLET SHALL BE INSTALLED FLUSH IN CMU WALL. CONTRACTOR SHALL COORDINATE WITH CMU CONTRACTOR TO INSTALL OUTLET FLUSH. CONTRACTOR TO CONFIRM ROUGH-INS WITH ARCHITECT TO ENSURE ALL TRADES ARE COORDINATED.
- $\langle 2 \rangle$ PROVIDE AND INSTALL WEATHERPROOF, GFCI, EXTERIOR OUTLET FOR DUGOUT. OUTLET SHALL BE PROVIDED WITH RAIN-TIGHT "WHILE-IN-USE" LOCKABLE COVER PER C.E.C REQUIREMENTS. CONTRACTOR SHALL COORDINATE WITH FENCING CONTRACTOR TO INSTALL OUTLET ON FENCE POST. CONTRACTOR TO CONFIRM ROUGH-INS WITH ARCHITECT TO ENSURE ALL TRADES ARE COORDINATED.
- $\langle 3 \rangle$  PROVIDE (N) TIMER SWITCH IN HEAVY DUTY, NEMA-3R, LOCKABLE, GASKET BOX. TIMER SHALL BE WATTSTOPPER "TS-400" TIME SWITCH. CONTRACTOR SHALL PROVIDE ALL REQUIRED ACCESSORIES, CONDUIT, CABLES, ETC. FOR COMPLETE INSTALLATION.

# NORTH

# **CONDUIT SCHEDULE:**

(N) (I) | 1/4"C - RECEPTACLE 2 (N) (I) I"C - LIGHTING









E3.2 SCALE: 1/4" = 1'-0"



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- 4. SEE SINGLE LINE DIAGRAM FOR WIRE SIZES AND CONDUIT REQUIREMENTS.
- 5. CONTRACTOR TO COORDINATE SITE PLAN TO COMBINE ALL UNDERGROUND CONDUIT IN COMMON TRENCH AS NECESSARY.
- 6. ALL EMPTY CONDUIT SHALL BE PROVIDED WITH NYLON PULL CORD AS NOTED IN THE SPECIFICATIONS.
- 7. SEE DETAIL I/E7.1 AND 7/E7.1 FOR TRENCHING REQUIREMENTS.
- CONTRACTOR TO PROVIDE ALL MATERIALS, EQUIPMENT, SPORT FIELD LIGHTS, CONTROL CABINETS, WIRING, CONDUITS, ETC TO SUCCESSFULLY INSTALL NEW SPORTFIELD LIGHTING. 9. ALL ELECTRICAL WORK SHALL BE INSTALLED PER 2022 CEC.
- IO. ALL CONDUITS FOR OUTLETS AND DATA SHALL BE CONCEALED IN WALL. CONTRACTOR SHALL BE RESPONSIBLE TO COORDINATE WITH DUGOUT CONTRACTOR IN ADVANCE TO ENSURE THEY ARE AWARE OF CONDUITS TO BE CONCEALED IN CMU WALL.

### **SHEET NOTES:**

- PROVIDE AND INSTALL WEATHERPROOF, GFCI, EXTERIOR OUTLET FOR DUGOUT. OUTLET SHALL BE PROVIDED WITH RAIN-TIGHT "WHILE-IN-USE" LOCKABLE COVER PER C.E.C REQUIREMENTS. OUTLET SHALL BE INSTALLED FLUSH IN CMU WALL. CONTRACTOR SHALL COORDINATE WITH CMU CONTRACTOR TO INSTALL OUTLET FLUSH. CONTRACTOR TO CONFIRM ROUGH-INS WITH ARCHITECT TO ENSURE ALL TRADES ARE COORDINATED.
- $\langle 2 \rangle$ PROVIDE AND INSTALL WEATHERPROOF, GFCI, EXTERIOR OUTLET FOR DUGOUT. OUTLET SHALL BE PROVIDED WITH RAIN-TIGHT "WHILE-IN-USE" LOCKABLE COVER PER C.E.C REQUIREMENTS. CONTRACTOR SHALL COORDINATE WITH FENCING CONTRACTOR TO INSTALL OUTLET ON FENCE POST. CONTRACTOR TO CONFIRM ROUGH-INS WITH ARCHITECT TO ENSURE ALL TRADES ARE COORDINATED.
- (з) PROVIDE (N) TIMER SWITCH IN HEAVY DUTY, NEMA-3R, LOCKABLE, GASKET BOX. TIMER SHALL BE WATTSTOPPER "TS-400" TIME SWITCH. CONTRACTOR SHALL PROVIDE ALL REQUIRED ACCESSORIES, CONDUIT, CABLES, ETC. FOR COMPLETE INSTALLATION.



### **CONDUIT SCHEDULE:**

(N) (I) | 1/4"C - RECEPTACLE 2 (N) (I) I"C - LIGHTING













# **GENERAL NOTES:**

- CONTRACTOR SHALL COORDINATE UNDERGROUND REQUIREMENTS WITH ALL OTHER TRADES TO AVOID CONFLICT.
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- 5. CONTRACTOR TO COORDINATE SITE PLAN TO COMBINE ALL UNDERGROUND CONDUIT IN COMMON TRENCH AS NECESSARY.
- 6. ALL EMPTY CONDUIT SHALL BE PROVIDED WITH NYLON PULL CORD AS NOTED IN THE SPECIFICATIONS.
- 7. SEE DETAIL 7/ET.I FOR TRENCHING REQUIREMENTS.
- 8. EXPOSED CONDUIT FOR BATTING CAGE LIGHTING SHALL BE RIGID STEEL CONDUIT.
- 9. ALL ELECTRICAL WORK SHALL BE INSTALLED PER 2022 CEC.

# **SHEET NOTES:**

- $\langle | \rangle$  provide (N) timer switch in heavy duty, Nema-3R, Lockable, Gasket BOX. TIMER SHALL BE WATTSTOPPER "TS-400" TIME SWITCH. CONTRACTOR SHALL PROVIDE ALL REQUIRED ACCESSORIES, CONDUIT, CABLES, ETC. FOR COMPLETE INSTALLATION.
- $\langle 2 \rangle$  NEW LIGHTING CONDUIT SHALL BE EXPOSED ON BATTING CAGE FENCE

## **CONDUIT SCHEDULE:**

(N) | 1/4"C - RECEPTACLE - BATTING CAGE

2 (N) | 1/4"C - LIGHTING - BATTING CAGE





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 $\overline{E5.1}$  NOT TO SCALE

1

DRAWING NAME: Z:\Projects\Year 2023\EK23098\_JFK High School Baseball Improv\E5.I\_Single Line Diagram.dwg PLOT DATE: 01-12-24 PLOTTED BY: wnguyen

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I. HIGH DENSITY REINFORCED CONCRETE BOX WITH NON-SETTING SHOULDERS POSITIONED TO MAINTAIN GRADE AND FACILITATE BACK FILLING. APPROXIMATE DIMENSIONS SHOWN.

2. ALL CONDUITS SHALL ENTER FROM SIDES OF PULL BOX. CONTRACTOR SHALL PROVIDE PULL BOX EXTENSION AS REQUIRED. NO CONDUITS SHALL BE ALLOWED FROM THE BOTTOM

3. CONTRACTOR SHALL STACK CONDUITS AS REQUIRED TO MEET THE NEC CODE REQUIREMENTS.

6. PROVIDE 4" DRAIN HOLE WITH MINIMUM 8" CRUSHED ROCK BEDDING AT BOTTOM OF





A HEAVY DUTY REINFORCED CONCRETE BOX WITH STANDARD KNOCKOUTS AND PULLING IRONS MADE IN CONFORMANCE WITH PG & E REQUIREMENTS.





NOTES:

- I. HIGH DENSITY REINFORCED CONCRETE BOX WITH NON-SETTING SHOULDERS POSITIONED TO MAINTAIN GRADE AND FACILITATE BACK FILLING. APPROXIMATE DIMENSIONS SHOWN.
- 2. ALL CONDUITS SHALL ENTER FROM SIDES OF PULL BOX. CONTRACTOR SHALL PROVIDE
- PULL BOX EXTENSION AS REQUIRED. NO CONDUITS SHALL BE ALLOWED FROM THE BOTTOM OF THE PULL BOX. 3. CONTRACTOR SHALL STACK CONDUITS AS REQUIRED TO MEET THE NEC CODE REQUIREMENTS.
- 4. PROVIDE BELL ENDS ON ALL CONDUIT.
- 5. ALL PENETRATIONS INTO BOXES SHALL BE SEALED WITH GROUT.
- 6. PROVIDE 4" DRAIN HOLE WITH MINIMUM 8" CRUSHED ROCK BEDDING AT BOTTOM OF BOX FOR DRAINAGE.





E7.1 NOT TO SCALE

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# FIXTURE MOUNTING NOTES:

() THE C BRACKETS' ROTATION AND ANGLES OF INSTALLATION SHALL BE ADJUSTED TO MAKE THE LIGHT FIXTURE STRAIGHT OR LEVEL TO THE GROUND.

# **FIXTURE MOUNTING ON BATTING CAGE**

E7.2 NOT TO SCALE

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- () SIZE OF CONDUCTORS SHALL COMPLY WITH NEC TABLE 250-66
- (2) BOND SEPARATE CONDUCTORS FROM GROUND ROD TO ELECTRICAL PANEL AND TO METAL BUILDING FRAME (NEC 250-50). IN ADDITION TO DETAIL ABOVE, BOND THE ELECTRICAL GROUND TO NEAREST METALLIC COLD WATER PIPE. (NEC 250-50)
- (3) CHECK RESISTANCE TO GROUND, IF RESISTANCE EXCEEDS 25 OHMS, INSTALL ADDITIONAL GROUND RODS AS REQUIRED. (NEC 250-56)
- (4) ALL MODULES OF METAL FRAME BUILDINGS SHALL BE ELECTRICALLY BONDED TOGETHER. (BOLTING ONLY IS NOT ACCEPTABLE BONDING.)





- (5) (N) 3 HOLE FLUSH FITTING L BRACKET
- (6) (N) POWER CONDUIT RIGID

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(7) CONTRACTOR TO COORDINATE WITH SCOREBOARD CONCRETE COLUMN INSTALLER TO MOUNT CONDUIT AS SHOWN.









() PULL CAN SHALL BE PROVIDED WITH SEPERATORS TO DIVIDE POWER & SIGNAL. PROVIDE AS REQUIRED TO COMPLY WITH N.E.C. NEMA-4X PULL CAN SHALL BE APPROVED U.L. LISTED.

(2) PROVIDE ENGRAVED NAME PLATE. IDENTIFY AS SCOREBOARD CONTROL. NAME PLATE SHALL BE PROVIDED PER SPECIFICATIONS.



-(6) (TYP.)



#### CONDUIT WALL PENETRATION DETAIL - 3

E7.3 NOT TO SCALE

---- (I) 2"CO - SCORE KEEPING DEVICE OUTLET TO SIGNAL PULLBOX (TYP).

(I) I"CO - SPARE TO SIGNAL PULLBOX

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#### DISTRIBUTION TRANSFORMER INSTALLATION DETAIL











E7.4 NOT TO SCALE

- 1) ENCLOSED PANELBOARD (MAX WEIGHT 250 LBS)
- (2) TYPE NEMA 3R ENCLOSER.
- (3) PROVIDE UNISTRUT PIOOO MINIMUM 12 GA GALV STEEL.
- (4) PROVIDE STAINLESS STEEL 1/2" $\phi \times 2-1/2$ " Nominal embedment KWIK BOLT TZ2 WEDGE ANCHOR (ICC-ES-ESR 4266), IN MINIMUM 2-3/4" DEEP HOLE. (4) ANCHOR BOLTS PER POST BASE.
- 5 CONCRETE SLAB.
- (6) 120/208V PANEL APPROX. DIMENSIONS OF ENCLOSURE 36"H x 24"W x 12"D
- 7 PROVIDE UNISTRUT FLOOR SUPPORT P2073ASQ POST BASE.
- 8 PROVIDE DOUBLE UNISTRUT PIOOI HS MINIMUM 12 GA GALV STEEL.
- 9 PROVIDE HEX HEAD CAP SCREWS 3/8"x2" WITH HEX NUTS AND WASHERS. (4) CAP SCREWS ARE FOR ATTACHMENT OF PANEL TO REAR STRUTS.
- 10 PROVIDE (2) 1/2" GALV BOLTS FROM P2073ASQ POST BASE INTO EACH SIDE OF VERTICAL UNISTRUT PIOOI. PROVIDE EACH BOLT WITH PIOIO NUT INSIDE STRUT. TYPICAL FOR BOTH P2073A POST BASE.
- (1) PROVIDE I/2" & GALV BOLT FASTENERS AT EACH INTERSECTION.
- (12) 277/480V PANEL APPROX. DIMENSIONS OF ENCLOSER 36"H x 24"W x 12"D
- (13) UNISTRUT BRACKET. PROVIDE PI843 WITH 1/2"\$ M.B. & 1/2"\$ HILTI KB TZ2 TO SLAB.
- (14) UNISTRUT SUPPORT. PROVIDE PIOOO WITH I/2"  $\phi$  M.B. EA END.





E7.4 NOT TO SCALE

IDENTIFICATION STAMP DIV. OF THE STATE ARCHITEC APP: 02-121752 INC: **REVIEWED FOR** SS 🗹 FLS 🗹 ACS 🗹 DATE: <u>3/19/2024</u> VERDE DESIGN LANDSCAPE ARCHITECTURE **CIVIL ENGINEERING** SPORT PLANNING & DESIGN 1843 Iron Point Rd. Suite 140 Folsom, CA 95630 tel: 916.415.6554 fax: 916.415.6525 www.VerdeDesignInc.com STAMP CONSULTANT American Consulting Engineers Electrical, Inc. 1590 The Alameda Suite 200 San Jose, CA 95126 JOB #EK23098 408/236–2312 Fax: 408/236–2316 KEY MAP SHEET TITLE ELECTRICAL DETAILS PROJECT NAME JOHN F. KENNEDY HIGH SCHOOL BASEBALL, SOFTBALL, & TENNIS COURT **IMPROVEMENTS** PROJECT ADDRESS 6715 GLORIA DRIVE SACRAMENTO, CA 95831 SUBMITTAL DATE 50% SUBMITTAL 08/20/23 10/25/23 100% SUBMITTAL 01/18/24 DSA BACKCHECK NO. REVISIONS DATE DRAWN BY CHECKED BY CN AA/SF DATE ISSUED SCALE 01/18/24 AS NOTED PROJ. NO. 2304200 SHEET NO. E7.4 ELECTRICAL DETAILS



••••••				DSA P.C. (	)4-12
		*	SCORFBOARD AS	SEMBLY WORKSHE	FT (ΤΔΒΙ F
4'-0" TO 8'-0"			STEP 1: DETERM ASSEMB HEIGHT,	INE DESIRED SCOREBO LY TABLE (TABLE A BEL PART WIDTH, AND PAF	ARD ASSEM OW). PROV
"TO 10'-0"	0" TO 10'-0"	*	, STEP 2: DETERM	INE TOTAL ASSEMBLY I	HEIGHT. WI
ON-BOLTED COLUMN	MARQUEE: MAT FOOTING		STEP 3: BASED C REQUIRE SEE SHE	N TOTAL ASSEMBLY W D COLUMNS. ETS SB1.X FOR 1 COLU SB2.X FOR 2 COLU SB3.X FOR 3 COLU SB4.X FOR 4 COLU SB6.1 FOR WALL N	IDTH, DETE MN ASSEM MN ASSEM MN ASSEM MN ASSEM MN ASSEM
80.2, SB1.2, SB5.1	SHEETS: SB0.1, SB0.2, SB1.3, SB5.1		STEP 4: PICK FOU WITH BC SHEET IN	4, 5, & 7) JNDATION TYPE (CAISS DLTED COLUMN, OR M, NDEX, SB0.1	ON WITH E AT FOOTING
8'-0" TO 28-0"	0" TO 28-0"	*	STEP 5: MARK AI SELECTE SB4.X)	PLICABLE CHECK BOX D COLUMN/FOUNDATI	FOR SCORE
			STEP 6: FILL IN S	ITE SPECIFIC SEISMIC A	ND WIND V
			STEP 7: FILL IN S	ITE SPECIFIC FLOOD ZO	NE AS REQI
			STEP 8: VERIFY A	ALL APPLICABLE SHEETS	SARE MARK
SON-BOLTED COLUMN 30.2. SB2.2. SB5.1	2 COLUMN: MAT FOOTING SHEETS: SB0.1, SB0.2, SB2.3, SB5.1		SITE SPECIFIC SUE	: ONLY MARKED SHEET: BMITTAL REQUIREN	S AS PART ( <b>JENTS</b>
			SEE DSA POLICY PL	07-02 FOR ADDITIONAL	L INSTRUCT
CON-BOLTED COLUMN BO.2, SB3.2, SB5.1	3 COLUMN: MAT FOOTING SHEETS: SB0.1, SB0.2, SB3.3, SB5.1 4 COLUMN: MAT FOOTING SHEETS: SB0.1, SB0.2, SB4.3, SB5.1	Value	<ul> <li>INCLUDE:</li> <li>1. COMPLETED DS. THE PRE-CHECK MARQUEE, TWO SCHEDULES.</li> <li>2. SITE PLAN OF NUMBER. LOCA SERVING THE SC</li> <li>3. WHERE WIRELE TRAVEL AND AG IDENTIFIED AND</li> <li>4. PROVIDE AN ELE DISPLAY COMPO THE PRE-CHECK</li> <li>5. THE APPLICABLE BOX ON THIS SH</li> <li>6. THE APPLICABLE CHECK BOX ON T</li> <li>7. PROVIDE CUT SH ON THE STRUCT</li> <li>8. SITE SPECIFIC SE</li> <li>9. SITE SPECIFIC SE</li> <li>9. SITE SPECIFIC SE</li> <li>9. SITE SPECIFIC BA ON THE DRAWIN</li> <li>10. STEEL COATING NOTED ON SBO.3</li> <li>11. A GEOHAZARD LOCATED IN A FI BY A GEOTECHN</li> </ul>	A 1 APPLICATION, DSA DOCUMENT WITH AP COLUMN, THREE COL FACILITY IDENTIFYING TION OF SCOREBOARD OREBOARD SHALL BE L SS CONTROLLERS ARE CESSIBLE SEATING FO PROVIDED. VATION OF PROPOSED DNENTS, SIGNAGE, TR DOCUMENT. ALL ELEN SHEETS SHALL BE IDE EET. CONFIGURATION SHA THE 'A' DETAILS ON THE HETS OF THE BOARDS URE. CUT SHEETS SHAL ISMIC DESIGN CRITERIA ASIC DESIGN WINDSPEN URE. CUT SHEETS SHAL ISMIC DESIGN CRITERIA SIC DESIGN WINDSPEN ASIC DESIGN WIN	<ul> <li>ALL STR</li> <li>ALL STR</li> <li>SHALL BE</li> <li>OCATED AN</li> <li>NOT SPEC</li> <li>NOT SPEC</li> <li>NOT SPEC</li> <li>SCOREBO,</li> <li>USSES, ANI</li> <li>AENTIFIED B</li> <li>LL BE IDEN<sup>-</sup></li> <li>E APPLICAB</li> <li>, BOXES, AN</li> <li>L INCLUDE</li> <li>A SHALL BE</li> <li>ED AND SIT</li> <li>WEATHER</li> <li>JIRED PER</li> <li>AN ZONE X,</li> <li>QUIRED VA</li> </ul>
		PER SCHEDULE	VALUES, PROVID	E INFORMATION IN TA	BLE D.
um)		30 psf	MINIMUM SETB	ACK REQUIREMENTS.	JR SIRUCI
		H/240 Value	13. PROVIDE A SITE	SPECIFIC DESIGN FOF	R STRUCTU
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ciont		П С + 0.18		THAT THE WALL FRAME	AMING IS
onents & Cladding ed Design Professional, and to be modified by applica	ble factors per ASCE 7)	q <sub>z</sub> =21.8xK <sub>z</sub> psf K <sub>z</sub> VARIES		BOARD ASSEMBLY	WORKSHI
Data		Value π	Nevco Part No.or Descriptio	on Part Height [ft.]	Par
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aximum)		$S_1 = 1.0 \text{ g}$ A through E $S_{DS} = 2.49 \text{ g}$	ADO 18-3	3'0''	18
		S <sub>D1</sub> = 1.0 g	Future ADO 18-	3 3'0"	18
m Non-Building Structure, ASCE 7-16 Cha	pter 15				
Signs and Billboards Table 15.4-2		R= 3.0 Cs= 0.83			
		$V = C_S W_D$		<b>12'0''</b>	18
a flood zone other than Zone X, a letter star fied in the PC are still applicable. <b>gn Data</b> pter 18A, Table 1806.A.2 (Class 5 Material) L + LL) r value has been increased per CBC Section 5	nped and signed from a Geotechnical Engineer is neede	ed to Value 1,500 psf 100 pcf	Total Assembly Height = Total Assembly Width = Total Assembly Weight = Distance from Finish Grade to Bottom of Sign = SCOREBOARD ASSEMBLY FOOTNOTES	12ft0in.         18ft0in.         615lbs.        10ft0in.         s	Total Height = Tota from Finish Gr
		100 psf	<ol> <li>Verify part number, dimension</li> <li>See Step 3 of Scoreboard Asser</li> </ol>	s, and weight with Nevco mbly Worksheet Instructions	
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BLE A, C & D) INSTRUCTIONS		CHECK ALL	SHEET IN	SHEET INDEX				
SEMBLY. FILL OU OVIDE NEVCO P HTS	ART NUMBERS, PART	(REQ'D)	SB0.1	COVER SHEET				
WIDTH, AND W	EIGHT, TABLE A	(REQ'D)	SB0.2	STRUCTURAL NOTES				
ETERMINE THE N	NUMBER OF		- <del>SB0.3</del>	EXAMPLE DSA 103 - TESTING AND INSPECTIONS				
EMBLY OPTIONS			SD1.1	MARQUEE CAISSON - EMDEDDED				
EMBLY OPTIONS EMBLY OPTIONS			2R1.5	MARQUEE CAISSON - BOLLED				
EMBLY OPTIONS D ASSEMBLY OP	TIONS (SKIP STEPS		-3B1.3	MARQUEE MAT FOOTING				
			<del>SB2.1</del>	TWO COLUMN CAISSON EMBEDDED				
H EMBEDDED CO TNG) . MARK AP	DLUMN, CAISSON PLICABLE SHEET ON	X	SB2.2	TWO COLUMN CAISSON - BOLTED				
			<del>SD2.3</del>	TWO COLUMN MAT FOOTING				
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			<del>SB3.2</del>	THREE COLUMN CAISSON BOLTED				
D VALUES TABLE	E C ON SB0.1.		<del>SD3.3</del>	THREE COLUMN MAT FOOTING				
EQUIRED, TABLE	D ON SB0.1		<del>SD4.1</del>	FOUR COLUMN CAISSON - EMBEDDED				
ARKED ON SHEET RT OF DSA SUBM	T INDEX, SB0.1. IITTAL		<del>5D4.2</del>	FOUR COLUMN CAISSON - DOLTED				
			<del>SB4.3</del>	FOUR COLUMN MAT FOOTING				
ICTIONS REGARI	DING USE AND SUBMITTALS SHALL	X	SB5.1	ATTACHMENT DETAILS				
			<del>SB5.2</del>	OPTIONAL SCOREBOARD FEATURE ATTACHMENT DETAI				
103, AND FILIN E DESIGN OPTIC	g fee and copy of On Marked on the		<del>SB5.3</del>	DECORATIVE ALUMINUM TRUSS ATTACHMENT DETAILS				
OUR COLUMN,	OR WALL ASSEMBLY		<del>SB5.1</del>	DECORATIVE ALUMINUM TRUSS ATTACHMENT DETAILS				
STRUCTURES BY BE IDENTIFIED. AND IDENTIFIE	/ DSA APPLICATION ELECTRICAL PANEL D.		<del>SB6.1</del>	INDOOR WALL MOUNTED SCOREDOARD				
PECIFIED, AN A SCOREBOARD C	CCESSIBLE PATH OF OPERATOR SHALL BE	2022 CALIFOR	RMATION	STANDARDS CODE (TITLE 24, CCR):				
BOARD IDENTIFYING ALL INSTALLED AND ADDITIONAL COMPONENTS IN EIGHTS SHALL BE SPECIFIED. BY MARKING APPROPRIATE CHECK ENTIFIED BY MARKING APPROPRIATE ABLE SHEET. AND EQUIPMENT TO BE MOUNTED		2022 ADMIN 2022 CALIFC 2022 CALIFC 2022 CALIFC 2022 CALIFC 2022 CALIFC 2022 CALIFC 2022 CALIFC	IISTRATIVE CO RNIA BUILDIN RNIA ELECTRI RNIA MECHAN RNIA PLUMBI RNIA ENERGY RNIA FIRE COI RNIA GREEN E	DE, PART 1, TITLE 24 CODE OF REGULATIONS (CCR) G CODE VOLUMES 1 &2, PART 2, TITLE 24 CCR CAL CODE, PART 3, TITLE 24 CCR NICAL CODE, PART 4, TITLE 24 CCR NG CODE, PART 5, TITLE 24 CCR CODE, PART 6, TITLE 24 CCR DE, PART 9, TITLE 24 CCR SUILDING STANDARDS CODE, PART 11, TITLE 24 CCR				
BE PROVIDED IN SITE EXPOSURE	I THE DRAWINGS. SHALL BE PROVIDED	REFERENCED CODE SECTIONS FOR APPLICABLE STANDARDS: 2022 CALIFORNIA BUILDING CODE, CHAPTER 35 2022 CALIFORNIA FIRE CODE, CHAPTER 80						
ER PROTECTION	IF DIFFERENT THAN							
ER IR A-4.13. IF	A SCOREBOARD IS	GENERAL N	OTES AND N	ATERIAL SPECIFICATIONS				
VALIDATING TH	HE ALLOWABLE SOIL	<u>GENERAL REC</u>	UIREMENTS					
	DO NOT MEET THE	1. THE ARC CHARGE	CHITECT OR SHALL SIGN AI	PROFESSIONAL ENGINEER IN GENERAL RESPONSIBLE ND SEAL ALL DRAWINGS AND SPECIFICATIONS PER TITLE				
ICTURES THAT	DO NOT MEET THE	24, PART 2. CHANGES	1, SECTIONS 4 5 TO THE APPF	-316(E) AND 4-317 (H). ROVED DRAWINGS AND SPECIFICATIONS SHALL BE MADE				
TURES LOCATEI	d in an area with	BY ADDE DIVISION SECTION	NDA, OR CON OF THE STAT 4-338.	ISTRUCTION CHANGE DOCUMENTS APPROVED BY THE E ARCHITECT (DSA), AS REQUIRED BY TITLE 24, PART 1,				
1), STRUCTUR IS CAPABLE OI	AL ANALYSIS AND F SUPPORTING THE	3. THE DIST STRUCTU INSPECTO CONTINU	RICT SHALL EI RE HEIGHT IS DR MAY BE IOUS INSPECT	MPLOY A CLASS 2 PROJECT INSPECTOR WHEN OVERALL 35 FEET OR GREATER, OTHERWISE A CLASS 3 PROJECT USED. THE PROJECT INSPECTOR SHALL PROVIDE ION OF THE WORK , AND SHALL SUBMIT VERIFIED				
SHEET <sup>(1)</sup>		REPORTS DEFINED	ON A DSA-6 IN TITLE 24, PA	FORM. THE DUTIES OF THE PROJECT INSPECTION ARE ART 1, SECTION 4-342.				
Part Width [ft]	Part Weight [lb]	4. ALL SCO	REBOARD CO	NTROLS SHALL BE FULLY ACCESSIBLE VIA WIRELESS				
18'0"	120	SITE-SPE		ION.				
		5. ALL ASSE ELECTRIC	MBLIES SHALL ALLY GROUND	ED PER CEC 600.7, SEE DETAIL B/SB5.1				
18'0"	120	<ol> <li>IN FLOOE</li> <li>ASCE 24,</li> <li>SEE PAGE</li> <li>REDUISCE</li> </ol>	D ZONES, LOC SECTION 7.2 P E, SB0.2, FOR A	ATION OF ELECTRICAL ELEMENTS SHALL CONFORM TO ER DSA PR-14-01 SECTION 1.2.1. LL MATERIAL SPECIFICATIONS AND NOTES.				
		PREPARA	TION OF THE	PROJECT SPECIFIC DSA 103 AND IS RESPONSIBLE FOR				
18'0"	615	ALL SHO 103	- drawing A	NU SUBIVITITAL REVIEWS. SEE SBU.3 FOR EXAMPLE DSA				
Total Assembly Height + D sh Grade to Bottom of Sign	istanceftin.							



#### HIRAM JOHNSON HIGH SCHOOL, SACRAMENTO, CA



SIGNATURE OF APPROVAL This rendering is for conceptual purposes only. It may not be to exact scale or specifications and should not be used for installation purposes. Every effort has been made to make it as accurate as possible. Beams and or pillars are for illustration only. Engineering specifications may require changes in the quantity, size and/or shape of beams and pillars to meet installation requirements. Nevco assumes no obligations or liability regarding the viability of applicability of existing structures. THIS DRAWING IS THE PROPERTY OF NEVCO INC. AND SHALL NOT BE REPRODUCED, COPIED, SHARED or DISTRIBUTED WITH ANYONE OTHER THAN THE INTENDED STAFF

OR CLIENT OF THE PROPOSED PROJECT WITHOUT THE EXPRESSED PERMISSION OF NEVCO INC.

# STRUCTURAL NOTES

#### GENERAL NOTES

on the job site.

- 1. The following notes, typical details and schedules shall apply to all phases of this project unless otherwise shown or noted.
- 2. Specific notes and details shall take precedence over general notes and typical details. 3. All materials and workmanship shall conform to the minimum standards of the 2022 edition Title 24 of the California Building Code (CBC) and such other regulating agencies exercising authority over any portion of the work. The contractor shall have a current copy of the CBC
- 4. The "Contract or Construction Documents" shall consist of these notes, details, schedules,
- plans, and drawings. 5. All specifications, including but not limited to materials and products, shall be those put forth in the "Contract or Construction Documents". No substitutions shall be permitted to be used or assumed to be used in the bidding or construction process without written approval by
- the Structural Engineer of Record. 6. The contractor shall examine the "Contract or Construction Documents" and shall notify the Architect or Structural Engineer of Record of any discrepancies he may find before proceeding with the work.
- 7. All information on existing conditions shown on drawings are based on best present knowledge available, but without guarantee of accuracy. The Contractor shall verify and be responsible for all dimensions and conditions at the site and shall notify the Architect or Structural Engineer of Record of any discrepancies between actual site conditions and information shown on or in the "Contract or Construction Documents" before proceeding with work.
- 8. The Contractor shall immediately notify the Architect or Structural Engineer of Record of any condition which in his opinion might endanger the stability of the structure or cause distress of the structure.
- 9. All work shall conform to the best practice prevailing in the various trades comprising work. The Contractor shall be responsible for coordinating the work of all trades.
- 10. These "Contract or Construction Documents" represent the finished structure, and do not indicate the method of construction. The Contractor shall supervise and direct the work and shall be solely responsible for construction means, methods, techniques, sequences and procedures.
- 11. Inspection and approval for fabricator's shops used for fabrication of structural load bearing members, components, materials or assemblies shall conform to CBC Section 1704A.2.5. A. Labeling (as required or specified) shall be provided in accordance with CBC Section
- 1703A 5 B. Evaluation and follow-up inspection services (as required or specified), shall conform to CBC Section 1703A.6.
- 12. The Contractor shall provide temporary bracing and shoring for all structural members as required for structural stability of the structure during all phases of construction.
- 13. The Contractor shall take all steps necessary to ensure proper alignment of the structure after the installation of all structural and finish materials. This shall include any necessary preloading of the structure to determine final position of the completed work.
- 14. Observation visits to the project site by field representatives of Architect and/or Structural Engineer of Record (support services) shall not include inspections of safety or protective measures, nor construction procedures, techniques or methods. Any support services performed by Architect or Structural Engineer of Record during any phase of construction, shall be distinguished from continuous and detailed inspection services (as required by any regulating governmental agency, e.g. the Authority Having Jurisdiction) provided by others. these support services, whether of material or work, are performed solely for the purpose of assisting in quality control and in achieving conformance with contract documents, but do not guarantee Contractor's performance and shall not be construed as supervision of construction.
- 15. These notes, details, drawings and specifications (Contract or Construction Documents) do not carry necessary provisions for construction safety. These documents and all phases of onstruction hereby contemplated are to be governed, at all times, by applicable provisions of the current California Occupational Safety and Health Act.
- 16. Where any conflict occurs between the requirements of federal, state and local laws, codes, ordinances, rules and regulations, the most stringent shall govern.
- 17. Written dimensions shall have precedence over scaled dimensions.
- 18. Drawings (notes, schedules, details and plans) shall have precedence over Structural Calculations.
- 19. In the event that certain features of the construction are not fully shown on the drawings or called for in the General Notes or Specifications, then their construction shall be of the same character as for similar conditions that are shown or called for.
- 20. ASTM designation and all standards refer to the latest amendments.
- 21. These structural "Contract or Construction Documents" shall not be modified without prior written approval of the Structural Engineer of Record.
- 22. Only structural working drawings approved by the Division of the State Architectare permitted to be used for construction on this project. All other drawings or documents are obsolete and are not permitted on the job site, nor shall they be used for any construction purposes. Contractors using unapproved drawings or documents are solely responsible for all work not performed in accordance with the "approved" drawings.
- 23. A Division of the State Architect certified project inspector employed by the District (Owner) and approved by the Division of the State Architect shall provide continuous inspection of the work. The duties of the inspector are defined in Section 4-342, Part 1, Title 24 California Code of Regulations. FOUNDATION NOTES
- 1. Basis: See Structural Design Values Chart, Sheet SB0.1 Table B
- 2. Unexpected soil conditions: Allowable values and foundation design are based upon the minimum values provided in Table 1806A.2 of the 2022 California Building Code. See SB0.1 for values
- 3. Excavate to required depths and dimensions (as indicated in drawings), cut square and smooth with firm level bottoms. Care shall be taken not to over-excavate foundation at 6. All welding shall be done by qualified and certified welders. lower elevation and prevent disturbing of soils around higher elevation.
- 4. Footings shall be poured in neat excavations, without side forms whenever possible. 5. Carry all foundations to required depths into compacted fill or natural soil (as per Structural
- Plans and Details). 6. All foundation excavations shall be inspected and approved by the Inspector of Record or Geotechnical Engineer prior to forming and placement of reinforcing or concrete.
- 7. Foundations shall not be poured until all required reinforcing steel, sleeves, inserts, conduits, pipes, etc. and formwork is properly placed and inspected by the Authority having
- Jurisdiction. 8. The sides and bottoms of excavations which are to have concrete contact must be moistened
- several times just prior to pouring upon them. 9. De-water footings, as required, to maintain dry working conditions.

REINFORCING STEEL

- 1. All reinforcing steel shall be deformed intermediate grade bars conforming to ASTM A615, Grade 60 ( $f_v = 60$  ksi) unless noted otherwise. 2. Reinforcing steel shall not be welded, unless specifically noted otherwise.
- 3. To hold reinforcing bars in their true position and prevent displacement, standard tie and
- Section 26.6.2. 4. Shop drawings for fabrication of any reinforcing steel shall be approved by Contractor and
- submitted to Project Specific Architect or Project Specific Structural Engineer of Record, for their review, prior to fabrication.
- 5. Refer to typical details for minimum splice length and minimum radius of bend of reinforcing
- 6. All reinforcing steel splices shall be staggered 24", unless specifically noted or detailed otherwise.
- 7. All reinforcing bar bends shall be made cold.
- 8. Fabrication, erection and placement of reinforcing steel shall conform to Concrete Reinforcing Steel Institute of Standard Practice.
- 9. Reinforcing steel shall be clean of rust, grease or other material likely to impair bond.

#### CONCRETE

- 1. All concrete shall have a minimum ultimate compressive strength  $(f_c)$  as outlined below at 28 days. All concrete shall be regular weight (unless specifically noted otherwise). A. Concrete for footings: 4,500 psi w/c = 0.45 max.
- 2. Maximum Fly Ash content shall be 15%, by weight, of total cementitious materials and shall conform to ASTM C618.
- 3. All concrete work shall comply with CBC Chapter 19A and ACI 318-19 and latest edition of ACI Manual of Concrete Practice.
- 4. Special Inspection (as required or specified) shall conform to CBC Chapter 17A.
- 5. Cement shall be portland cement Type V and shall conform to ASTM C150.

### PR00F #58153C-PR

### PROOF INCLUDES:

- Model 1608-ETN Baseball/Softball LED Scoreboard 18'W x 6'H x 8"D Scoreboard Color: #73 Maroon Digit Color: White Electronic Team Name Color: White
- Non-illuminated Sign 18'W x 3'H



DATE

ABBREVIATIONS
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		A.B.
6.	Aggregates shall conform to ASTM C33, provide aggregates from a single source.	ABV. ACI
7.	Water shall conform to ASTM C94 and be potable.	ADJ. AHJ
8.	Where not specifically detailed, the minimum concrete cover on reinforcing steel shall be:	AISC
	A. Concrete cast against and permanently exposed to earth or weather: 3"	AOR
10.	All reinforcing steel, anchor bolts, dowels, inserts and any other hardware to be set in concrete shall be well secured in position prior to pouring of concrete.	APPRO
11.	Vibrate all concrete as it is placed, with a mechanical vibrator operated by experienced personnel. The vibrator shall be used to consolidate the concrete, not transport it. Reinforcing and forms shall not be vibrated.	ARCH ASTM
12.	Formwork design and removal shall conform to ACI 318-19 Section 26.11. Remove forms in accordance with the following minimum schedule:	AWS
	A.Side forms of footings:Minimum 48 hoursB.Column and pier forms:72 hours & 70% of design strength	B.O. BOT. b/t
15.	Concrete shall not free fall more than six feet. Use tremie, pump or other approved methods.	CAC
16.	Concrete shall be maintained in a moist condition for a minimum of 5 days after placement.	CBC
17.	The Contractor may use concrete admixtures as a construction means and methods to execute "Contract or Construction Documents". Use of admixture is solely the responsibility of the Contractor.	CJP Ç CLR.
18.	Mix designs shall be prepared by an approved testing laboratory, signed by a licensed engineer and shall be submitted to the Project Specific Design Professional of Record for approval. SSG is not responsible for review or approval of site specific concrete mix design.	COL. CONC CONN CONS
19.	Only one grade of concrete shall be allowed on project site at any one time	CONT
20.	Concrete strength shall be verified by standard cylinder tests (in accordance with CBC Section 1905A.1.16) made by an approved testing laboratory.	Ø DBL. DET.
21.	Concrete placed when the air temperature has fallen to, or is expected to fall below 40° shall conform to ACI 318-19 Section 26.5.4, and ACI 306R-16.	DL DSA DWGS
22.	Concrete placed during hot weather shall conform to ACI 318-19 Section 26.5.5, and ACI 305R-14.	EA. F.F.
23.	Conduits and sleeves placed within structural concrete shall not be tied directly to structural reinforcement.	ELEC. ELEV.
2.4	A. 1" concrete cover shall be maintained around all reinforcement.	EOR
24.	No stakes shall be permitted within the footing section.	EQ.
25.	Concrete shall reach minimum 75% design strength or cure for 3 days minimum prior to installation of steel columns and scoreboard components.	E.S. E.W. EXT.
DRI	LLED CAISSON/PIER AND GRADE BEAM NOTES	FAB.
1.	Excavations for drilled caissons/pier shall be performed in compliance with local grading codes and ordinances as well as CBC Chapters 18A and 33A.	FDN. F.G. F.O
2.	Provide Special Inspection in accordance with CBC Section 1705A.8 and Table 1705A.8.	FRMG
3.	Excavations for all drilled caissons/piers shall be approved by the Project Geotechnical Engineer or Project Special Inspector prior to placing of concrete.	FTG.
4.	Reinforcement for drilled caissons/pier shall be approved by the Structural Engineer of Record prior to placing in caisson/pier excavation.	GALV. GEOR
5.	De-water caisson/pier footings and building excavation as required to maintain dry working conditions.	
6.	Caisson/piers are to be poured within 24 hours after completion of drilling operation. Shoring requirements shall be determined by contractor. Contractor shall be provide fall protection and safety barriers at and near the drilled hole as required by OSHA and the Authority Having Jurisdiction.	1. All allo
7.	The Contractor shall be responsible for all shoring, bracing, etc. necessary to support cut and/or fill banks, and existing structures during excavation, and the forming and placement of concrete.	2. Te
8.	Bottom of caissons/piers shall be thoroughly cleaned prior to placement of concrete.	S
<u>STR</u>	UCTURAL STEEL AND WELDING	3. Ар
1.	All structural steel construction shall conform to AISC 360-16 and AISC 341-16.	an wr

- A. Fabrication of all structural steel shall be done in the shop of an approved fabricator. Inspection and approval for fabricator's shops used for fabrication of structural load bearing members, components, materials or assemblies shall conform to CBC Section 1704A.2.5.
- 2. All structural steel shall conform to the following specifications: A. Angles, channels, plates, bars, rounds, and other miscellaneous shapes
- Shall conform to ASTM A36 and shall have a minimum yield stress ( $F_v$ ) of 36 ksi. B. Wide-flange shapes: Shall conform to ASTM A992 and shall have a minimum yield stress ( $F_{y}$ ) of 50 ksi.
- C. Structural tubes: Shall be ASTM A500, Grade C, and shall have a min. yield stress ( $F_v$ ) of 50ksi.
- All structural steel fasteners shall conform to the following specifications:
- A. Bolts shall conform to ASTM A307 Anchor Bolts shall conform to ASTM F1554, Grade as noted in drawings
- Carbon steel nuts shall conform to ASTM A563 D. Stainless steel nuts shall conform to ASTM F594
- E. Washers shall conform to ASTM F436
- 4. Special Inspection shall be provided for all structural steel and welding, in accordance with CBC Chapter 17A. All structural steel shall be fabricated, erected and welded in accordance with AISC
- Specifications for Structural Steel Buildings (AISC 360-16) and Code of Standard Practice for Steel Buildings and Bridges (AISC 303-16).
- 7. Shop drawings for the fabrication of any structural steel shall be approved by the Contractor and submitted to Project Specific Architect or Project Specific Structural Engineer of Record for their review, prior to fabrication.
- 8. No holes other than those specifically detailed shall be allowed through structural steel members. Burning of holes is not permitted.
- 9. All welding shall conform to 'AWS D1.1' specifications for welding. (E-70XX Electrodes). 10. Where fillet weld size is not indicated, use 'AWS' minimum size based on the thickness of the thinner part being welded, as specified in AISC Specifications for Structural Steel Buildings
- (AISC 360-10), Section J2.2. 11. All butt welds to be complete joint penetration, unless specifically noted otherwise.
- 12. Welder qualification requirements, welding procedure and welding electrodes for all structural steel (except structural sheet steel, see steel decking) shall conform to CBC Sections 1705A.2.1 and 2204A.1.
- 13. Provide 3" minimum concrete cover around all structural steel below grade.
- 14. Structural steel embedded into concrete shall be uncoated.
- 15. Structural steel shall be hot-dip galvanized (minimum ASTM A123 or A153 Class D) or painted with zinc-rich primer, undercoat, and finish coat; or equivalent paint system.
- anchorage devices must be provided. Placing of reinforcement shall conform to ACI 318-19 16. All exposed steel fasteners, including cast-in-place anchor bolts/rods, shall be stainless steel (Type 304 minimum), hot-dip galvanized (ASTM A153, Class D minimum or ASTM F2329), or protected with corrosion-preventive coating that demonstrated no more than 2% of red rust in minimum 1,000 hours of exposure in salt spray test per ASTM B117. Zinc plated fasteners do not comply with this requirement.

Anchor Bolt HORIZ. Horizontal ABV. Above American Concrete Institute HSS Hollow Steel Section Adjacent HT Height ADJ. AHJ Division of the State Architect ICC International Building Code AISC American Institute of Steel International Code Council Construction ICC AOR Architect of Record Inside Diameter APPROX. Approximate(ly) Inch, Inches IN. INT. ASCE American Society of Civil Interior Engineers ARCH. Architect, Architecture ksi Kips per Square Inch ASTM American Society of Testing Live Load and Materials ATR All Thread Rod MAX. Maximum American Welding Society AWS MB Machine Bolt MFR Manufactured, Manufacturer Bottom of B.O. \_\_\_\_\_ MIN. Bottom Minimum BOT. MPH Miles per Hour Between California Administrative Code N/R Not Required Not to Scale N.T.S. California Building Code Cast-in-place On Center 0.C. Complete Joint Penetration Over Centerline Outside Diameter OD Clear Column COL. PEN. Penetration CONC. Concrete PL. Plate CONN. Connection PJP CONST. Partial Joint Penetration Construction psi Pounds per Square Inch CONT. Continue. Continuous PSF Pounds per Square Foot Diameter REBAR Reinforcing Bar Double REINF. Reinforcement DET. Detail req'd Required Dead Load DSA Division of State Architect S.F. Square Feet DWGS. Drawings SHT. Sheet SIM. Similar Each FA. SMS Sheet Metal Screw Each Face SQ. Square ELEC. Electric, Electrical stagg'd Staggered ELEV. Elevation STD. Standard EMBED. Embedded, Embedment STL. Steel EOR Engineer of Record Structural Engineer of Record SEOR Equal EQUIP. Equipment Top and bottom T&B Each Side THR'D Threaded Each Way = W/ Т.О. Top of \_\_\_\_\_ EXT. Exterior TYP. Typical FAB. Fabricated U.N.O. Unless Noted Otherwise FDN. Foundation Finish Grade VERT. Vertical Face of \_\_\_\_\_ F.(). Verify in Field Framing VIF FRMG. Foot,Feet With w/ Footing TG. w/c Water/Cement Ratio Welded Steel Stud GA. Gauge WSS GALV. Galvanized WT. Weight GEOR Geotechnical Engineer of Record

#### OST INSTALLED ANCHOR & TESTING

All post-installed anchors are to be tension tested with the exception that torque testing is allowed if the anchors are specifically designed as torque controlled

Test quantity of post-installed anchors as noted below:							
Application	Quantity						
Non-structural (Equipment Anchorage, etc.)	50%						
Structural	100%						

- Apply proof test loads to anchors without removing the nut if possible. if not, remove nut and install a threaded coupler to the same tightness of the original nut using a torque wrench and apply load.
- All tests shall be performed in the presence of the inspector.
- 5. Reaction loads from test fixtures may be applied close to the anchor being tested, provided the anchor is not restrained from withdrawing or restricted from a concrete shear cone type failure mechanism.
- 6. Test equipment is to be calibrated by an approved testing laboratory in accordance with standard recognized procedures.
- 7. The following criteria apply for the acceptance of installed anchors: A. Hydraulic ram method: anchors tested with a hydraulic jack or spring loaded devices shall maintain the test load for a minimum of 15 seconds and shall exhibit no discernable movement during the tension test, e.g. as evidenced by loosening of the washer under the nut.

B. Torque wrench method: anchors tested with a calibrated torque wrench must attain the manufacturer recommended torque within  $\frac{1}{2}$  turn of the nut. Exceptions

- Wedge or sleeve type: one-quarter turn of the nut from 3/8" sleeve anchor
- Threaded type: one-quarter turn of the screw after initial seating of the screw head.
- 8. If any anchor fails testing, test all anchors of the same type not previously tested until twenty consecutive anchors pass, then resume the initial test frequency. if the anchors are used for the support and bracing of non-structural components (pipe, duct or conduit), the twenty shall be only those anchors installed by the same trade.

9. Test loads per ICC ESR, IAPMO, OR UES report

10. When installing drilled-in anchors and/or powder driven pins in existing non-prestressed reinforced concrete, use care and caution to avoid cutting or damaging the existing reinforcing bars. When installing them into existing prestressed concrete (pre- or post-tensioned) locate the prestressed tendons by using a non-destructive method prior to installation. Exercise extreme care and caution to avoid cutting or damaging the tendons during installation. Maintain a minimum clearance of one inch between the reinforcement and the drilled-in anchor and/or pin.

ANCHOR TORQUE TEST VALUES						
	CONCRETE		MASONRY			
Anchor Diameter	HILTI KB TZ 2	SIMPSON STRONG BOLT 2	HILTI KB TZ 2	SIMPSON STRONG BOLT 2		
	ESR-4266 ESR-3037		ESR-4561	ER-240		
3/8"	30 ft-lb	30 ft-lb	15 ft-lb	20 ft-lb		
1/2"	50 ft-lb	60 ft-lb	25 ft-lb	35 ft-lb		
5%"	40 ft-lb	90 ft-lb	30 ft-lb	55 ft-lb		
3⁄4"	110 ft-lb	150 ft-lb	50 ft-lb	100 ft-lb		

If the manufacturer's recommended installation torque is less than the test torque noted in the table, the manufacturer's recommended installation torque should be used in lieu of the tabulated values.

See manufacturer's ESR report for Maximum Impact Wrench Torque Rating.





TWO COLLINAN ASSEMBLY																			
										ASSEIVIDLI		DACE							
		ASSEIVIBLEC		COLUMN			חורם					BASE							
WIDTH, W	THIS APPLICATION	WEIGHT	HEIGHT, H	SPACING, S	SIZE	W/O FLAG	DIAMETER, d	DEPTH, D	LONG. REINF.	REINF. (1)	THICKNESS, t	WIDTH, B	LENGTH, L	WELD	DIAMETER	GRADE	DISTANCE, X	HEIGHT	embed
		770 lbs.	≤ 8'-0"	6'-0"	W8x24	W8x24	36"Ø	7'-0"	8 - #6	#4 @ 4 <sup>1</sup> / <sub>2</sub> " o.c.	1"	20"	20"	5/16	(4) - 1 <sup>1</sup> / <sub>8</sub> "Ø	F1554 - GR.36	2 <sup>1</sup> / <sub>4</sub> "	2"	10"
8'-0"		1,160 lbs.	≤ 12'-0"	6'-0"	W10x33	W10x33	36"Ø	8'-0"	8 - #6	#4 @ 4½" o.c.	11/8"	20"	20"	<u> </u>	$(4) - 1\frac{1}{8}"\emptyset$	F1554 - GR.36	21/4"	21	48"
		1,540 lbs.	≤ 16'-0"	6'-0"	W12x40	W12x40	36"Ø	9'-0"	8 - #6	#4 @ 4½" O.C.	11/1	20"	20"	<u> </u>	$(4) - 1\frac{1}{8}$	F1554 - GR.55	21/	2"	48"
		870 lbs	≤ 20 -0	8'-0"	VV 14X61	VV 14X01	42 Ø	<u>9-9</u> 7'_3"	<u>8 - #8</u>	$\#1 @ 1^{1/2} = 0$	1"	24	24	<u> </u>	$(4) - 1/4 \emptyset$ $(4) - 11/4 \emptyset$	F1554 - GR 36	272	<u> </u>	18"
		1.300 lbs.	< 12'-0"	8-0	W10x33	W10x33	36"Ø	8'-3"	8 - #6	#4 @ 4 <sup>1</sup> /2" o.c.	1 <sup>1</sup> //"	20"	20"	716 5/16	$(4) - 1\frac{1}{8}$	F1554 - GR 36	21/4	2"	48"
9'-0"		1,730 lbs.	< 16'-0"	8'-0''	W12×45	W12x40	36"Ø	9'-3"	8 - #6	#4 @ 4½" o.c.	11/2"	20"	20"	5/16	$(4) - 1\frac{1}{8}$	F1554 - GR.55	21/1"	2"	48"
		2,160 lbs.	< 20'-0"	8'-0''	W14x61	VV1 4×61	42"Ø	10'-0"	8- #8	#4 @ 6" o.c.	11/1"	24"	24"	3/2	(4) - 1 <sup>1</sup> / <sub>4</sub> "Ø	F1554 - GR.55	21/2"	2"	64"
		960 lbs.	≤ 8'-0"	8'-0"	W8x24	W8x24	Sc!!d	7'-6"	8 - #6	#4 @ 4½" o.c.	1"	20"	20"	5/16	(4) - 1 <sup>1</sup> / <sub>8</sub> "Ø	F1554 - GR.36	21/1"	2"	48"
		1,440 lbs.	≤ 12'-0"	8'-0"	W10x33	W10x33	36"Ø		8 - #6	#4 @ 4 <sup>1</sup> /"	11/8"	20"	20"	5/16	(4) - 1 <sup>1</sup> / <sub>8</sub> "Ø	F1554 - GR.36	21/4"	2"	48"
10'-0"		1,920 lbs.	<u>≤ 16'-0"</u>	8'-0"	W14x48	W14x48	42"Ø	9'-9"		#4 @ 4 <sup>1</sup> / <sub>2</sub> " o.c.	11/8"	24"	24"	5/16	(4) - 1 <sup>1</sup> / <sub>4</sub> "Ø	F1554 - GR.105	2 <sup>1</sup> / <sub>2</sub> "	2"	48"
		2,400 lbs.	≤ 20'-0''	8'-0"	W14x61	W14x61	48"Ø	91.01	8 - #8	#4 @ 6" o.c.	11/4"	24"	24"	3/8	(4) - 1¼"Ø	F1554 - GR.105	2 <sup>1</sup> /2"	2"	64"
		1,160 lbs.	≤ 8'-0''	8'-0"	W10x33	W10x30	2011	8'-0"	8 - #6	#4 @ 4 <sup>1</sup> / <sub>2</sub> " o.c.		20"	20"	5/16	(4) - 1½"Ø	F1554 - GR.36	21/4"	2"	48"
		1,730 lbs.	≤ 12'-0"	8'-0"	W14x43	14/1-1743	42"Ø	9'-3"	8 - #6	#4 @ 4½" o.c.	11/8"	24"	24"	5/16	(4) - 1¼"Ø	F1554 - GR.55	21/2"	2"	48"
12'-0"		2,310 lbs.	≤ 16'-0"	8'-0"		W14x53	42"Ø	10'-3"	8 - #8	#4 @ 4 <sup>1</sup> / <sub>2</sub> " o.c.	11/8"	24"	24	3/8	(6) - 1¼"Ø	F1554 - GR.55	21/2"	2"	64"
		2,880 lbs.	≤ 20'-0''	0'0	W14x61	W14x61	48"Ø	10'-3"	8- #8	#4 @ 6" o.c.	13/8"	24"	24"	716	(6) - 1¼"Ø	F1554 - GR.55	21/2"	2"	64"
		1,540 lbs.	-0-0"	8'-0''	W10x33	W10x33	36"Ø	8'-9"	8 - #6	#4 @ 4½" o.c.	11/8"	20"	20"	5/16	$(4) - \frac{1}{10}$	F1554 - GR.55	21/4"	2"	48"
1.01.01		2,510 lbs.	≤ 12'-0''	8'-0"	W12x45	W12x40	36"Ø	10'-3"	8 - #6	#4 @ 4½" o.c.	11/4"	24"	24"	3/8	(6) - 1¼"Ø	F1554 - GR 55	2 <sup>1</sup> / <sub>2</sub> "	2"	48"
160		3,080 lbs.	≤ 16'-0"	8'-0"	W14x61	W14x61	48"Ø	12'-0"	8 - #8	#4 @ 6" o.c.	13/8"	24"	24"	7/16	(6) - 1¼"Ø	F1554 - GR.55	272	2"	64"
		3,840 lbs.	≤ 20'-0''	8'-0"	W16x77	W16x67	48"Ø	12'-0"	12- #8	#4 @ 6" o.c.	11/2"	24"	24"	1/2	(6) - 1¼"Ø	F1554 - GR.105	2 <sup>1</sup> / <sub>2</sub> "	Z	64"
		1,730 lbs.	≤ 8'-0"	10'-0"	W12x35	W12x35	36"Ø	9'-0"	8 - #6	#4 @ 4½" o.c.	11/8"	20"	20"	3/8	(4) - 1 <sup>1</sup> ⁄ <sub>8</sub> "Ø	F1554 - GR.36	2 <sup>1</sup> /4"	2"	48
10' 0"	Х	2,600 lbs.	≤ 12'-0"	10'-0"	W14x48	W14x43	42"Ø	10'-0"	8 - #8	#4 @ 6" o.c.	11/4"	24"	24"	3/8	(4) - 1¼"Ø	F1554 - GR.55	21/2"	2"	64"
10-0		3,400 103.	<u> </u>	101-011	W/14x61	W/14x61	48"Ø	10'-9"	8 - #8	#4 @ 6" o.c.	11/4"	24"	24"	7/20	$(6) - 1^{1/2}$	E1EE4 6R.55	212	2	64
		4.320 lbs		10' 0"	W10x77	VV10X//	48 Ø	13'-0"	12- #8	#4 @ 6" o.c.	13/4"	24	30"	910	$(6) - 1^{1} (6)$	E155/L- GR 55	3"	2"	64"
		2,310 lbs.	≤ 8'-0''	14'-0"	W14x43	W14x43	36"Ø	9'-9"	8 - #6	#4 @ 4½" o.c.	11/8"	24"	24"	3/8	(4) - 1¼"Ø	F1554 - GR.55	2 <sup>1</sup> /2"	2"	10
		3,460 lbs.	≤ 12'-0''	14'-0"	W14x61	W14x61	36"Ø	11'-6"	8 - #8	#4 @ 6" o.c.	13/8"	24"	24"	3/8	(6) - 1¼"Ø	F1554 - GR.55	21/11	2"	64"
24'-0"		4,610 lbs.	<u>- 16' 0"</u>	14'-0"	W16x67	W16x67	48"Ø	11'-9"	12 - #8	#4 @ 6" o.c.	13⁄4"	24"	30"	7/16	(4) - 1 <sup>1</sup> / <sub>2</sub> "/4	11554 - GR.55	3"	2"	64"
24-0		5,760 lbs.	≤ 20'-0"	14'-0"	W/18x86	W18x86	48"Ø	13'-3"	12 - #8	#4 @ 6" o.c.	13⁄4"	24"	30"	716	(6) - 1½"∅	F1554 - GR.55	3"	2"	64"
		6,920 lbs.	≤ 24'-0''	14'-0"	W18x130	W18x115	48"Ø	14'-6"	12 - #8	#4 @ 6" o.c.	2"	21	30"	CJP	(6) - 1½"Ø	F1554 - GR.105	3"	2"	64"
		8,070 lbs.	≤ 28'-0"	14'-0"	W18x158	W18x143	54"Ø	10 0	12 - #8	#4 @ 6 <sup>11</sup> ere.	21/2"	24"	36"	CJP	(6) - 2''Ø	F1554 - GR.105	4"	2"	64"
		2,690 lbs.	≤ 8'-0''	14'-0"	W14x43	W14x43	42"Ø	10'-0"	8 - #7	## C 41/" O C.	11/4"	24"	24"	3/8	(4) - 1¼"Ø	F1554 - GR.55	2 <sup>1</sup> / <sub>2</sub> "	2"	64"
		4,040 lbs.	≤ 12'-0''	14'-0"	W14x61	W14x61	- <del>1</del> 0 Ø	11'-3"	8 - #8	#4 @ 6" o.c.	13/4"	24"	30"	3/8	(4) - 1½"Ø	F1554 - GR.55	3"	2"	64"
28'-0"		5,380 lbs.	≤ 16'-0"	14'-0"	\ <u>\</u> \10,777	W16x67	48"Ø	12'-9"	12 - #8	#4 @ 6" o.c.	2"	24"	30	1/2	(6) - 1½"∅	F1554 - GR.55	3"	2"	64"
		6,720 lbs.	< 201 011	14'-0"	W18x97	W18x97	48"Ø	14'-3"	12 - #8	#4 @ 6" o.c.	2"	24"	30"	CJP	(0) 11/10	E1554 - GR.105	3"	2"	64"
		8,070 lbs.	≤ 24'-0''	14'-0''	W18x143	W18x143	54"Ø	15'-9"	12 - #8	#4 @ 6" o.c.	21/2"	24"	36"	CJP	(6) - 2"Ø	F1554 - GR.105	4 11	21/2"	64"
		9,410 lbs.	≤ 28'-0''	14'-0"	W18x175	W18x175	54"Ø	16'-6"	14 - #8	#4 @ 6"o.c.	3"	24"	36"	CJP	(6) - 2"Ø	F1554 -GR.105	4"	21/2	64"

NOTES: (#) 1. CONTRACTOR OPTION TO PROVIDE TIES OR SPIRAL REINFORCING. SEE C/SB2.2 FOR TIE OPTION, SEE D/SB2.2 FOR SPIRAL OPTION 2. CONTRACTOR IS RESPONSIBLE FOR CASING PIERS AND DRILLING SEQUENCING TO PROTECT PIER EXCAVATION

# TWO COLUMN SCOREBOARD INSTALLATION



![](_page_13_Figure_4.jpeg)

![](_page_14_Figure_0.jpeg)

![](_page_14_Figure_1.jpeg)

![](_page_15_Picture_0.jpeg)

![](_page_15_Picture_5.jpeg)

SHEET SCHEDULE
CONTENTS
TITLE SHEET / REVISION & SHEET SCHEDULE
GENERAL NOTES / SYMBOL LEGEND
DESIGN CRITERIA AND CODE SUMMARY
DESIGN CRITERIA AND CODE SUMMARY
FLOOR PLAN
ADA CLEARANCES
INTERIOR ELEVATIONS VIEWS
EXTERIOR ELEVATION VIEWS
EXTERIOR ELEVATION VIEWS
SECTION VIEWS
WALL FINISH SCHEDULE (INTERIOR/EXTERIOR)
DOOR SCHEDULE
DOOR DETAILS
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FOUNDATION DETAILS
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ROOF CONNECTION DETAILS
ROOF CONNECTION DETAILS
ROOFING PLAN
ROOFING DETAILS
PLUMBING SCHEDULE
PLUMBING PLAN
ELECTRICAL SCHEDULE
ELECTRICAL PLAN
ELECTRICAL RISER DETAILS
FLECTRICAL PANEL SCHEDULE

TITLE 24 DOCUMENTS

![](_page_15_Figure_9.jpeg)

![](_page_15_Picture_10.jpeg)

![](_page_16_Figure_0.jpeg)

•	POFESS /ON/A	
	K ROSE	
EE		
	Image: Signal and Signal an	
	OF CAL	GENERAL N
•		
	1. THIS PROJECT SHALL COMPLY WITH A OR EXCEED INDUSTRY STANDARDS FOR M	ALL 2022 CALIFORNIA BUILDING CO IATERIALS, WORKMANSHIP, ETC.
	2. CONTRACTOR SHALL REVIEW THE DR DOCUMENTS SHALL BE BROUGHT TO THE IN CONFLICT WITH OTHER WORK, OR IS NO	AWINGS THOROUGHLY BEFORE PRO IMMEDIATE ATTENTION OF ROMTEC OT APPROVED BY CODE, UNTIL RESO
	3. CONTRACTOR SHALL MAINTAIN GENEI	RAL LIABILITY INSURANCE AND WOR
	4. FOOTINGS SHALL BE CONSTRUCTED	ON UNDISTURBED NATIVE SOIL OR E
<b>`</b>	CONDITIONS TO SATISFY CRITERIA OR NOT	TIFY THE STRUCTURAL ENGINEER TO
	CBC 1804A.4. REFER TO GEOTECHNICAL R	LESS THAN A 5% 5L REPORT BY UES, NO. 4630.2300076
/1 %	5. A. CAST-IN-PLACE CONCRETE: 3000	PSI MINIMUM COMPRESSIVE STRENG
	MATERIALS IN ACCORDANCE WITH ACI 318 WORK, SEE FOUNDATION DETAILS FOR REC	STANDARD. FINE BROOM FINISH IN QUIREMENTS.
	B. CMU BLOCKS "MEDIUM WEIGHT DEN PSI. ALL CMU BLOCKS MUST BE FULLY GR	ISITY" ARE MANUFACTURED TO ASTN OUTED IN 4 FT MAXIMUM LIFTS AND
	MORTAR CONFORMING TO ASTM C270.	MINIMUM COMPRESSIVE STRENGTH
	ACCORDANCE TO MEET ACI 318. COURSE (	SROUT MAY BE USED IN ACCORDAN
ŕ	LOSS AND SETTLEMENT HAS OCCURRED. C VELOCITY VIBRATOR WITH A 3/4 IN. HEAD	IS USED.
	6. ANCHOR AND MACHINE BOLTS SHALL METHOD REQUIRED TURNS FOR PRE-TENSI	. BE ASTM A307, UNLESS NOTED OT ONING FROM SNUG-TIGHT UNO IN
	SCREMS AND MACHINE BOLT CALLOUTS AF	RE MINIMUM SIZE SIZE ALLOWED, AC
	GLU-LAM BEAMS SHALL BE GRADE 24F-V4	4 OR AS STATED IN NOTE #10.
	7. QUESTIONS CONCERNING MATERIALS	OR CONSTRUCTION CONTACT ROM
	8. ROMTEC SCOPE SUPPLY AND DESIGN	I SUBMITTAL (SSDS) IDENTIFY SPECI
	FIXTURES AND ACCESSORIES. REFER TO TH SUPPLIED BY THE INSTALLER.	HE SSDS FOR SPECIFIC LIST OF ITEN
	9. THE OWNER / CONTRACTOR MAY EXE (E.G., LIGHTS, COMFORT HEATERS, ETC.)	RCISE DISCRETION IN SELECTING TH
	10. GLUE LAMINATED BEAMS SHALL BE D	OUGLAS FIR-LARCH, U.N.O. WITH 1-1.
	SUPPLEMENTS THEREOF.	RAL GLUED LAMINATED TIMDER , A
	GLUE LAMINATED BEAM SHALL HAVE THE F - FOR SIMPLY SUPPORTED BEAMS	FOLLOWING GRADES (U.N.O. ON PLA COMBINATION 24F-V4
	- FOR CANTILEVERED BEAMS OR BEAN BEAMS SHALL CONFORM TO A.P.AE.W.S. (	15 CONTINUOUS OVER SUPPORTS OR A.I.T.C. INDUSTRIAL APPEARANC
	MOISTURE CONTENT OF THE LUMBER AT TH	HE TIME OF GLUING SHALL NOT BE N
ER	"CERTIFICATE OF INSPECTION" BY AN APPR	ROVED INSPECTION AGENCY SHALL
	NOTE: ARCHITECT/ENGINEER IS	NOT RESPONSIBLE FOR ANY
	I ENGINEERING AND WILL NOT BE F ISSUES RELATED TO THIS SITE. IT	IS THE OWNER'S RESPONSIBI
	ACCURATELY LOCATE THIS BUILD	NG, SET FLOOR AND ADJAC
	UEIERMINE SILE IS SUITABLE FOR	CONSIKUCION, VERIFY AL

NOTES	

DDES AND STANDARDS IDENTIFIED ON SHEET G2. ALL WORK SHALL MEET

OCEEDING WITH ANY WORK. ANY DISCREPANCIES FOUND WITHIN THESE C. CONTRACTOR SHALL NOT PROCEED WITH ANY WORK HE KNOWS TO BE SOLVED BY ROMTEC OR THE ENGINEER/ARCHITECT.

RKER'S COMP. INSURANCE AS PER SPECIFIC STATE MINIMUM REQUIREMENTS.

ENGINEER APPROVED FILL. CONTRACTOR TO VERIFY ASSUMED SOIL ED THE ASSUMED SOIL BEARING CAPACITY, CONTRACTOR TO MODIFY SOIL D REVISE DESIGN PER CONDITIONS ENCOUNTERED. BACKFILL AROUND LOPE FOR A MINIMUM DISTANCE OF 10' FROM THE BUILDING, PER 2022 5.0016, DATED OCTOBER 17, 2023.

NGTH AT 28 DAYS 4" +/- 1" SLUMP, WITH MAX 1" AGGREGATE, AND ALL NTERIOR SURFACES AND EXTERIOR SLABS. JOINTS REQUIRED IN FLAT

M C90-16 STANDARDS WITH A MIN COMPRESSIVE STRENGTH FM = 2000 NOT BE WETTED. THE MORTAR TO BE USED SHALL BE TYPE S 1800 PSI

AT 28 DAYS 9" +/- 1" SLUMP, WITH MAX 1/2" AGGREGATE, AND TESTED IN NCE WITH 2022 CBC. CONSOLIDATE GROUT AT THE TIME OF PLACEMENT. N, AND RECONSOLIDATE BY MECHANICAL VIBRATION AFTER INITIAL WATER ATION ARE NORMALLY ACHIEVED WITH A MECHANICAL VIBRATOR. A LOW

THERWISE BOLTS SHALL BE INSTALLED PER TURN-OF-NUT INSTALLATION I THIS PLANSET OR BY ANCHOR, BOLT OR FASTENER MANUFACTURER. CTUAL SIZE MAY VARY. STEEL PLATES & SHAPES SHALL BE ASTM A36, Fy = E 60). WOOD FRAMING SHALL BE #2 OR BTR DOUGLAS FIR, UNO.

ITEC TECHNICAL ASSISTANCE AT: 541-496-3541

TFIC MODEL, MANUFACTURER & BRAND OF ALL PLUMBING AND ELECTRICAL EMS SUPPLIED BY ROMTEC, ANY ITEMS NOT LISTED IN THE SSDS IS ASSUMED

HE FINAL LOCATION FOR NON-DIMENSIONED ACCESSORIES AND FIXTURES

1/2" OUTER AND CORE LAMINATIONS AND SHALL CONFORM TO THE AITC/A.P.A.-E.W.S. #117, ANSI/AITC A-190.1 AND ALL APPROVED

ANS):

Y SITE DESIGN OR BLE FOR ANY BILITY TO CENT ELEVATIONS, L UTILITIES, ETC.

![](_page_16_Picture_18.jpeg)

RECYCLE ALL USED SHIPPING MATERIALS AND LEFT OVER BUILDING MATERIALS

![](_page_16_Figure_20.jpeg)

![](_page_16_Picture_21.jpeg)

CODES AND STAND	ARDS			SPECIAL IN
2022 CALIFORNIA AD	MINISTRATIVE CODE	E (CAC), TITLE 24 PART 1		TABLE 3
2022 CALIFORNIA BUI	ILDING CODE (CBC),	TITLE 24 PART 2 (BASED ON 2021 IBC)		
2022 CALIFORNIA EL	ECTRICAL CODE, TIT	LE 24 PART 3 (BASED ON 2020 NFPA, N	NEC)	
2022 CALIFORNIA ME	CHANICAL CODE, TIT	TLE 24 PART 4 (BASED ON 2021 UMC)		
2022 CALIFORNIA PLI 2022 CALIFORNIA BUI	UMBING CODE, III LE	= 24  PART 5 (BASED ON 2021 UPC) DE TITLE 24 PART 6		
2022 CALIFORNIA FIR	RE CODE, TITLE 24 P	2, THE 21 FART 0 ART 9, (BASED ON 2021 IFC)		PRIOR TO SPECIFIC
2022 CALIFORNIA GR	REEN BUILDING STAN	DARDS CODE		DURING STABILIT
ACI AMERICAN CONCI	RETE INSTITUTE, ACI 318	3-19, "BUILDING CODE REQUIREMENTS FOR STRU	JCTURAL CONCRETE"	TO THE F DURING 5,000 sq.
TMS THE MASONRY SC	DCIETY, TMS 402-16, "BUI	LDING CODE REQUIREMENTS FOR MASONRY ST	RUCTURES"	DURING
AISC AMERICAN INSTIT	UTE OF STEEL CONSTR	UCTION		AS DELIN MORTAR
"STEEL CONSTRU	CTION MANUAL, 15TH E	DITION"		(a) R=RE
				TABLE 4 -
CODE SUMMAR	<b>R</b> Y:			
OCCUPANCY C	LASS : U			
CONSTRUCTION	N: VB			
AREA: <u>222</u> F AREA ALLOMA	BLE: <u>5500</u> FT <sup>2</sup>			1 .05 .040
HEIGHT: <u>1</u> STO	RY NABLE: 1 STORY			FOLLOW
OCCUPANT LO	$ADLL: \underline{1} \text{ DIV}$			A. Pi
				B. G
DESIGN LOADS		20 PGE		C. GF
ROOF: DEAD	LOAD	15 PSF		B
CBC SEISMIC 1	DESIGN CATEGOR	RY D		D. PR
DESIGN WIND	SPEED (ULTIMATE	) 95 MPH Exposure c		E. PR
ALLOMABLE S	OIL BEARING	1500 PSF PER GEOTECHNICAL ENGINEERING	REPORT	
		BTUES, DATED OCTOBER 11, 2023	2.	
SEISMIC DE	SIGN DATA:	WIND DESIGN :		2. PRIOR COMPLIA
RISK CATEGORY:		RISK CATEGORY	П	A. GI
IMPORTANCE FACTOR:	1.0			
SS:	0.620	VVINU SPEED =	90 IVIPH	B. PL/
55. S1 <sup>.</sup>	0.266	EXPOSURE:	С	C. PL/ ANCH
SITE CLASS:	D	INTERNAL PRESSURE COEFE =	± 0.18	D. PR PRES
SMS:	0.809			3. VERIE
SM1:	0.550			
SDS:	0.539			
SD1	0.367			B. PL/ C
ISMIC DESIGN CATEGORY	D			
	-			
BASE SHEAR: V =	0.108 W			
				E. WE
				G. AP
EARING WALL SYSTEM: SP	ECIAL REINFORCED MAS	SONRY SHEAR WALL		G. A F

ANALYSIS METHOD: EQUIVALENT STATIC FORCE METHOD

#### SPECIAL INSPECTIONS

SPECIAL INSPECTION AND TESTS OF CONCRETE CONSTRUCTION ARE REQUIRED FOR FOUNDATIONS SUPPORTING CMU WALLS

	SPECIAL INSPECTIONS (TMS 602-16) TABLE 3 MINIMUM VERIFICATION REQUIREMENTS				]
	MINIMUM VERIFICATION	REQUIRED FOR QUALITY ASSURANCE (a)		REFERENCE FOR CRITERIA	
			LEVEL 3	TMS 602	
	PRIOR TO CONSTRUCTION, VERIFICATION OF COMPLIANCE OF SUBMITTALS		R	ART. 1.5	
	PRIOR TO CONSTRUCTION, VERIFICATION OF <i>f</i> ' <i>m</i> AND <i>f</i> 'AAC, EXCEPT WHERE SPECIFICALLY EXEMPT BY THE CODE.		R	ART. 1.4 B	
	DURING CONSTRUCTION, VERIFICATION OF SLUMP FLOW AND VISUAL STABILITY INDEX (VSI) WHEN SELF-CONSOLIDATING GROUT IS DELIVERED TO THE PROJECT SITE.		R	ART. 1.5 & 1.6.3	
CONCRETE"	DURING CONSTRUCTION, VERIFICATION OF <i>f</i> 'm AND f'AAC FOR EVERY 5,000 sq. ft. (465 sq.m).		R	ART. 1.4 B	
ES"	DURING CONSTRUCTION, VERIFICATION OF PORPORTIONS OF MATERIALS AS DELIVERED TO THE PROJECT SITE FOR PREMIXED OR PREBLENDED MORTAR, PRESTRESSING GROUT, AND GROUT OTHER THAN SELF-CONSOLIDATING GROUT.		R	ART. 1.4 B	
	(a) R=REQUIRED, NR=NOT REQUIRED				J
	TABLE 4 MINIMUM SPECIAL INSPECTION REQUIREMENTS				
	MINIMUM SPECIAL INSPECTION				
	INSPECTION TASK	REQUIRED FOR QUALITY AS	SURANCE (a)	REFERENCE	FOR CRI
			LEVEL 3	TMS 402	
	1. AS MASONRY CONSTRUCTION BEGINS, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE:				
	A. PROPORTION OF SITE-PREPARED MORTAR		Ρ		ART. 2
	B. GRADE AND SIZE OF PRESTRESSING TENDONS AND ANCHORAGES		Р		ART
	C. GRADE, TYPE AND SIZE OF REINFORCEMENT, CONNECTORS, ANCHOR BOLTS, AND PRESTRESSING TENDONS AND ANCHORAGES		Р		AR
	D. PRESTRESSING TECHNIQUE		Р		
	E. PROPERTIES OF THIN-BED MORTAR FOR AAC MASONRY		С		Δ
	F. SAMPLE PANEL CONSTRUCTION		С		A
	2. PRIOR TO GROUTING, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE:				
	A. GROUT SPACE		С		ART
MPH	B. PLACEMENT OF PRESTRESSING TENDONS AND ANCHORAGES		Р	SEC. 10.8 & 10.9	AF
	C. PLACEMENT OF REINFORCEMENT, CONNECTORS, AND ANCHOR BOLTS		С	SEC. 6.1, 6.3.1, 6.3.6, & 6.3.7	AF
8	D. PROPORTIONS OF SITE-PREPARED GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS		Р		ART. 2
	3. VERIFY COMPLIANCE OF THE FOLLOWING DURING CONSTRUCTION:				
	A. MATERIALS AND PROCEDURES WITH THE APPROVED SUBMITTALS		Р		
	B. PLACEMENT OF MASONRY UNITS AND MORTAR JOINT CONSTRUCTION		Р		
	C. SIZE AND LOCATION OF STRUCTURAL MEMBERS		Р		
	D. TYPE, SIZE AND LOCATION OF ANCHORS, INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES OR OTHER CONSTRUCTION.		С	SEC. 1.2.1(e), 6.2.1 & 6.3.1	
	E. WELDING OF REINFORCEMENT		С	SEC. 6.1.6.1.2	
	F. PREPARATION, CONSTRUCTION, AND PROTECTION OF MASONRY DURING COLD WEATHER (TEMPERATURE BELOW 40°F(4.4°C)) OR HOT WEATHER (TEMPERATURE ABOVE 90°F(32.2°C))		Р		ART
	G. APPLICATION AND MEASUREMENT OF PRESTRESSING FORCE		С		
	H. PLACEMENT OF GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS IS IN COMPLIANCE		С		AR

I. PLACEMENT OF AAC MASONRY UNITS AND CONSTRUCTION OF THIN-BED MORTAR JOINTS

4. OBSERVE PREPARATION OF GROUT SPECIMENS, MORTAR SPECIMENS, AND/OR PRISMS

(a) FREQUENCY REFERS TO THE FREQUENCY OF INSPECTION, WHICH MAY BE CONTINUOUS DURING THE LISTED TASK OR PERIODICALLY DURING THE LISTED TASK, AS DEFINED IN THE TABLE. NR=NOT REQUIRED, P=PERIODIC, C=CONTINUOUS

(b) REQUIRED FOR THE FIRST 5000 SQUARE FEET (465 SQUARE METERS) OF AAC MASONRY. (c) REQUIRED AFTER THE FIRST 5000 SQUARE FEET (465 SQUARE METERS) OF AAC MASONRY.

a)	REFERENCE FOR CRITERIA
3	TMS 602
	ART. 1.5
	ART. 1.4 B
	ART. 1.5 & 1.6.3
	ART. 1.4 B
	ART. 1.4 B

CE (a)	REFERENCE F	OR CRITERIA
/EL 3	TMS 402	TMS 602
Ρ		ART. 2.1 , 2.6 A, & 2.6 C
Ρ		ART. 2.4 B & 2.4 H
Ρ		ART. 3.4 & 3.6 A
P		ART. 3.6 B
С		ART. 2.1 C.1
С		ART. 2.1 C.1
С		ART. 3.2 D & 3.2 F
P	SEC. 10.8 & 10.9	ART. 2.4 & 3.6
С	SEC. 6.1, 6.3.1, 6.3.6, & 6.3.7	ART. 2.4 & 3.6
P		ART. 2.6 B & 2.4 G.1.b
P		ART. 1.5
P		ART. 3.3 B
Ρ		ART. 3.3 F
С	SEC. 1.2.1(e), 6.2.1 & 6.3.1	
С	SEC. 6.1.6.1.2	
Ρ		ART. 1.8 C & 1.8 D
С		ART. 3.6 B
С		ART. 3.5 & 3.6 C
С		ART. 3.3 B.9 & 3.3 F.1.b
с		ART. 1.4 B.2.a.3, 1.4 B.2.b.3, 1.4 B.2.c.3, 1.4 B.3, & 1.4 B.4

С

С

![](_page_17_Picture_11.jpeg)

![](_page_17_Figure_12.jpeg)

![](_page_17_Picture_13.jpeg)

I ABLE 1705A.3 REQUIRED SPECIAL INSPECTIONS AND TESTS OF CONCRETE CONSTRUCTION					
TYPE	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION	REFERENCED STANDARD <sup>a</sup>		
. Inspect and test reinforcement, including prestressing			ACI 318: Ch. 20,	1705	
tendons, and verify placement.	x		25.2, 25.3, 25.5.1, 26.6 1–26.6 3	191	
elements of special structural walls and coupling beams.	Α		26.13.1, 26.13.3.2,	[DSA-	
b. All other reinforcement	—	Х	26.13.3.3	190	
. Reinforcing bar welding:		v			
A706;		Л			
b. Inspect single-pass fillet welds, maximum $\frac{5}{16}$ ; not	—	Х	AWS D1.4		
c. Inspect all other welds.	х				
d. Reinforcing steel resisting flexural and axial forces in	Х		ACI 318: 18.2.8,		
intermediate and special moment frames, and boundary			25.5.7, 26.6.4, 26.13.1.4.26.13.3.2		
concrete and shear reinforcement.			26.13.3.3		
e. Shear reinforcement.	Х	_			
			ACI 318: 17.8.2,		
. Inspect anchors cast in concrete.	—	Х	26.7.2, 26.8.2,		
Inspect and test analysis post installed in hardened concrete			20.13.1, 20.13.3.3 ACI 219: 17.9.2.4		
members. <sup>b</sup>			26.7.2, 26.13.1,	1705	
a. Adhesive anchors installed in horizontally or upwardly	Х	—	26.13.3.2	$\begin{bmatrix} DSA \\ 1705 \end{bmatrix}$	
inclined orientations to resist sustained tension loads.		v	ACI 318: 17.8.2 26.7.2,	[DSA-	
b. Weenamear anchors and adhesive anchors not defined in 4.a.		Λ	20.13.1, 20.13.3.3	10034 5	
			ACI 218, Ch. 10	1903A.J	
Verify use of required design mix.	Х	—	26.4. 26.13.3.2	[OSHP	
			2011, 20110 1012	[DSA- 190	
Prior to <i>and during</i> concrete placement, fabricate specimens			ASTM C31	1705/	
for strength tests, perform slump and air content tests, and	Х		ASTM C172, ACI	1705A	
determine the temperature of the concrete.			318: 26.4, 26.5, 26.12	[DSA-	
Inspect concrete and shotcrete placement for proper applica-	x		ACI 318: 26.5	1705A 1905A 1	
tion techniques.	<b>A</b>		26.13, ACI 506: 3.4	190	
Verify maintenance of specified curing temperature and		х	ACI 318: 26.5.3-		
techniques.			26.5.5, 26.13.3.3		
a Application of prestressing forces: and	x		ACI 318: 26.10.2,		
b. Grouting of bonded prestressing tendons.	X		26.13.1, 26.13.3.2		
. Inspect erection of precast concrete members.		Х	ACI 318: 26.9,		
			26.13.1, 26.13.3.3		
for precast concrete diaphragm connections or reinforcement at ioints classified as moderate or high deformability elements					
(MDE or HDE) in structures assigned to Seismic Design Cate-			ACI 318: 26.13.1.3		
gory D, E or F, inspect such connections and reinforcement in the field for:					
a. Installation of the embedded parts	Х		ACI 550.5		
b. Completion of the continuity of reinforcement across joints.	X	—			
c. Completion of connections in the field.	Χ				
connections for compliance with ACI 550.5.	—	Х	ACI 318: 26.13.1.3		
. Verify in-situ concrete strength, prior to stressing of tendons in			ACT 219, 26 10 2		
post-tensioned concrete and prior to removal of shores and	—	Х	ACI 318: 20.10.2, 26.11.2, 26.13.3.3		
forms from beams and structural slabs.					
. Inspect formwork for shape, location and dimensions of the concrete member being formed.	—	Х	ACI 318: 26.11.1.2(b), 26.13.3.3	IDSA-	
SI: 1 inch = $25.4 \text{ mm}$				12.511	
Where applicable, see Section 1705A.13.					
Specific requirements for special inspection shall be included in the re-	esearch report fo	or the anchor is	sued by an approved sour	rce in acco	
ACI 318, or other qualification procedures. Where specific requirement	nts are not provi	ded, special in	spection requirements sha	all be spec	
lesign professional and shall be approved by the building official prio	or to the commen	ncement of the	work.	C . J A JI.	
nsialialion of all adhesive anchors in norizontal and upwardly inclu	nea positions sh	au de perform	ea by an ACI/CKSI Certi	ijiea Aane	

California Building Code 2022 (Vol 1 & 2)

1705A.3 Concrete Construction

![](_page_18_Figure_9.jpeg)

![](_page_18_Picture_10.jpeg)

![](_page_19_Figure_0.jpeg)

![](_page_19_Picture_6.jpeg)