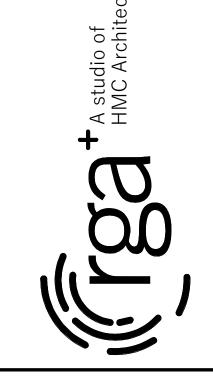


SPECIAL ELEVATION REFERENCE

WALL SECTION REFERENCE

1 Section Number

DIV. OF THE STATE ARCHITE APP: 02-119978 INC: REVIEWED FOR SS 🗹 FLS 🗹 ACS 🗹





ALICI

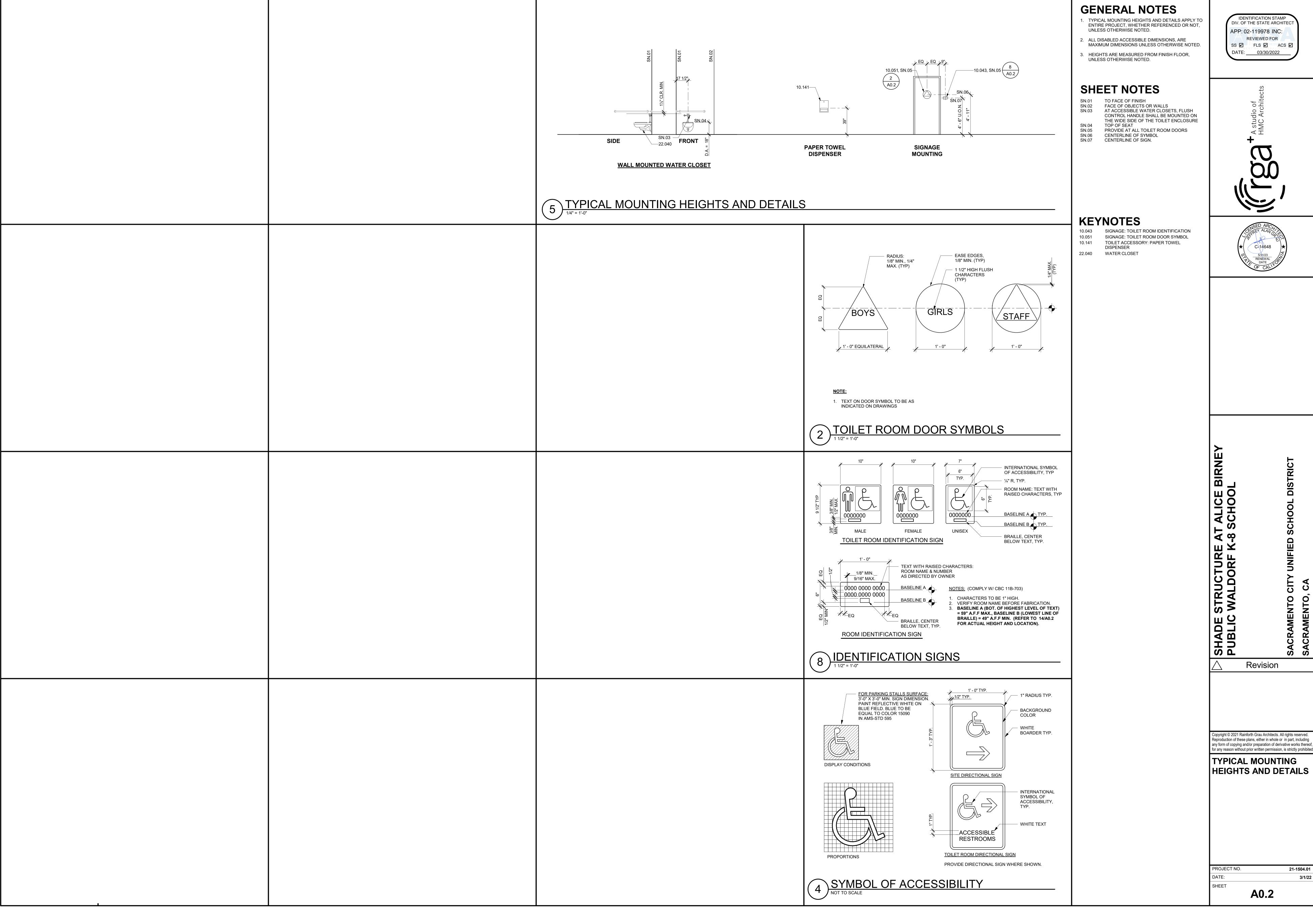
TU ADI BLI

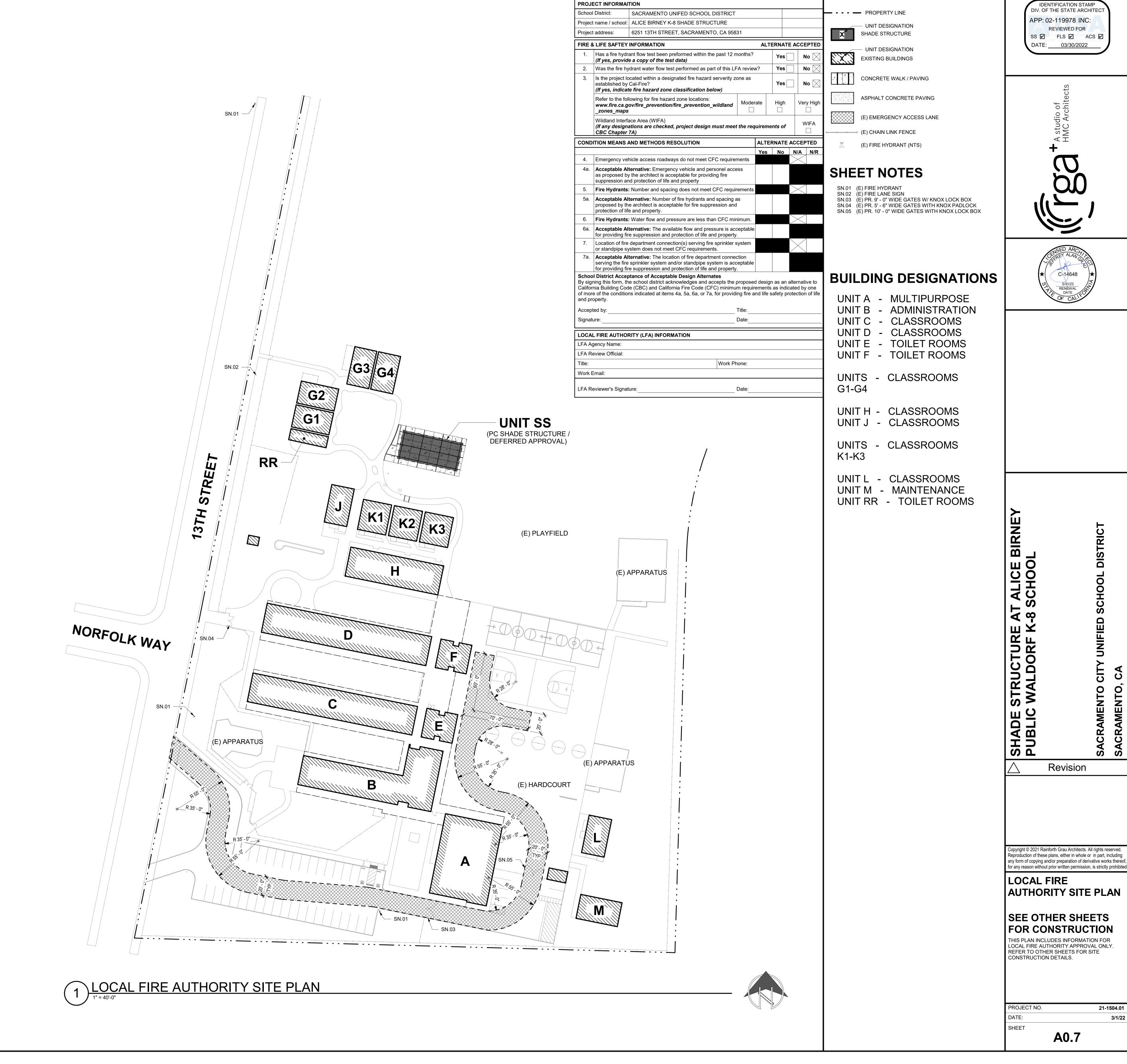
Revision

Copyright © 2019 Rainforth Grau Architects. All rights reserved. any form of copying and/or preparation of derivative works thereof, for any reason without prior written permission, is strictly prohibited COVER SHEET

Vicinity Map:

SP



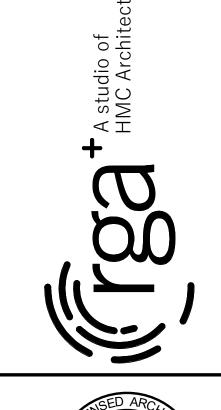


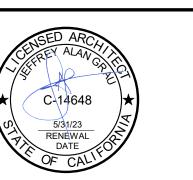
DSA-810

FIRE & LIFE SAFETY SITE CONDITIONS SUBMITTAL

LEGEND

IDENTIFICATION STAMP DIV. OF THE STATE ARCHITEC APP: 02-119978 INC: SS 🗹 FLS 🗹 ACS 🗹

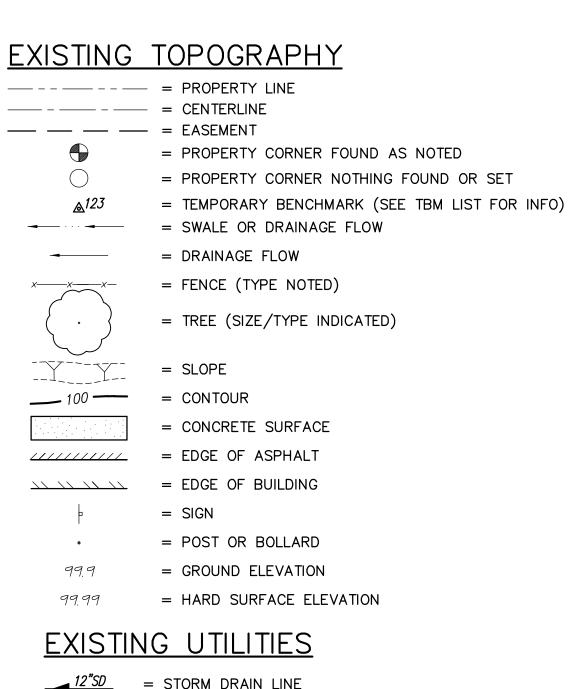




Copyright © 2021 Rainforth Grau Architects. All rights reserved.

AUTHORITY SITE PLAN

SEE OTHER SHEETS FOR CONSTRUCTION



= STORM DRAIN LINE (SIZE & DIRECTION OF FLOW) ______12"SD__ = STORM DRAIN LINE (RECORD INFORMATION) _ = STORM DRAIN LINE (UNDERGROUND LOCATING) = STORM DRAIN MANHOLE = STORM DRAIN CLEANOUT = DROP INLET = AREA DRAIN = RAIN WATER LEADER = DOWNSPOUT = SANITARY SEWER LINE (SIZE & DIRECTION OF FLOW) ________________ = SANITARY SEWER LINE (RECORD INFORMATION) = SANITARY SEWER LINE (UNDERGROUND LOCATING) = SANITARY SEWER MANHOLE = SANITARY SEWER CLEANOUT = WATER LINE (SIZE INDICATED) - -W - -W = WATER LINE (RECORD INFORMATION)-W- - W = WATER LINE (UNDERGROUND LOCATING)= WATER MANHOLE = WATER VALVE = WATER METER = WATER BOX = IRRIGATION CONTROL VALVE = FIRE HYDRANT = BACKFLOW PREVENTER = SPRINKLER = HOSE BIBB -OH-E-- = OVERHEAD ELECTRIC LINE ——E—— = UNDERGROUND ELECTRIC LINE ---E--- = UNDERGROUND ELECTRIC LINE (RECORD INFORMATION) = ELECTRIC MANHOLE = UTILITY POLE (WITH GUY WIRE) = ELECTRIC METER

= ELECTRIC BOX

= FLOOD LIGHT

--- G ---- = GAS LINE (SIZE INDICATED)

= GAS MANHOLE

= GAS VALVE

= GAS METER

--- T --- = TELEPHONE LINE

---G--- = GAS LINE (RECORD INFORMATION)

--G--= GAS LINE (UNDERGROUND LOCATING)

 $---\tau$ = TELEPHONE LINE (RECORD INFORMATION)

= STORM DRAIN BOX

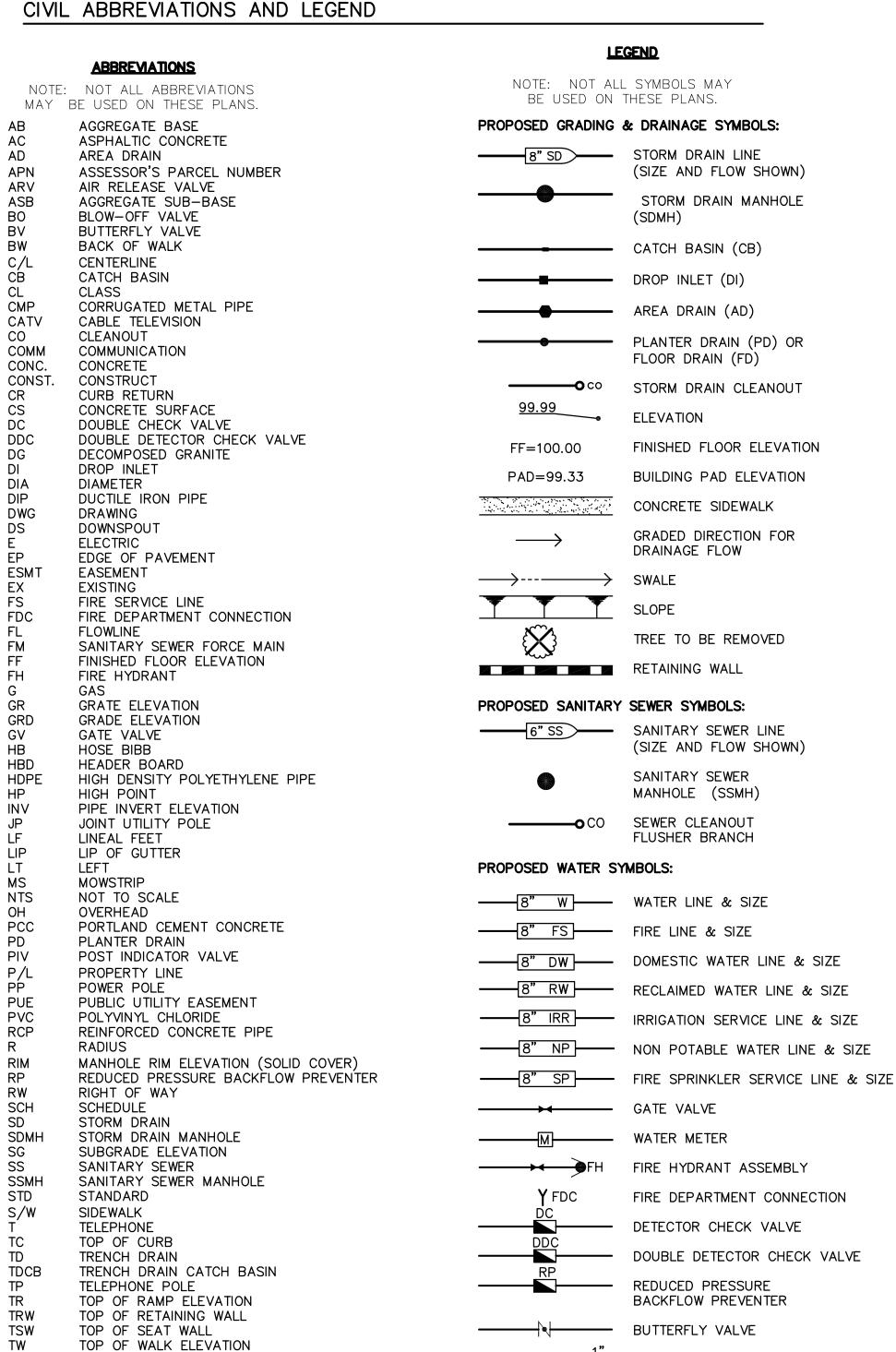
= TRAFFIC SIGNAL BOX

-- τ -- = TELEPHONE LINE (UNDERGROUND LOCATING)

 \square \square \square \square \square \square = LIGHT STANDARD

= STREET LIGHTING BOX

= ELECTRICAL OUTLET



UTILITY

WATER

WITHOUT WATER VALVE

WITH

UNDERGROUND

UNLESS OTHERWISE NOTED

VITRIFIED CLAY PIPE

AIR RELEASE VALVE + SIZE

BLOW-OFF VALVE + SIZE

POST INDICATOR VALVE

DEMOLITION GENERAL NOTES

- IN THE EVENT THAT ANY UNUSUAL CONDITIONS NOT COVERED BY THE GEOTECHNICAL INVESTIGATION REPORT OR ARE ENCOUNTERED DURING GRADING OPERATIONS THE GEOTECHNICAL ENGINEER AND THE ARCHITECT SHALL BE IMMEDIATELY NOTIFIED FOR DIRECTIONS.
- 2. NO BURNING OR BLASTING SHALL BE PERMITTED.
- ADDITIONAL DEMOLITION INFORMATION MAY BE SHOWN ON THE GRADING, DRAINAGE, AND UTILITY PLANS, AND THOSE PLANS PREPARED BY OTHER DISCIPLINES FOR THIS PROJECT.
- 4. ALL DEMOLISHED ITEMS SHALL BE DISPOSED OF OFFSITE AT A SUITABLE, LEGAL, DUMP SITE OR OTHER FACILITY.
- 5. ALL DISPOSED OF MATERIALS SHALL BE RECYCLED IF POSSIBLE
- 6. THE TYPES, LOCATIONS, SIZES AND/OR DEPTHS OF EXISTING UNDERGROUND UTILITIES AS SHOWN IN THESE PLANS WERE OBTAINED FROM SOURCES OF VARYING RELIABILITY. THE CONTRACTOR IS CAUTIONED THAT ONLY ACTUAL EXCAVATION WILL REVEAL THE TYPES, EXTENT, SIZES, LOCATIONS, AND DEPTHS OF SUCH UNDERGROUND UTILITIES. A REASONABLE EFFORT HAS BEEN MADE TO LOCATE AND DELINEATE ALL KNOWN UNDERGROUND UTILITIES. HOWEVER, WARREN CONSULTING ENGINEERS CAN ASSUME NO RESPONSIBILITY FOR THE COMPLETENESS OR ACCURACY OF ITS DELINEATION OF SUCH UNDERGROUND UTILITIES, NOR FOR THE EXISTENCE OF OTHER BURIED OBJECTS OR UTILITIES WHICH MAY BE ENCOUNTERED BUT WHICH ARE NOT SHOWN ON THESE DRAWINGS. THE CONTRACTOR OR ANY SUBCONTRACTOR FOR THIS CONTRACT SHALL NOTIFY THE DISTRICT TWO (2) WORKING DAYS IN ADVANCE OF PERFORMING ANY EXCAVATION WORK IN ORDER TO VERIFY TO THE GREATEST EXTENT POSSIBLE THE EXISTING UTILITY LINES, CONFLICTS AND PROPOSED UTILITY CONNECTION POINTS.
- 7. THE SCHOOL DISTRICT SHALL HAVE SALVAGE RIGHTS TO ANY DEMOLISHED ITEMS SHOWN HEREON. THE CONTRACTOR SHALL GIVE THE DISTRICT NOTICE 7 DAYS PRIOR TO THE START OF DEMOLITION. THE DISTRICT SHALL MOVE ANY RETAINED ITEMS OUT OF THE CONTRACTORS WORK AREA, UNLESS ANOTHER ARRANGEMENT IS MADE WITH THE CONTRACTOR. ANY REMAINING ITEMS BECOME THE PROPERTY OF THE CONTRACTOR AND SHALL BE REMOVED FROM THE SITE. ANY ITEMS NOT SHOWN FOR REMOVAL SHALL REMAIN AND SHALL BE PROTECTED FROM DAMAGE DURING CONSTRUCTION TO A REASONABLE
- 8. EXISTING UTILITY STRUCTURES IN AREAS OF NEW PAVING SHALL BE REMOVED AND REPLACED WITH NEW BOX/COVER AT NEW GRADE UNLESS SPECIFICALLY NOTED OTHERWISE.
- 9. ITEMS OUTSIDE THE LIMITS OF DEMOLITION SHALL REMAIN AND BE PROTECTED FROM DAMAGE DURING CONSTRUCTION.
- 10. EXISTING UTILITY STRUCTURES AND PIPING NOT SHOWN ON DEMOLITION PLAN TO BE REMOVED SHALL REMAIN AND BE PROTECTED.

UTILITY VERIFICATION NOTE

PRIOR TO THE START OF CONSTRUCTION, VERIFY AND POTHOLE ALL UTILITY POINTS OF CONNECTION FOR LOCATION, DEPTH. AND SIZE, I CONFLICT IS FOUND, CONTACT THE ENGINEER IMMEDIATELY FOR

IRRIGATION DEMOLITION NOTE

WITHIN LANDSCAPE AREAS TO BE DEMOLISHED THERE MAY BE EXISTING IRRIGATION LINES NOT SHOWN ON THIS PLAN. CONTRACTOR SHALL REMOVE LATERAL LINES AND HEADS ENCOUNTERED. MAIN LINES AND CONTROL WIRES MAY ONLY BE REMOVED PROVIDED THAT ROUTING IS KNOWN AND REMOVAL WILL NOT DEACTIVATE AN IRRIGATION SYSTEMS INTENDED TO REMAIN. IF CONFLICT IS FOUND, CONTACT THE ENGINEER FOR DIRECTION.

GENERAL NOTES:

 THE TYPES, LOCATIONS, SIZES, AND/OR DEPTHS OF EXISTING UNDERGROUND UTILITIES AS SHOWN ON THESE PLANS WERE OBTAINED FROM SOURCES OF VARYING RELIABILITY THE CONTRACTOR IS CAUTIONED THAT ONLY ACTUAL EXCAVATION WILL REVEAL THE TYPES, EXTENT, SIZES, LOCATIONS AND DEPTHS OF SUCH UNDERGROUND UTILITIES. A REASONABLE EFFORT HAS BEEN MADE TO LOCATE AND DELINEATE ALL KNOWN UNDERGROUND UTILITIES. HOWEVER. WARREN CONSULTING ENGINEERS CAN ASSUME NO RESPONSIBILITY FOR THE COMPLETENESS OR ACCURACY OF ITS DELINEATION OF SUCH UNDERGROUND UTILITIES, NOR FOR THE EXISTENCE OF OTHER BURIED OBJECTS OR UTILITIES WHICH MAY BE ENCOUNTERED BUT WHICH ARE NOT SHOWN ON THESE PLANS. THE CONTRACTOR OR ANY SUBCONTRACTOR FOR THIS CONTRACT SHALL NOTIFY MEMBERS OF UNDERGROUND SERVICE ALERT (USA) TWO (2) WORKING DAYS IN ADVANCE OF PERFORMING ANY EXCAVATION WORK BY CALLING TOLL FREE 1-800-227-2600, OR 811.



Call before you dig. . WARREN CONSULTING ENGINEERS, INC. (WCE) ASSUMES NO RESPONSIBILITY FOR ERRORS IN PHYSICAL LOCATION OF

- LOCATION MAY AFFECT THE INTENDED DESIGN OF SUCH IMPROVEMENTS AND WCE CANNOT BE HELD RESPONSIBLE FOR SUCH CONDITIONS WHICH ARE A RESULT OF ERRORS IN SURVEYING, OR IMPROPER CONSTRUCTION. 3. IF SUBSURFACE CULTURAL RESOURCES, REMAINS, AND/OR ARTIFACTS ARE UNCOVERED DURING PROJECT
- CONSTRUCTION, ALL WORK IN THE VICINITY SHALL BE STOPPED UNTIL SUCH ITEMS CAN BE ASSESSED BY AN APPROPRIATE MEMBER OF THE COUNTY ENVIRONMENTAL IMPACT SECTION STAFF.

IMPROVEMENTS, HORIZONTAL OR VERTICAL, ÍF STAKED BY OTHERS. IN ADDITION, ANY SUCH ERRORS IN PHYSICAL

- 4. CONTRACTOR AGREES THAT HE/SHE SHALL ASSUME SOLE AND COMPLETE RESPONSIBILITY FOR JOB SITE CONDITIONS DURING THE COURSE OF CONSTRUCTION OF THIS PROJECT, INCLUDING SAFETY OF ALL PERSONS AND PROPERTY: THAT THIS REQUIREMENT SHALL APPLY CONTINUOUSLY AND SHALL NOT BE LIMITED TO NORMAL WORKING HOURS: AND THAT THE CONTRACTOR SHALL DEFEND, INDEMNIFY AND HOLD THE OWNER AND ENGINEER HARMLESS FROM ANY AND ALL LIABILITY, REAL OR ALLEGED, IN CONNECTION WITH THE PERFORMANCE OF WORK ON THIS PROJECT, EXCEPTING FOR LIABILITY ARISING FROM THE SOLE NEGLIGENCE OF THE OWNER OR ENGINEER.
- 5. THE CONTRACTOR SHALL OBTAIN AN EXCAVATION PERMIT FROM THE STATE OF CALIFORNIA DEPARTMENT OF INDUSTRIAL SAFETY FOR ALL EXCAVATIONS OF 5 FEET OR MORE IN DEPTH.
- 6. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO MAKE ALL NECESSARY PRE-BID AND PRE-CONSTRUCTION SITE INSPECTION. AND/OR OBSERVATIONS ON THE SITE TO PRE-DETERMINE ALL HIS/HER MEANS AND METHODS NECESSARY TO COMPLETE THE IMPROVEMENTS SHOWN ON THESE PLANS AND PER THE PROJECT SPECIFICATIONS. IT IS THE CONTRACTORS RESPONSIBILITY TO DETERMINE, AND INCLUDE IN HIS/HER CONTRACT, ALL MEANS AND METHODS NECESSARY TO PERFORM A COMPLETE AND ACCEPTABLE JOB.
- 7. WHERE IMPROVEMENTS LIE WITHIN AN EXISTING DEVELOPED AREA, CONTRACTOR SHALL USE CAUTION WHEN ACCESSING THE SITE THROUGH THESE EXISTING IMPROVEMENTS. IT IS THE CONTRACTORS RESPONSIBILITY TO PROTECT ANY SUCH EXISTING IMPROVEMENTS OUTSIDE THE PROJECT BOUNDARY, OR EXISTING IMPROVEMENTS WITHIN THE BOUNDARY WHICH ARE TO REMAIN. PROPER PRECAUTIONS SHALL BE PROVIDED AND MAINTAINED THROUGHOUT CONSTRUCTION. ANY DAMAGE SHALL BE REPAIRED OR REPLACED TO THE SATISFACTION OF THE
- 8. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO KEEP DETAILED RECORDS OF MINOR CHANGES OR ADJUSTMENTS MADE DURING CONSTRUCTION (WHICH WERE NOT FORMALLY ISSUED). UPON PROJECT COMPLETION, THESE RECORDS AND/OR INFORMATION SHALL BE PROVIDED TO THE OWNER AND WARREN CONSULTING ENGINEERS, INC. UNLESS AN OFFICIAL "AS-BUILT" SET OF PLANS IS A REQUIREMENT OF THE CONTRACT. IF AS-BUILT PLANS ARE A REQUIREMENT OF THE CONTRACT, REFER TO SPECIFICATIONS FOR AS-BUILT DELIVERABLE REQUIREMENTS.
- 9. IN VEHICULAR PATHWAYS, EXISTING ASPHALTIC AND/OR CONCRETE SURFACES SHALL BE CUT TO A NEAT AND STRAIGHT LINE, PARALLEL OR PERPENDICULAR TO THE VEHICULAR TRAVELED PATH. THIS IS TYPICALLY THE ROADWAY CENTERLINE, BUT MAY VARY. THAT SAWCUT EDGE SHALL BE PROTECTED FROM DAMAGE DURING CONSTRUCTION SO A CLEAN EDGE REMAINS FOR PATCH BACK.. IF EDGE IS DAMAGED, A NEW SAW CUT WILL BE REQUIRED. THE EXPOSED EDGE SHALL BE "TACKED" WITH EMULSION PRIOR TO PAVING.
- 10. NO BURNING OR BLASTING SHALL BE ALLOWED ONSITE UNLESS SPECIFICALLY ADDRESSED ON PLANS, OR SPECIFICALLY APPROVED AND COORDINATED WITH THE ARCHITECT, ENGINEER, AND LOCAL AGENCY OR OTHER ADMINISTRATIVE AUTHORITY.
- 11. SUBGRADE AND RESULTING FINISHED GRADE SHALL BE CONSTRUCTED SMOOTH AND UNIFORM BETWEEN SPOT ELEVATIONS, CONTOURS OR OTHER STRUCTURE ELEVATIONS SHOWN ON GRADING OR OTHER PLANS. NO MOUNDS, RUTS, DEPRESSIONS OR OTHER GRADING DEFICIENCIES WILL BE ALLOWED UNLESS SPECIFICALLY SHOWN ON PLANS.
- 12. ON NEW WATER SYSTEMS, SERVICE LATERALS SHALL BE MADE USING APPROPRIATE "TEE" AND "WYE" FITTINGS. SADDLE TAPS WILL ONLY BE ALLOWED WHEN MAKING CONNECTIONS TO EXISTING WATER MAINS.
- 13. CURING COMPOUND SHALL BE APPLIED IN A CONTINUOUS SOLID WET FLOWING COAT. ANY "SPOTTY" APPLICATIONS SHALL BE RECOATED IMMEDIATELY. APPLICATION SHALL BE INSPECTED BY PROJECT INSPECTOR DURING
- APPLICATION. 14. EMBEDMENT OF FEATURES IN CONCRETE PAVING, CURBS, OR WALLS, SUCH AS SQUARE OR ROUND TUBING, POSTS, OR COLUMNS. STEEL BOLTED PLATES, OR OTHER STRUCTURES, SHALL REQUIRE ADDITIONAL SCORE OR EXPANSION JOINTS TO PREVENT UNCONTROLLED CRACKING. THOSE ADDITIONAL JOINTS MAY OR MAY NOT BE SPECIFICALLY
- SHOWN ON PLANS BUT SHALL BE PROVIDED BY THE CONTRACTOR. 15. EMBEDMENT OF FEATURES IN CONCRETE PAVING, CURBS, OR WALLS, SUCH AS SQUARE OR ROUND TUBING, POSTS, OR COLUMNS, STEEL BOLTED PLATES, OR OTHER STRUCTURES, SHALL REQUIRE A MINOR ADJUSTMENT OF REBAR WITHIN CONCRETE TO ALLOW FOR SUCH STRUCTURE. THAT REBAR ADJUSTMENT MAY NOT BE SPECIFICALLY SHOWN
- 16. NO MORE THAN 1 GALLON OF WATER PER YARD OF CONCRETE CAN BE ADDED TO THE TRUCK AFTER ARRIVAL TO PROJECT SITE. THE ADDITION OF WATER CAN ONLY BE ADDED UNDER THE SUPERVISION OF THE CONCRETE INSPECTOR OR LABORATORY TECHNICIAN.
- 17. WHEN PUMPING CONCRETE FOR PLACEMENT, ABSOLUTELY NO WATER IS TO BE ADDED TO PUMP HOPPER. ANY WATER ADDED TO HOPPER WILL BE REASON FOR CONCRETE REJECTION AT THE CONTRACTORS EXPENSE.

18. ALL CONTRACTION/CONSTRUCTION JOINTS "CJ" SHALL BE 1/4 THE SLAB THICKNESS DEEP, BUT NO LESS THAN 1"

FOR CONTROLLING OF CRACKING. CONTRACTOR SHALL EXERCISE CAUTION WHEN FINAL TROWELING OF CONCRETE SO

- AS NOT TO FILL IN THESE JOINTS WITH CONCRETE CREAM. ANY CRACKS OUTSIDE OF JOINTS WHICH WERE CONSTRUCTED LESS THAN 1" DEEP, SHALL BE CAUSE FOR CONCRETE SLAB(S) TO BE REMOVED AND REPLACE AT CONTRACTORS EXPENSE. 19. ANY SCREED BOARDS SET WITHIN CONCRETE SLABS SHALL BE AN "OVERHEAD SCREED" SO THERE IS NO
- INTERFERENCE WITH THE PLACEMENT AND ALIGNMENT OF SLAB REINFORCING.
- 20. 3-1/2" FELT JOINTS WILL NOT BE ACCEPTED. PROVIDE A FULL 4" FELT JOINT FOR 4" SLAB CONSTRUCTION, AND A 6" FELT JOINT FOR A 6" SLAB SLAB CONSTRUCTION.
- 21. SHOULD ANY SHRINKAGE CRACKS OCCUR OUTSIDE OF EITHER THE EXPANSION JOINTS OR CRACK CONTROL JOINTS, THEN THE CONCRETE SLAB SHALL BE SAWCUT AT THE NEAREST JOINTS ON EACH SIDE OF THE CRACK AND THE CONCRETE SECTION SHALL BE, REMOVED AND REPLACED. NEW CONCRETE SHALL BE DOWELED INTO EXISTING CONCRETE PER DRAWING DETAIL.
- 22. ALL AREAS DISTURBED BY GRADING OPERATIONS WHETHER SHOWN ON THE DRAWINGS OR NOT SHALL BE HYDRO SEEDED UNLESS OTHERWISE NOTED. HYDRO SEEDING SHALL CONFORM TO LOCAL CITY/COUNTY STANDARDS.
- 23. REPAIR OR PATCHING OF GALVANIZED METALS, SUCH AS AFTER WELDING GALVANIZED COMPONENTS, SHALL BE MADE USING A ZINC COMPOSITION "HOT STICK" APPLICATION PER ASTM A 780-01. GALVANIZING PAINTS WILL NOT BE ALLOWED.

GENERAL PAVING SURFACE NOTES:

- 1. PROVIDE EQUIVALENT OF MEDIUM BROOM FINISH AT SLOPES UP TO 5.99%, TYPICAL. PROVIDE EQUIVALENT OF HEAVY BROOM FINISH AT SLOPES 6% AND GREATER. REFER TO SPECIFICATIONS.
- 2. ALL NEW PEDESTRIAN WALKWAYS (NON-RAMP) SHALL BE SLOPED NO GREATER THAN 2.0%, AND NO LESS THAN 0.75% IN ANY DIRECTION, UNLESS SPECIFICALLY LABELED OTHERWISE. ALL CONCRETE SHALL MEET THE FOLLOWING SLOPE REQUIREMENTS:
- NO GREATER THAN 5% SLOPE IN THE DIRECTION OF TRAVEL.
- NO GREATER THAN 2% SLOPE CROSSING THE DIRECTION OF TRAVEL. - NO GREATER THAN 2% SLOPE IN ANY DIRECTION IN COURTYARD OR PLAZA AREAS.

CIVIL SHEET INDEX

CO.1 CIVIL GENERAL NOTES AND ABBREVIATIONS

C1.1 DEMOLITION, GRADING AND PAVING PLAN

IDENTIFICATION STAMP DIV. OF THE STATE ARCHITEC APP: 02-119978 INC: REVIEWED FOR SS 🗹 FLS 🗹 ACS 🗹 DATE: 03/30/2022





U O A O \square T S S S >

 \Box

R

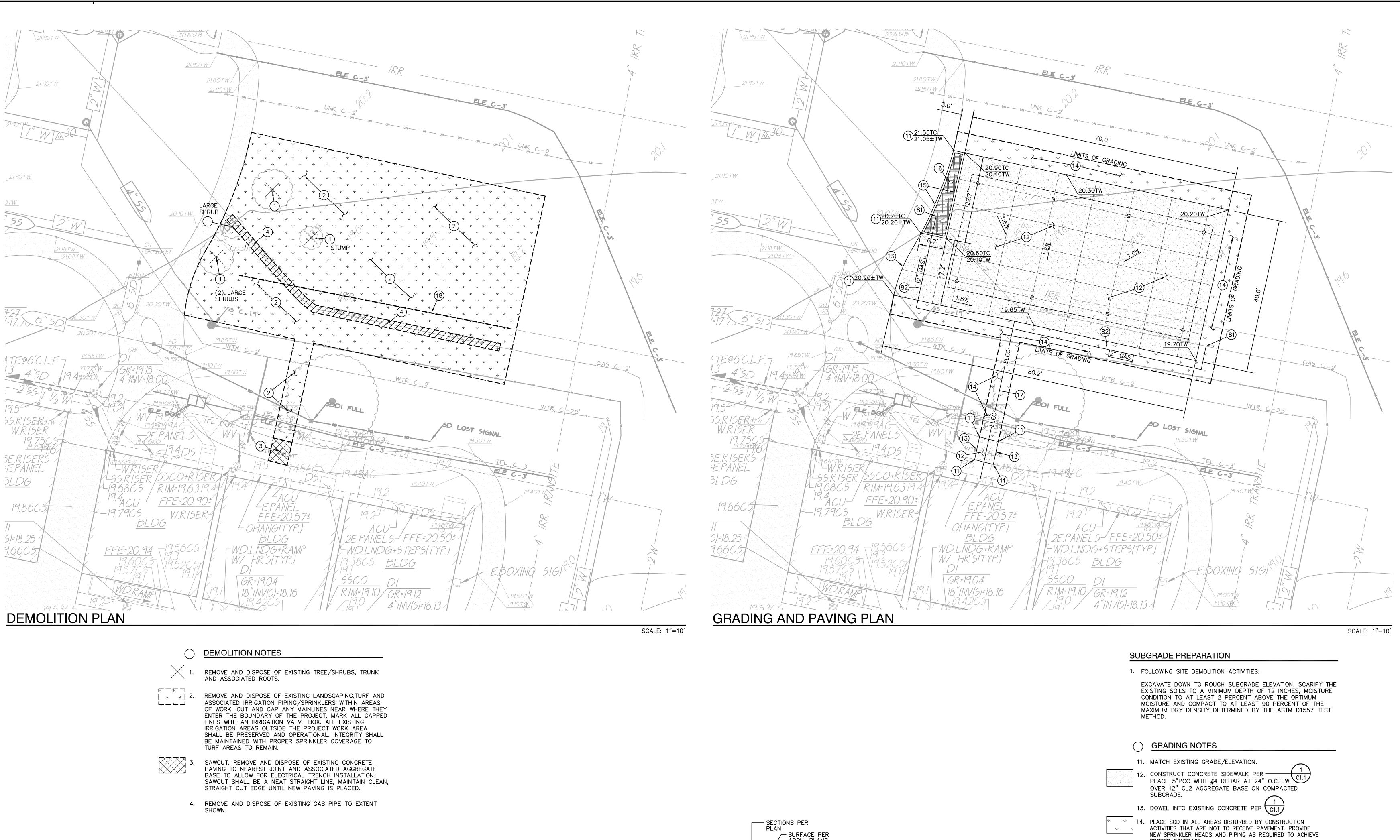
M M

ഗെ പ

Copyright © 2021 Rainforth Grau Architects. All rights reserved. Reproduction of these plans, either in whole or in part, including any form of copying and/or preparation of derivative works thereof, for any reason without prior written permission, is strictly prohibited.

C

CIVIL GENERAL NOTES AND ABBREVIATIONS



1" DIA. DOWEL SPACING TO -

CONCRETE PAVING

(NEW OR EXIST) PER PLAN

2" MIN.¬

TOOLED EDGE

₩4 BAR CONTINUOUS

PROVIDE FELT EXPANSION JOINTS (E.J.) AT 60 FEET O.C.

MAXIMUM, EXCEPT WHEN PLACING ADJACENT TO CONCRETE

MAXIMUM PROVIDE CONTROL JOINTS AT 10 FEET O.C.

WALKS THE EXPANSION JOINTS SHALL ALIGN WITH THE

EXPANSION JOINTS SHOWN FOR THE CONCRETE WALKS.

2. AT E.J. USE 1/2"X24" SMOOTH DOWELS, ALIGN WITH REBAR,

CONCRETE CURB

GREASE 1/2 THE LENGTH BEFORE CONCRETE PLACEMENT.

BARK/MULCH

NO SCALE

C1.1

MATCH PAVING. 12" DOWEL

EMBED 3" INTO CURB.

DEPTH OF BURY FOR GAS PIPE TO BE 30"MIN.

6" O.D. 6"

SURFACING PER PLANS

INTERMEDIATE BACKFILL SUITABLE NATIVE MATERIAL COMPACTED TO

90% RELATIVE COMPACTION.

COMPACTED SAND TO 90% RELATIVE COMPACTION

SOLDER ALL CONNECTIONS

4" BEDDING SAND LIGHTLY

-#10 THW SOLID COPPER TRACER

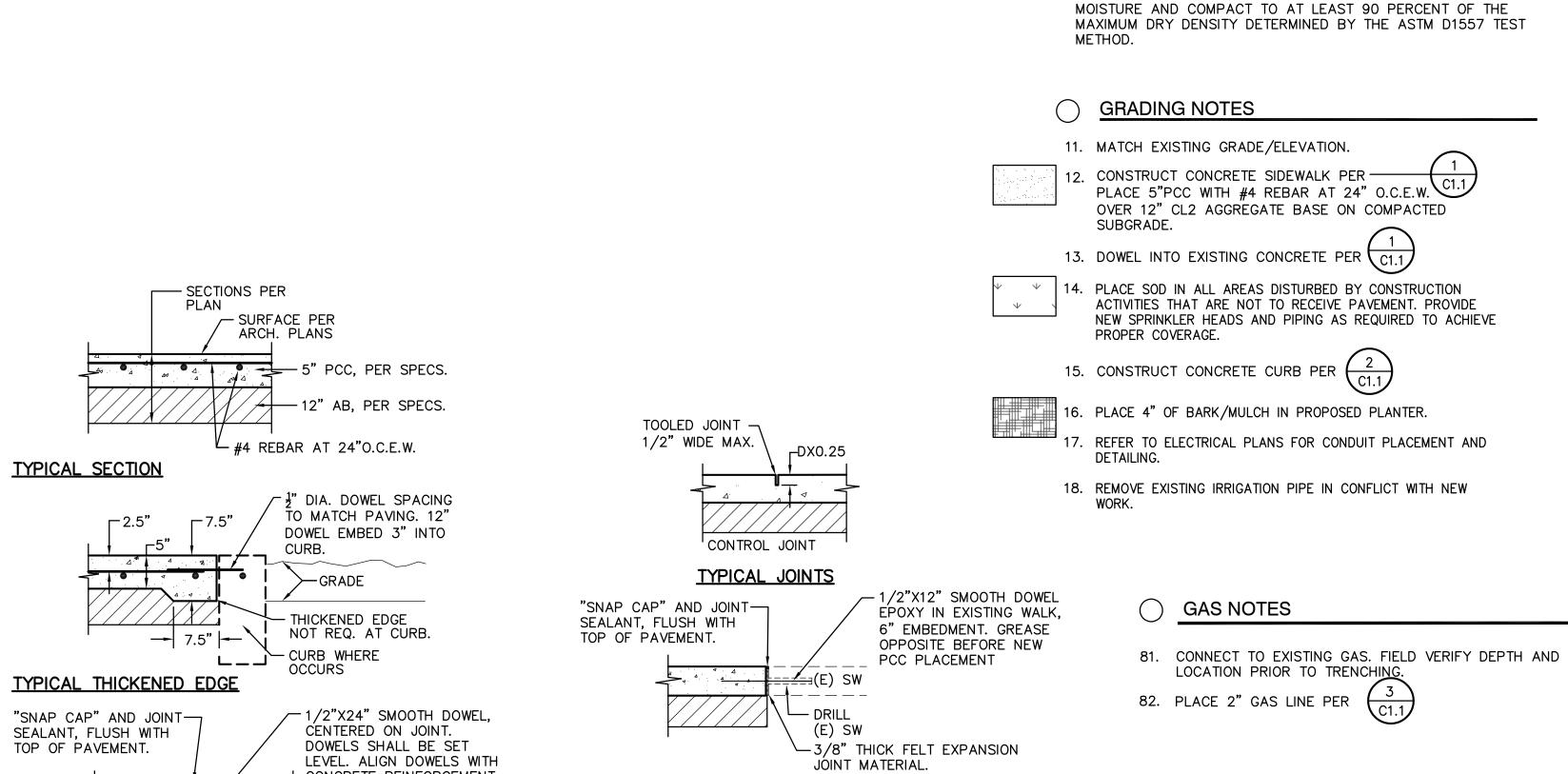
NO SCALE

WIRE FOR NON-METALLIC PIPE

INITIAL BACKFILL-

COMPACTED

GAS TRENCH



JOINT MATERIAL.

NO SCALE

CONNECTION TO (E) CONCRETE

1. PROVIDE FELT EXPANSION JOINTS AT 20 FEET O.C. MIN.

PROVIDE CONTROL JOINTS AT 10 FEET O.C. MIN.

1/2" IN SURFACE WIDTH.

3. EXPANSION OR CONTROL JOINTS SHALL NOT EXCEED

CONCRETE REINFORCEMENT

GREASE ONE END BEFORE

AND TIE TO REINFORCEMENT.

CONCRETE PLACEMENT.

-3/8" THICK FELT EXPANSION

JÓINT MATERIAL.

CONCRETE SIDEWALK

GRAPHIC SCALE PROJECT NO. THIS DRAWING MAY HAVE BEEN ENLARGED OR REDUCED.

APP: 02-119978 INC: REVIEWED FOR SS 🗹 FLS 🗹 ACS 🗹

DATE: 03/30/2022

1117 WINDFIELD WAY, SUITE 110 EL DORADO HILLS, CA 95762 | (916) 985-1870

BIRN

U Ŏ

ALI

▼ ∞

TURI ORF

STRUC' WALDO

ADE BLIC

S

ACR.

Copyright © 2021 Rainforth Grau Architects. All rights reserved.

Reproduction of these plans, either in whole or in part, including

any form of copying and/or preparation of derivative works thereof,

for any reason without prior written permission, is strictly prohibited.

DEMOLITION,

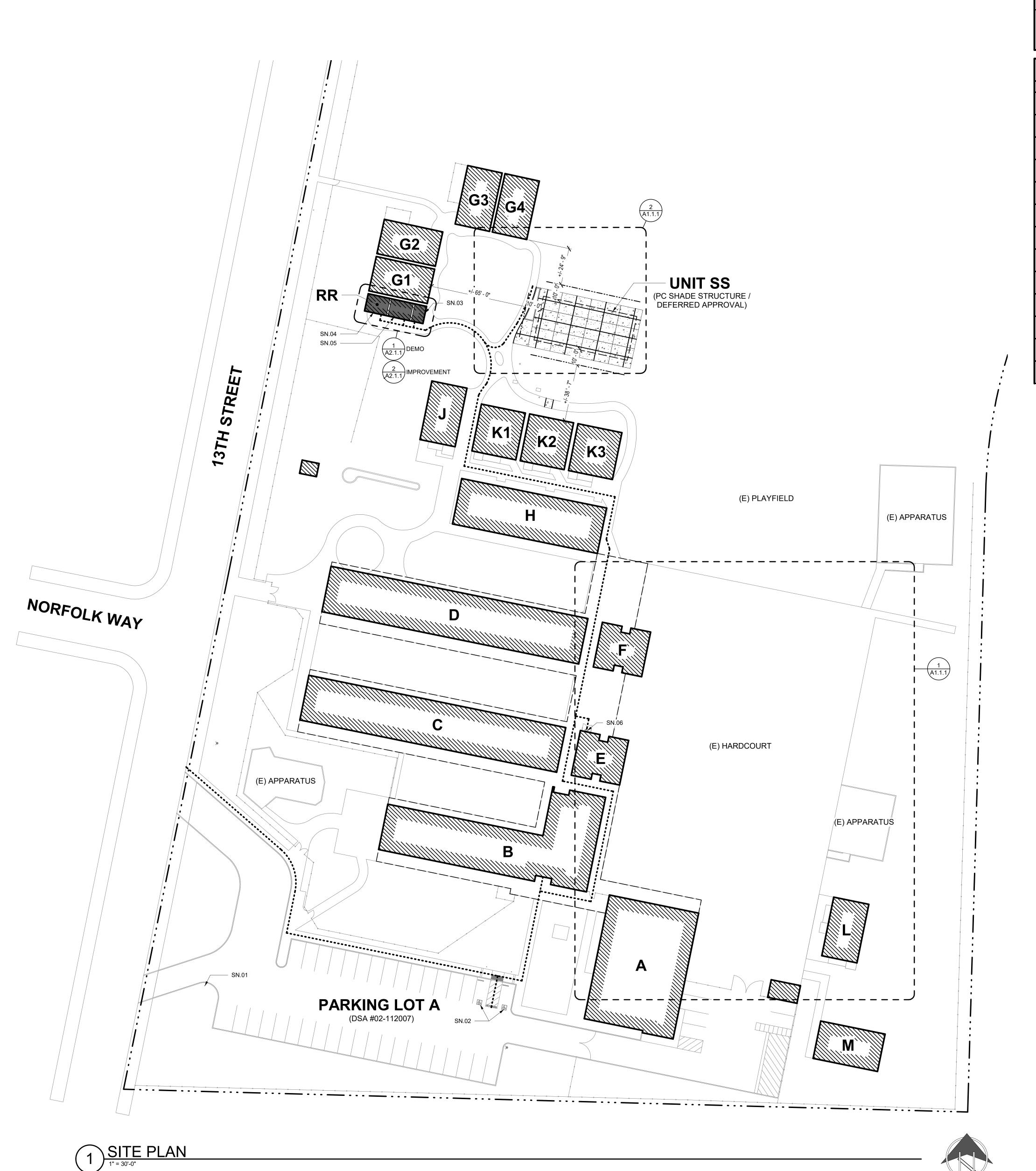
GRADING

AND PAVING PLAN

21-1504.01

3/1/22

FILENAME:I:\22-034\CIVIL\ALICE BIRNEY\DWG\22-034-C11BIRNEY.DWG



		PRO	POSED SHADE ST	RUCTURE		
UNIT	DESCRIPTION	OCCUPANCY	CONSTRUCTION TYPE	ALLOWABLE AREA (TABLE 506.2)	ACTUAL AREA	OCCUPANCY CALCULATION
SS	SHADE STRUCTURE	A-3	V-B NON-SPRINKLERED	6,000 S.F.	1,920 S.F.	1,920 S.F. / 15 NET = 128 OCC.

EXISTING BUILDING DESIGNATIONS								
UNIT	DESCRIPTION	DSA APPLICATION #	AREA (SF)	NOTES				
Α	MULTIPURPOSE	17026, 02-102142	5,187					
В	ADMINISTRATION	17026, 02-102142	5,570					
С	CLASSROOMS	17026, 02-102142	5,396					
D	CLASSROOMS	17026, 02-102142	5,396					
E	TOILET ROOMS	17026, 02-102142	970					
F	TOILET ROOMS	17026, 02-102142	970					
G1-G4	RELOCATABLE CLASSROOMS	65194, 02-107132	960 EACH					
Н	RELOCATABLE CLASSROOMS	19861	3,069					
J	RELOCATABLE CLASSROOMS	02-112460, 04-104946	960					
K1-K3	RELOCATABLE CLASSROOMS	19861	960 EACH					
L	RELOCATABLE CLASSROOMS	02-112007	960					
М	MAINTENANCE	-	1,122					
RR	TOILET ROOMS	02-101090, THIS APPLICATION	480					

EXISTING PATH OF TRAVEL (POT): ARCHITECT STATEMENT

DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE IN CHARGE STATEMENT: THE POT IDENTIFIED IN THESE CONSTRUCTION DOCUMENTS IS COMPLIANT WITH THE CURRENT APPLICABLE CALIFORNIA BUILDING CODE ACCESSIBILITY PROVISIONS FOR PATH OF TRAVEL REQUIREMENTS FOR ALTERATIONS, ADDITIONS AND STRUCTURAL REPAIRS. AS PART OF THE DESIGN OF THIS PROJECT, THE POT WAS EXAMINED AND ANY ELEMENTS, COMPONENTS OR PORTIONS OF THE POT THAT WERE DETERMINED TO BE NON-COMPLIANT 1) HAVE BEEN IDENTIFIED AND

2) THE CORRECTIVE WORK NECESSARY TO BRING THEM INTO COMPLIANCE HAS BEEN INCLUDED WITHIN THE SCOPE OF THIS PROJECT'S WORK THROUGH DETAILS, DRAWINGS, AN SPECIFICATIONS INCORPORATED INTO THESE CONSTRUCTION DOCUMENTS. ANY NONCOMPLIANT ELEMENTS, COMPONENTS OR PORTIONS OF THE POT THAT WILL NOT BE CORRECTED BY THIS PROJECT BASED ON VALUATION THRESHOLD LIMITATIONS OR A FINDING OF UNREASONABLE HARDSHIP ARE SO INDICATED IN THESE CONSTRUCTION DOCUMENTS. DURING CONSTRUCTION, IF POT ITEMS WITHIN THE SCOPE OF THE PROJECT REPRESENTED AS

CODE COMPLIANT ARE FOUND TO BE NON-CONFORMING BEYOND REASONABLE CONSTRUCTION TOLERANCES, THEY SHALL BE BROUGHT TO COMPLIANCE WITH THE CBC AS PART OF THIS

ACCESSIBLE PARKING STALL CALCULATION

PROJECT BY MEANS OF A CONSTRUCTION CHANGE DOCUMENT.

TOTAL PARKING STALL COUNT: (TABLE 11B-208.2) ACCESSIBLE PARKING STALLS REQUIRED ACCESSIBLE STALLS: 2 (26-50 TOTAL STALLS) REQUIRED VAN ACCESSIBLE STALLS: 1 (1-6 ACCESSIBLE STALLS) ACCESSIBLE STALLS PROVIDED: 1 STANDARD & 1 VAN

LEGEND

— • • • — PROPERTY LINE

---- ASSUMED PROPERTY LINE — UNIT DESIGNATION

EXISTING BUILDINGS

PC SHADE STRUCTURE / DEFERRED APPROVAL

UNIT DESIGNATION

EXPANSION JOINT

CONCRETE WALK / PAVING - CONTROL JOINT

ASPHALT CONCRETE PAVING

•••••••••••••••••••••••• ACCESSIBLE PATH OF TRAVEL SITE WALKWAYS SHALL PROVIDE A BARRIER-FREE P.O.T. ABRUPT CHANGES IN LEVEL ALONG ANY P.O.T. ARE ALLOWED UP TO 1/2". ONLY ABRUPT CHANGES IN ELEVATION UP TO 1/4" ARE ALLOWED TO HAVE A VERTICAL TRANSITION. ABRUPT CHANGES IN ELEVATION BETWEEN 1/4" AND 1/2" SHALL BE BEVELED WITH A SLOPE NO GREATER THAN 1-UNIT VERTICAL TO 2-UNITS HORIZONTAL. WALKWAYS SHALL BE FREE OF GRATINGS WHEREVER POSSIBLE. GRATING WHICH OCCUR WITHIN THE P.O.T. SHALL HAVE OPENINGS WHICH DO NOT EXCEED 1/2" IN THE DIRECTION OF TRAVEL PER CBC SECTION 11B-302.3. AN ABRUPT DROP-OFF CHANGE IN ELEVATION AT THE EDGE OF ANY WALK INTO AN ADJACENT PLANTER SHALL NOT EXCEED 4". SLOPES IN THE DIRECTION OF THE P.O.T. GREATER THAN 1-UNIT VERTICAL TO 20-UNITS HORIZONTAL SHALL BE CONSIDERED A RAMP AND WILL REQUIRE HANDRAILS ON BOTH SIDES PER CBC SECTION 11B-505 SLOPES IN THE

DIRECTION OF THE P.O.T. ALONG WALKWAYS SHALL NOT EXCEED 5%. CROSS SLOPES IN THE P.O.T. ALONG WALKWAYS SHALL NOT EXCEED 2%. ALL WALKWAYS WITHIN THE P.O.T. SHALL BE A MINIMUM OF 48" IN WIDTH. SURFACES WITH A SLOPE OF 5% OR LESS SHALL BE AT LEAST AS SLIP-RESISTANT AS THAT PROVIDED BY A LIGHT BROOM FINISH. SURFACES WITH A SLOPE OF MORE THAN 5% SHALL BE AT LEAST AS SLIP-

OBJECTS PROTRUDING INTO THE P.O.T. SHALL NOT REDUCE THE CLEAR WIDTH OR MANEUVERING SPACE WITHIN THE P.O.T. PER CBC SECTION 11B-307. PASSING SPACES (11B-403.5.3) OF 60" X 60" MIN. ARE LOCATED NOT MORE THAN 200' APART. WALKS WITH CONTINUOUS GRADIENTS SHALL HAVE 60" IN LENGTH LEVEL RESTING AREAS (11B-403.7) NOT MORE THAN 400' APART. P.O.T. SHALL BE MAINTAINED FREE OF OVERHANGING OBSTRUCTIONS TO 80" MIN (11B-307.4) AND FREE OF PROTRUDING OBJECTS (11B-307) GREATER THAN 4" PROJECTION FROM WALL ABOVE 27" AND LESS THAN 80". OBJECTS PROTRUDING INTO THE P.O.T SHALL NOT REDUCE THE CLEAR WIDTH OR MANEUVERING SPACE REQUIRED FOR ACCESSIBLE ROUTES (11B-307.5).

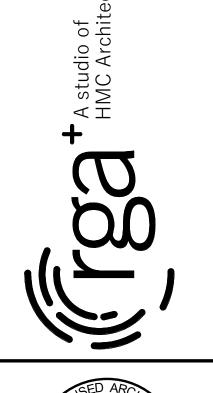
RESISTANT AS THAT PROVIDED BY A MEDIUM BROOM

SHEET NOTES

SN.01 (E) PARKING LOT ENTRANCE SIGN PER DSA #02-112007 SN.02 (E) ACCESSIBLE PARKING STALL PER DSA #02-112007 SN.03 (E) ACCESSIBLE STAFF TOILET ROOM UPGRADED

SN.05 (E) ACCESSIBLE BOY'S TOILET ROOM UPGRADED PÉR THIS APPLICATION

IDENTIFICATION STAMP DIV. OF THE STATE ARCHITECT APP: 02-119978 INC: REVIEWED FOR SS 🗹 FLS 🗹 ACS 🗹 DATE: 03/30/2022





PER THIS APPLICATION

SN.04 (E) ACCESSIBLE GIRL'S TOILET ROOM UPGRADED PER THIS APPLICATION

SN.06 (E) ACCESSIBLE DRINKING FOUNTAIN REVIEWED AND VERIFIED PER THIS APPLICATION. SEE 2/A1.1.0-1

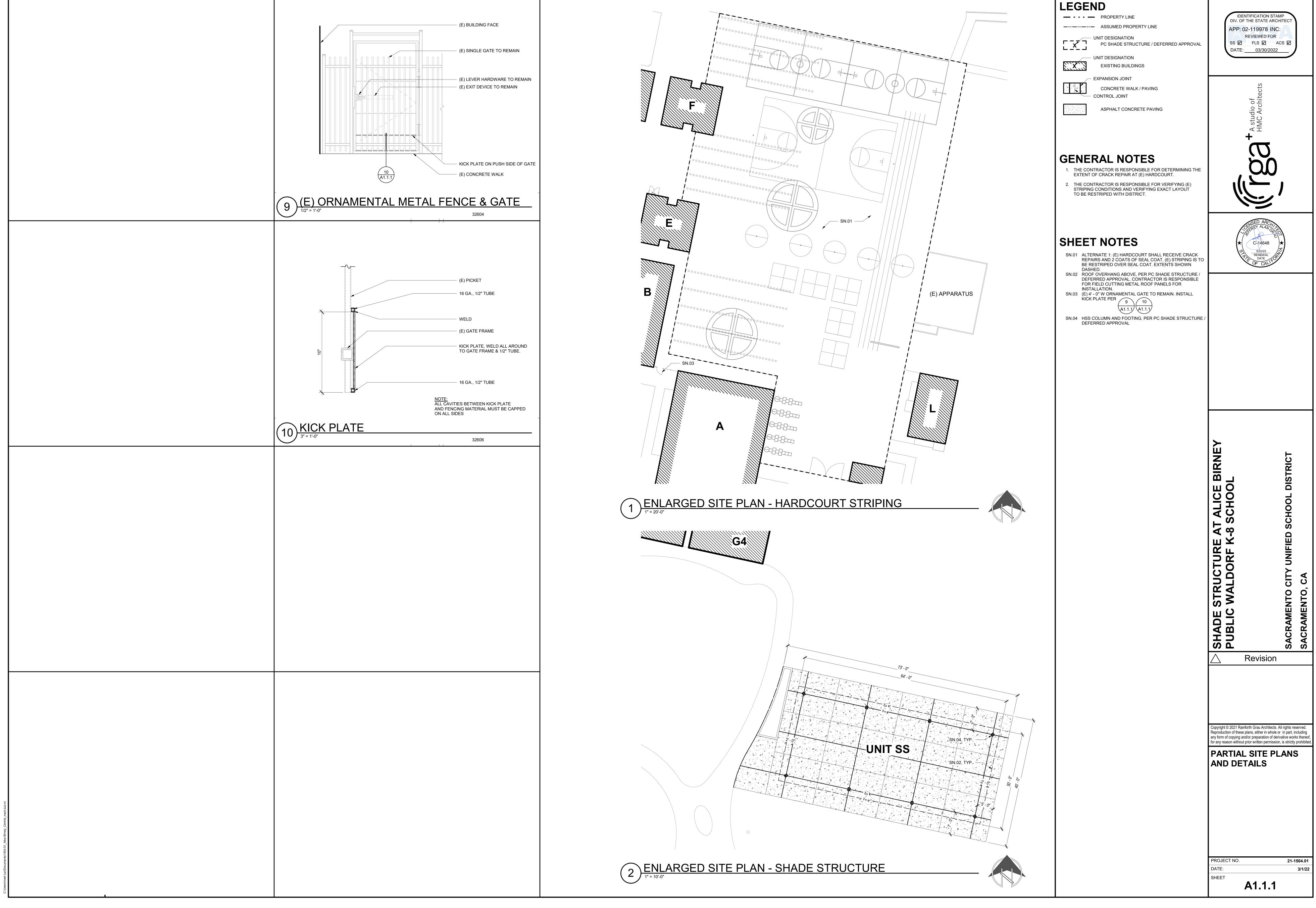
SHADI

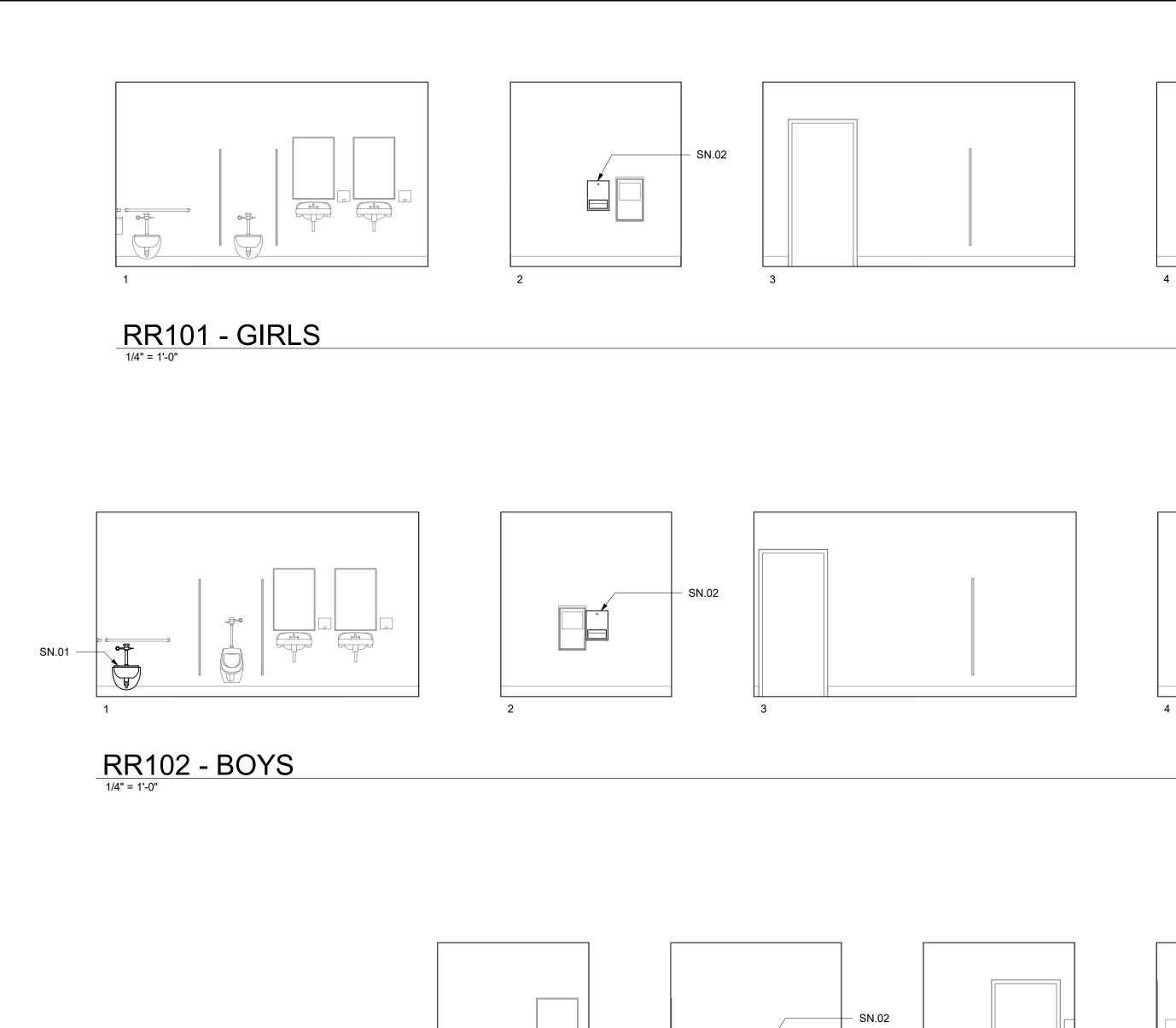
Revision

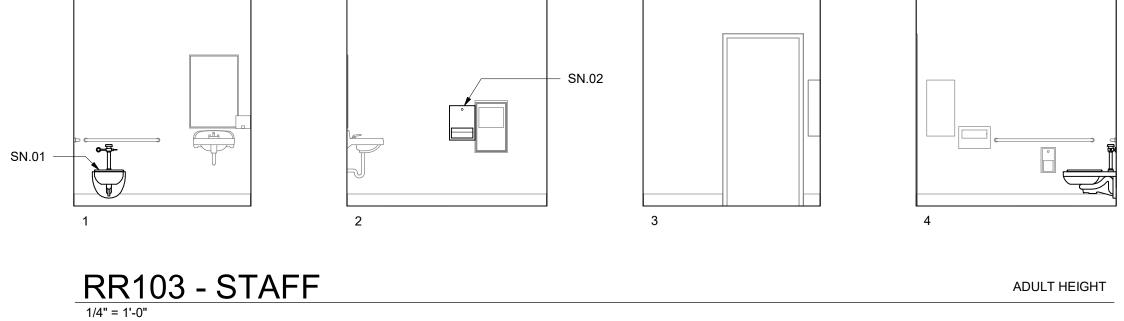
Copyright © 2021 Rainforth Grau Architects. All rights reserved. Reproduction of these plans, either in whole or in part, including any form of copying and/or preparation of derivative works thereof, for any reason without prior written permission, is strictly prohibited

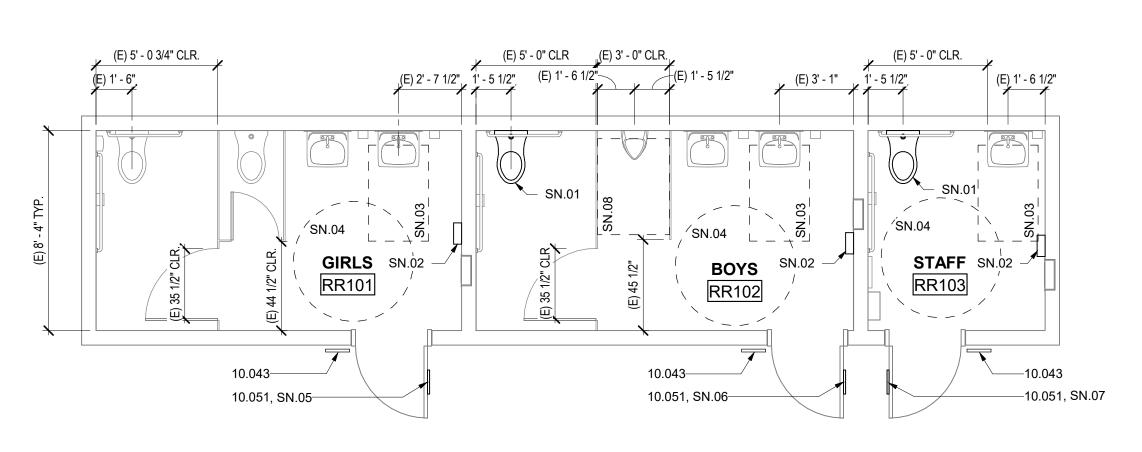
SITE PLAN AND CODE INFORMATION

A1.1.0

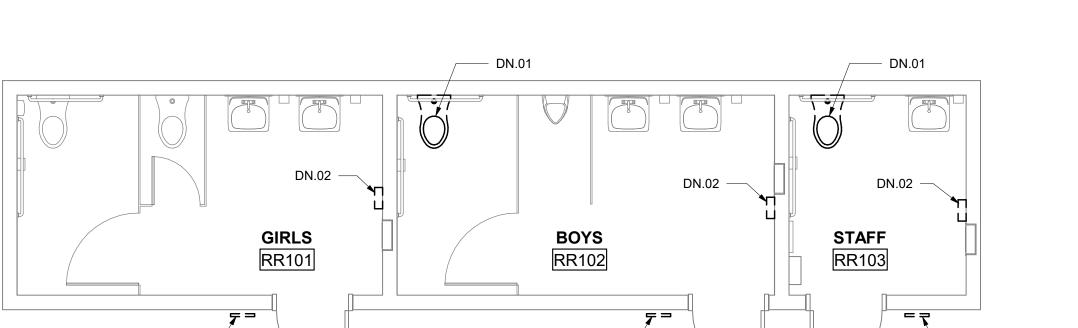












1 TOILET ROOMS - DEMOLITION

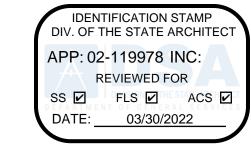


ADULT HEIGHT

ADULT HEIGHT

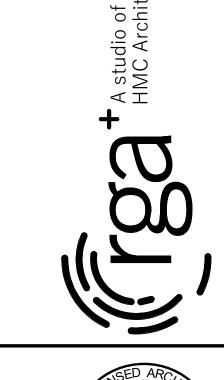
INTERIOR ELEV.

CONSECUTIVE NUMBERING CONVENTION FOR INTERIOR ELEVATIONS AND ROOM FINISHES.



GENERAL NOTES

- FOR MOUNTING HEIGHTS, LOCATIONS, AND DETAILS, INCLUDING THOSE FOR DISABLED ACCESSIBITY, REFER TO SHEET A0.2 PROTECT ALL ADJACENT SURFACES, ITEMS AND FINISHES NOT NOTED TO BE DEMOLISHED.
- EQUIPMENT/FIXTURES NOTED AS "SALVAGED FOR REINSTALLATION" WILL BE REMOVED AND STORED BY THE CONTRACTOR PRIOR TO START OF DEMOLITION. THESE EQUIPMENT/FIXTURES SHALL BE REINSTALLED BY THE CONTRACTOR UNDER THIS CONTRACT.
- REMOVE ALL ITEMS SCHEDULED TO BE REMOVED, INCLUDING MOUNTING HARDWARE.
- DEMO AND REPAIR WALL FINISH AS NECESSARY TO PERFORM FIXTURE AND EQUIPMENT WORK AS NOTED. ADJACENT FINISHES TO BE VERIFIED BY CONTRACTOR.



DEMOLITION NOTES

DN.01 REMOVE (E) WALL-MOUNTED WATER CLOSET AND SALVAGE FOR REINSTALLATION
DN.02 REMOVE (E) PAPER TOWEL DISPENSER AND SALVAGE FOR REINSTALLATION DN.03 REMOVE (E) TOILET ROOM I.D. SIGN DN.04 REMOVE (E) TOILET ROOM DOOR SYMBOL



SHEET NOTES

- SN.01 REINSTALL (E) SALVAGED WALL-MOUNTED WATER CLOSET TO COMPLY WITH A0.2. ADJUST (E) WATER CARRIER AS REQUIRED FOR RECONNECTION TO WATER CLOSET. RECONNECT TO (E) WATER LINE, WASTE LINE AND VENT.
- SN.02 REINSTALL (E) SALVAGED PAPER TOWEL DISPENSER TO COMPLY WITH A0.2 SN.03 30" X 48" CLEAR SPACE
- SN.04 60" DIA. TURNING CIRCLE
- SN.05 SIGN TO READ "GIRLS" SN.06 SIGN TO READ "BOYS"
- SN.07 SIGN TO READ "STAFF" SN.08 36" X 48" CLEAR SPACE

KEYNOTES

10.043 SIGNAGE: TOILET ROOM IDENTIFICATION 10.051 SIGNAGE: TOILET ROOM DOOR SYMBOL

SHADE (PUBLIC

Revision

Copyright © 2021 Rainforth Grau Architects. All rights reserved. Reproduction of these plans, either in whole or in part, including any form of copying and/or preparation of derivative works thereof, for any reason without prior written permission, is strictly prohibited.

TOILET ROOM DEMOLITION AND IMPROVEMENT PLANS AND INTERIOR ELEVATIONS

UNIT RR

A2.1.1

ABBREVIATION LIST AMPERE ALTERNATING CURRENT AIR CONDITIONING ARC ENERGY REDUCTION AMP FRAME ABOVE FINISHED FLOOR AMPERES INTERRUPTING CAPACITY AMP TRIP SETTING AMERICAN WIRE GAUGE BARE COPPER BELOW FINISHED CEILING BREAKER BLDG BUILDING **BOOSTER POWER SUPPLY** CONDUIT CIRCUIT BREAKER CONTRACTOR FURNISHED. CONTRACTOR INSTALLED CIRCUIT CEILING CONDUIT ONLY, WITH PULL LINE CONT CONTINUOUS METALLIC COLD WATER PIPE DEMOLISH DIRECT CURRENT DISCONNECT DISTRIBUTION PANEL EXISTING EACH WITH **EVENING LIGHT** ELECTRIC EMERGENCY ELECTRICAL METALLIC TUBING END OF LINE DEVICE **EQUIPMENT** EXISTING RELOCATED ELECTRICAL WATER COOLER ELECTRIC WATER HEATER FIRE ALARM CONTROL PANEL FAEP FIRE ALARM EXTENDER PANEL FATC FIRE ALARM TERMINAL CABINET FURNISHED BY OTHERS **FLUOR** FLUORESCENT GROUND FAULT CIRCUIT INTERRUPT GENERAL LIGHTING ZONE METALLIC GAS PIPE GYPSUM HIGH INTENSITY DISCHARGE HORSE POWER HEIGHT HERTZ INTERMEDIATE METALLIC CONDUIT SHORT CIRCUIT CURRENT (RMS SYMMETRICAL) ISOLATED J-B0X JUNCTION BOX THOUSAND CIRCULAR MILLS KCMIL KILO VOLT AMP KILOWATT LIGHTING CONTROL PANEL LOW VOLTAGE THOUSAND CIRCULAR MILLS MECHANICAL MAIN DISTRIBUTION PANEL METAL HALIDE MISCELLANEOUS MAIN LUGS ONLY MAIN POINT OF ENTRY MAIN SWITCHBOARD NOT IN CONTRACT NOT IN ELECTRICAL SECTION OF THESE PLANS & SPECS. NIGHT LIGHT NUMBFR NOT TO SCALE ON CENTER OFCI OWNER FURNISHED, CONTRTRACTOR INSTALLED OFOI OWNER FURNISHED, OWNER INSTALLED PULL BOX PROVISION FOR FUTURE BREAKER W/ PFB MOUNTING HARDWARE PRIMARY DAYLIT ZONE PROVISION FOR FUTURE CURRENT TRANSFORMER PHASE PLYWOOD PLYWD PANEL PNLPAIR POLYVINYL CHLORIDE CONDUIT PVC RELOCATE / RELOCATED REQUIRED ROOM RIGID METAL CONDUIT REMOVE AND REPLACE SECONDARY DAYLIT ZONE SKYLIGHT DAYLIT ZONE SPEC SPECIFICATION SIGNAL TERMINAL CABINET SQUARE SWITCH TELEPHONE TELECOMMUNICATIONS GROUNDING TELECOMMUNICATIONS MAIN GROUNDING BUSBAR TELEPHONE TERMINAL BOARD TYPICAL UNDERGROUND UNLESS OTHERWISE NOTED UON VOLTS WEATHERPROOF WEIGHT WATT

TRANSFORMER

GENERAL NOTES

- 1. PLANS ARE NOT FOR CONSTRUCTION UNTIL APPROVED BY THE AUTHORITY HAVING JURISDICTION. THE CONTRACTOR SHALL NOT ORDER ANY MATERIALS OR INSTALL ANY EQUIPMENT, PIPING, ETC. UNTIL PLANS ARE APPROVED BY THE AUTHORITY HAVING JURISDICTION.
- 2. ALL WORK SHALL BE DONE AT SUCH TIME AND IN SUCH MANNER AS PRESCRIBED BY THE SCHOOL'S REPRESENTATIVE.
- 3. PROTECT EXISTING EQUIPMENT AND FURNISHINGS FROM ANY DAMAGE DUE TO DUST, MOISTURE OR CONTACT WITH WORK CREW OR MATERIALS.
- 4. THE SCHOOL SHALL BE NOTIFIED AT LEAST FORTY-EIGHT (48) HOURS IN ADVANCE OF ANY POWER SHUTDOWN OF EXISTING PANELS OR SERVICE. SCHEDULE OF SHUTDOWNS SHALL BE AT CONVENIENCE OF THE SCHOOL. THE SCHOOL MAY, AT THEIR OPTION, HAVE A REPRESENTATIVE PRESENT DURING SHUTDOWN. ALL WORK REQUIRING SHUTDOWNS OF EXISTING PANELS OR SERVICE SHALL BE DONE BETWEEN 12:00 AM MIDNIGHT AND 6:00AM WEEKDAYS OR ON SATURDAY AND SUNDAY. REQUIRED SHUTDOWNS SHALL BE KEPT TO A MINIMUM.
- 5. ADEQUATELY STRAP AND SUPPORT ALL CONDUIT WORK PER CEC. IN GENERAL, SUPPORT ALL CONDUIT WITHIN THREE FEET (3') OF OUTLET BOX, CABINET OR PANEL AND MAXIMUM TEN FEET (10') ON CENTER THEREAFTER.
- 6. CORE BORE SHALL BE 1" DIAMETER LARGER THAN EACH CONDUIT. SPACE CONDUIT HOLES 3" APART. SEAL AROUND CONDUIT WITH NON-SHRINK, NON-METALLIC GROUT.
- 7. ALL CONDUCTORS INSTALLED IN PANELBOARDS SHALL BE TRAINED, LACED, AND INSTALLED WITH PHASE TAPE ON ALL CONDUCTORS. 8. LABEL DEVICES (I.E. RECEPTACLES, ETC.) ON EACH COVER PLATE IDENTIFYING CIRCUIT AND PANEL DEVICE IS CONNECTED TO.
- 9. CLEAN ALL EXTERIOR AND INTERIOR SURFACES OF PANELS AND ALL MATERIAL AND METAL SHAVINGS FROM PANEL AND CABINET INTERIORS. ALL OPENINGS SHALL BE SEALED AND APPLY TOUCH-UP SPRAY PAINT WHERE NEEDED.
- 10. FIELD COORDINATE DEVICE LOCATIONS PRIOR TO ROUGH-IN.
- 11. CONTRACTOR WILL PROVIDE WARNING LABELS NOTING THE POTENTIAL FOR ELECTRIC ARC FLASH HAZARDS PER CEC 110.16. PROVIDE LABELS ON EQUIPMENT SUCH AS SWITCHBOARDS, SWITCHGEAR, PANELBOARDS, INDUSTRIAL CONTROL PANELS, METER SOCKET ENCLOSURES, MOTOR CONTROL CENTERS, MOTOR STARTER / CONTACTOR PANELS, DISCONNECTS, ETC.. PROVIDE WARNING LABELS BY BRADY, MODEL NO. 101517, OR EQUAL, ON ALL
- 12. INSTALLATION SHALL COMPLY WITH CEC 210.4 EACH MULTIWIRE BRANCH CIRCUIT SHALL BE PROVIDED WITH A MEANS THAT WILL SIMULTANEOUSLY DISCONNECT ALL UNGROUNDED CONDUCTORS AT THE POINT WHERE THE BRANCH CIRCUIT ORIGINATES. THEREFORE ANY CIRCUIT SHARING A COMMON NEUTRAL SHALL BE CAPABLE OF SIMULTANEOUS DISCONNECT OR DEDICATED NEUTRALS SHALL BE INSTALLED.
- 13. SUPPORT ENCLOSURES, BOXES AND CONDUIT INSTALLATIONS PER CEC 314.23 (A) THROUGH (H).
- 14. SEAL CONDUIT OPENINGS THROUGH WALLS AND CEILINGS. INSTALL ESCUTCHEON PLATES AT BUILDING INTERIOR. WHERE EQUIPMENT IS INSTALLED ON THE EXTERIOR WALL, STUB CONDUITS THROUGH WALL AND SEAL CONDUIT OPENINGS, THEN INSTALL EXTERIOR EQUIPMENT. ALSO, SEAL AROUND THE PERIMETER EDGE OF THE EQUIPMENT ENCLOSURE BETWEEN THE ENCLOSURE AND BUILDING.
- 15. CONDUITS INSTALLED ON ROOF AND BUILDING EXTERIOR SHALL BE RIGID GALV. STEEL (HEAVY WALL) WITH THREADED FITTINGS. CONDUIT AND WALL TO BE PAINTED OUT TO MATCH EXTERIOR FINISH.
- 16. SPLICES AND TERMINALS SHALL BE COMPRESSION TYPE OF SEAMLESS PURE COPPER, TIN PLATED, LONG BARREL (TERMINALS WITH TWO-HOLE PAD AND INSPECTION WINDOW WITH NEMA DRILLING), AS MANUFACTURED BY BURNDY TYPE YS, YAZ-2N OR EQUAL. CLEAN ALL SURFACES AND INSTALL WITH OXIDE INHIBITING COMPOUND, BURNDY PENETROX-E OR EQUAL. APPLY COMPOUND BETWEEN BUS AND LUG PAD AND BETWEEN CONDUCTOR AND LUG BARREL. INSTALL COMPRESSION CONNECTORS WITH 360° CIRCUMFERENTIAL COMPRESSION DYE, BURNDY HYPRESS OR EQUAL. THE INDENTER OR OTHER TYPE TOOLS WILL NOT BE ACCEPTABLE.
- 17. INSTALL 'MECHANICALLY FASTENED PHENOLIC NAMEPLATE WITH WHITE LETTERING ON BLACK BACKGROUND ON ALL EQUIPMENT, INCLUDING PULL BOXES, WITH DESCRIPTION INDICATED ON DRAWINGS. NAMEPLATES SHALL READ EXACTLY AS DESCRIBED ON THE DRAWINGS. IN GENERAL NAMEPLATE LETTERING SIZE SHALL BE 3/16" HIGH FOR ALL NAMEPLATES SERVING FEEDER AND BRANCH CIRCUIT BREAKERS. ON MAIN SERVICE PANEL, DISTRIBUTION PANELS AND ALL OTHER NAMEPLATES LETTERING SHALL BE 1/4" HIGH.
- 17.1. ALL SWITCHBOARDS, SWITCHGEAR, PANELBOARDS, VFD'S, MOTORS, JUNCTION BOXES, PULL BOXES, DISCONNECT SWITCHES, ETC., SHALL BE MARKED TO INDICATE EACH DEVICE OR EQUIPMENT WHERE THE POWER ORIGINATES PER CEC 408.4, FIELD IDENTIFICATION REQUIRED, (B) SOURCE OF SUPPLY.
- 18. COORDINATE EQUIPMENT LOCATIONS, CONTROL AND POWER WIRING REQUIREMENTS AND CONNECT POINTS WITH ALL APPLICABLE DISCIPLINES.
- 19. PROVIDE AND INSTALL FUSES PER UNIT NAMEPLATE DATA ON THE EQUIPMENT PROVIDED.
- 20. A LAMINATED COPY OF THE FINAL RECORD ONE LINE DIAGRAM SHALL BE PLACED IN ELEC ROOM.
- 21. PROVIDE WIRING DEVICES AND COVER PLATES IN COLOR(S) SELECTED BY ARCHITECT. THE COLOR OF THE WIRING DEVICE AND COVER PLATE SHALL BE THE SAME UNLESS SPECIFICALLY NOTED OTHERWISE.
- 22. RECEPTACLE WEATHERPROOF COVERS SHALL BE LISTED "EXTRA DUTY", LOCAKBLE, METAL, IN-USE TYPE.
- 23. REINSTALL EXISTING ELECTRICAL INSTALLATIONS DISTURBED. CERTAIN EXISTING ELECTRICAL INSTALLATIONS MAY BE LOCATED IN WALLS, CEILINGS OR FLOORS THAT ARE TO BE REMOVED AND ARE ESSENTIAL FOR THE OPERATION OF OTHER REMAINING INSTALLATIONS. WHERE THIS CONDITIONS OCCURS, PROVIDE A NEW EXTENSION OF ORIGINAL CIRCUITS, RACEWAYS, EQUIPMENT AND OUTLETS TO RETAIN SERVICE CONTINUITY. INSTALLATIONS SHALL BE CONCEALED IN FINISHED AREAS.
- 24. FOR ROOF PENETRATIONS, REFER TO ARCHITECTURAL PLANS FOR INSTALLATION REQUIREMENTS.
- 25. FOR WALL PENETRATION INSTALLATIONS, REFER TO ARCHITECTURAL PLANS FOR REQUIREMENTS.
- 26. PROVIDE "LOCK-ON" DEVICE FOR ALL CIRCUIT BREAKERS ON EMERGENCY DEDICATED CIRCUITS.
- 27. DRAWINGS ARE TO BE CONSIDERED DIAGRAMMATIC. CONTRACTOR SHALL ACCEPT RESPONSIBILITY IN FAMILIARIZING THEMSELVES WITH ARCHITECTURAL AND STRUCTURAL CONDITIONS ALONG WITH INHERENT SPACE LIMITATIONS. WITH THAT UNDERSTANDING SHALL PROVIDE ALL ITEMS OF LABOR, MATERIALS AND TOOLS REQUIRED TO PROVIDE A COMPLETE INSTALLATION.
- 28. MAINTAIN A MINIMUM OF 12" SEPARATION BETWEEN ANY CONDUIT AND (E) UTILITY CONDUIT.
- 29. FOR INTERSECTING TRENCHED CONDUIT, MAINTAIN OR EXCEED THE MINIMUM CONDUIT DEPTH REQUIREMENTS.

MEP COMPONENT ANCHORAGE NOTE

ALL PERMANENT EQUIPMENT AND COMPONENTS.

ALL MECHANICAL, PLUMBING AND ELECTRICAL COMPONENTS SHALL BE ANCHORED AND INSTALLED PER THE DETAILS ON THE DSA APPROVED CONSTRUCTION DOCUMENTS. THE FOLLOWING COMPONENTS SHALL BE ANCHORED AND BRACED TO MEET THE FORCE AND DISPLACEMENT REQUIREMENTS PRESCRIBED IN THE 2019 CBC SECTIONS 1617A.1.18 THROUGH 1617A.1.26 AND ASCE 7-16 CHAPTERS 13, 26 AND 30:

- TEMPORARY, MOVEABLE OR MOBILE EQUIPMENT THAT IS PERMANENTLY ATTACHED (E.G. HARD WIRED) TO THE BUILDING UTILITY SERVICES SUCH AS ELECTRICITY, GAS OR WATER. "PERMANENTLY ATTACHED" SHALL INCLUDE ALL ELECTRICAL CONNECTIONS EXCEPT PLUGS FOR 110/20 VOLT RECEPTACLES HAVING A FLEXIBLE CABLE. 3. TEMPORARY, MOVEABLE OR MOBILE EQUIPMENT WHICH IS HEAVIER THAN 400 POUNDS OR HAS A CENTER OF MASS
- LOCATED 4 FEET OR MORE ABOVE THE ADJACENT FLOOR OR ROOF LEVEL THAT DIRECTLY SUPPORTS THE COMPONENT IS REQUIRED TO BE RESTRAINED IN A MANNER APPROVED BY DSA.
- THE FOLLOWING MECHANICAL AND ELECTRICAL COMPONENTS SHALL BE POSITIVELY ATTACHED TO THE STRUCTURE, BUT NEED NOT DEMONSTRATE DESIGN COMPLIANCE WITH THE REFERENCES NOTED ABOVE. THESE COMPONENTS SHALL HAVE FLEXIBLE CONNECTIONS PROVIDED BETWEEN THE COMPONENT AND ASSOCIATED DUCTWORK, PIPING, AND CONDUIT. FLEXIBLE CONNECTIONS MUST ALLOW MOVEMENT IN BOTH TRANSVERSE AND LONGITUDINAL DIRECTIONS:
- A. COMPONENTS WEIGHING LESS THAN 400 POUNDS AND HAVING A CENTER OF MASS LOCATED 4 FEET OR LESS
- ABOVE THE ADJACENT FLOOR OR ROOF LEVEL THAT DIRECTLY SUPPORTS THE COMPONENT. B. COMPONENTS WEIGHING LESS THAN 20 POUNDS, OR IN THE CASE OF DISTRIBUTED SYSTEMS, LESS THAN 5 POUNDS PER FOOT, WHICH ARE SUSPENDED FROM A ROOF OR FLOOR OR HUNG FROM A WALL.
- THE ANCHORAGE OF ALL MECHANICAL, ELECTRICAL AND PLUMBING COMPONENTS SHALL BE SUBJECT TO THE APPROVAL OF THE DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE CHARGE OR STRUCTURAL ENGINEER DELEGATED RESPONSIBILITY AND ACCEPTANCE BY DSA. THE PROJECT INSPECTOR WILL VERIFY THAT ALL COMPONENTS AND EQUIPMENT HAVE BEEN ANCHORED IN ACCORDANCE WITH THE ABOVE REQUIREMENTS.

PIPING, DUCTWORK, AND ELECTRICAL DISTRIBUTION SYSTEM BRACING NOTE

PIPING, DUCTWORK, AND ELECTRICAL DISTRIBUTION SYSTEMS SHALL BE BRACED TO COMPLY WITH THE FORCES AND DISPLACEMENTS PRESCRIBED IN ASCE 7-16 SECTION 13.3 AS DEFINED IN ASCE 7-16 SECTIONS 13.6.5, 13.6.6, 13.6.7, 13.6.8 AND 2019 CBC, SECTIONS 1617A.1.24, 1617A.1.25 AND 1617A.1.26.

THE METHOD OF SHOWING BRACING AND ATTACHMENTS TO THE STRUCTURE FOR THE IDENTIFIED DISTRIBUTION SYSTEM ARE AS NOTED BELOW. WHEN BRACING AND ATTACHMENTS ARE BASED ON A PREAPPROVED INSTALLATION GUIDE (E.G., OSHPD OPM FOR 2019 CBC OR LATER), COPIES OF THE BRACING SYSTEM INSTALLATION GUIDE OR MANUAL SHALL BE AVAILABLE ON THE JOBSITE PRIOR TO THE START OF AND DURING THE HANGING AND BRACING OF THE DISTRIBUTION SYSTEMS. THE STRUCTURAL ENGINEER OF RECORD SHALL VERIFY THE ADEQUACY OF THE STRUCTURE TO SUPPORT THE HANGER AND BRACE LOADS.

MECHANICAL PIPING (MP), MECHANICAL DUCTS (MD), PLUMBING PIPING (PP), ELECTRICAL DISTRIBUTION SYSTEMS (E): MP ☐ MD ☐ PP ☐ E ■ OPTION 1: DETAILED ON THE APPROVED DRAWINGS WITH PROJECT SPECIFIC NOTES AND DETAILS.

MP ☐ MD ☐ PP ☐ E ☐ OPTION 2: SHALL COMPLY WITH THE APPLICABLE OSHPD PRE-APPROVAL (OPM #)

SYMBOLS LIST

- FV FUSED DISCONNECT SWITCH
- □ DUPLEX CONVENIENCE OUTLET
- DOUBLE DUPLEX CONVENIENCE OUTLET GROUND FAULT CIRCUIT INTERRUPTER DUPLEX OUTLET
- GROUND FAULT CIRCUIT INTERRUPTER DOUBLE DUPLEX OUTLET
- SPECIAL OUTLET TO MATCH CAP PROVIDED WITH MACHINE
- FLUSH FLOOR BOX OR "POKE-THRU" UNIT EQUIPPED WITH FLUSH
- OR PEDESTAL DUPLEX RECEPTACLE AND VOICE/DATA OUTLETS AS NOTED, OR REFER TO SCHEDULE ON DRAWINGS.
- PLUGMOLD/WIREMOLD RECEPTACLE SYSTEM
- △ TRANSFORMER
- JUNCTION BOX, SIZE AS REQUIRED BY CODE
- FLEX CONNECTION TO FIXTURE
- PANELBOARD, RECESSED MOUNTED PANELBOARD, SURFACE MOUNTED
- MAIN SWITCHBOARD
- TERMINAL CABINET, RECESSED MOUNTED
- ☐ TERMINAL CABINET, SURFACE MOUNTED
- → HOMERUN TO PANELBOARD OR RESPECTIVE TERMINAL — III — CONDUIT RUN CONCEALED IN CEILING OR WALL, SEE SYMBOLS LIST NOTES
- ---- CONDUIT RUN UNDERGROUND OR UNDER FLOOR
- —EM— EMERGENCY SYSTEM CONDUIT AND WIRES
- INSULATED GREEN GROUND CONDUCTOR
- >> INSULATED ISOLATED GROUND CONDUCTOR, GREEN WITH TRACER STRIPE -----O CONDUIT RISER
- - EXISTING EQUIPMENT, LIGHTING, DEVICES, CONDUIT, WIRING, ETC., ARE SHOWN LIGHT. NEW OR RELOCATED EQUIPMENT, LIGHTING, DEVICES, CONDUIT, WIRING, ETC., ARE SHOWN DARK.
- X X EXISTING ELECTRICAL EQUIPMENT TO BE REMOVED
- SEE SURFACE RACEWAY SCHEDULE (1) 1> SYMBOLS REFERRING TO KEYED NOTES ON SAME SHEET
- MECHANICAL EQUIPMENT BY OTHERS, CONNECTED BY ELECTRICAL CONTRACTOR
- DETAIL DESIGNATION, "A" SIGNIFIES DETAIL, "E-1" SIGNIFIES SHEET NUMBER

WIREMOLD SURFACE RACEWAY(S) WITH OUTLETS AS SHOWN OR NOTED,

(1)1-1/2°C \leftarrow INDICATES SIZE OF CONDUIT = ONE AND ONE HALF INCH CONDUIT — NUMBER WITHIN PARENTHESIS INDICATES QUANTITY OF CONDUITS

SYMBOLS LIST NOTES:

- 1. MOUNT SWITCH BOXES AT +48" TO TOP OF BOX UNLESS OTHERWISE NOTED.
- 2. MOUNT OUTLET BOXES AT +15" TO BOTTOM OF BOX UNLESS OTHERWISE NOTED.
- "A" ADJACENT TO OUTLET INDICATES OUTLET BOX TO BE MOUNTED ABOVE COUNTER. COORDINATE WITH COUNTER HEIGHT AND DEPTH PRIOR TO ROUGH IN. MOUNT OUTLET ABOVE COUNTERS AT: 3.1. +46" MAX TO TOP OF BOX WHERE BOX IS INSTALLED OVER BASE CABINET.

3.2. +44" MAX TO TOP OF BOX WITH OPEN COUNTERS WITH FORWARD APPROACH.

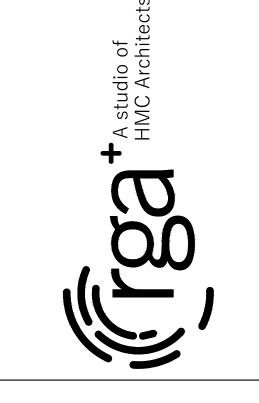
- 4. OUTLET BOXES SHALL BE:
- 4.1. WALL MOUNTED -4" SQ. $\times 2-1/8$ " DEEP MINIMUM 4.2. CEILING MOUNTED -4" SQ. OR 4" OCT. x 2-1/8" DEEP MINIMUM
- 5. OUTLET BOXES REQUIRING 1-1/4", 1-1/2" OR 2" CONDUITS SHALL BE 4-11/16" x 3-1/4" DEEP MINIMUM.
- 6. FLUSH MOUNTED OUTLET BOXES SHALL UTILIZE TRIM RINGS. COORDINATE TRIM RING DEPTH WITH WALL FINISH PRIOR TO ROUGH-IN.
- 7. NO CROSSBARS ON CONDUIT RUN INDICATES MINIMUM 1" CONDUIT, TWO #10 CU CONDUCTORS PLUS 1#10 CU GND. CROSSBARS INDICATE NUMBER OF #10 CU CONDUCTORS IN CONDUIT. CONDUCTOR SIZES OTHER THAN #10 NOTED ON DRAWINGS. INCREASE CONDUIT SIZE AS REQUIRED TO ACCOMMODATE C.E.C. WIRE FILL REQUIREMENTS. INCLUDE ADDITIONAL BOND WIRE IN ALL PVC AND FLEXIBLE CONDUIT. LONG CROSSBAR INDICATES NEUTRAL CONDUCTOR, SHORT CROSSBARS INDICATE PHASE CONDUCTORS.
- 8. INCREASE BRANCH CIRCUIT CU CONDUCTOR SIZES AS REQUIRED BY THE 120V BRANCH CIRCUIT VOLT DROP CONDUCTOR LENGTH CHART BELOW. USE CONDUCTOR LENGTHS AS FIELD MEASURED, BASED UPON MEASURED FIELD ROUTING LENGTHS. INCREASE MINIMUM CONDUIT SIZE AS REQUIRED TO ACCOMMODATE A MAXIMUM 40% CONDUCTOR FILL OF THE BRANCH CIRCUIT CONDUCTORS. WHERE NECESSARY, PROVIDE A JUNCTION BOX AT ACCESSIBLE CEILING SPACE TO CONVERT THE LAST 15 FEET OF CONDUCTORS TO #10 AWG TO ACCOMMODATE TERMINATION OF CONDUCTORS AT WIRING DEVICES, LIGHTING FIXTURES, CIRCUIT BREAKER, ETC.
- 9. INSTALL CU GROUND CONDUCTOR IN ALL BRANCH CIRCUITS FOR LIGHT FIXTURES AND POWER DEVICES.

120V BRANCH CIRCUIT VOLT DROP CONDUCTOR LENGTH CHART

OLI DI	0, 00		OIL PEI	idiii O	11/3111			
LOAD IN	LENGTH OF CONDUCTOR							
VOLT	WIRE SIZE IN (GAUGE)							
AMPERES	#12	#10	#8	#6	#4			
1200VA	74	121	183	284	434			
1560VA	57	93	141	218	334			
1800VA	49	81	122	189	289			
1920VA	46	76	115	178	271			
2340VA	Х	62	94	146	223			
2880VA	Х	51	76	118	181			
3000VA	Х	48	73	114	174			
3900VA	Х	Х	56	87	134			

- 4800VA X X 46 71 1. THIS CHART IS FOR COPPER CONDUCTORS ONLY.
- THIS CHART ASSUMES AN 80% POWER FACTOR AND STEEL RACEWAYS. 3. 2019 CALIFORNIA ENERGY CODE, 130.5(c) ALLOWS A MAXIMUM COMBINED VOLTAGE DROP OF 5%. THIS CHART ASSUMES A MAXIMUM DROP OF 3% FOR FEEDERS. THIS CHART PROVIDES THE MAXIMUM LENGTH OF CONDUCTORS FOR LESS THAN 2% VOLTAGE DROP ON A BRANCH
- CIRCUIT AT GIVEN VA LOAD. 4. USE WIRE SIZE FROM THIS CHART UNLESS LARGER CONDUCTOR SIZES ARE NOTED ON THE
- 5. FOR VA VALUES NOT SHOWN USE NEXT HIGHEST VALUE FROM THE CHART

IDENTIFICATION STAMP DIV. OF THE STATE ARCHITEC APP: 02-119978 INC: REVIEWED FOR SS 🗹 FLS 🗹 ACS 🗹 DATE: 03/30/2022



7750 College Town Dr. ste.10 Sacramento, CA 95826 ETERS | Tel (916) 447-2841 g www.peterseng.com Job no. 22.010 consulting mechanical and electrical engineers



FIED

0

С TR S >

Revision

S D

Copyright © 2021 Rainforth Grau Architects. All rights reserved.

Reproduction of these plans, either in whole or in part, including any form of copying and/or preparation of derivative works thereof, for any reason without prior written permission, is strictly prohibited.

SYMBOLS, NOTES

E0.1



consulting mechanical and electrical engineers



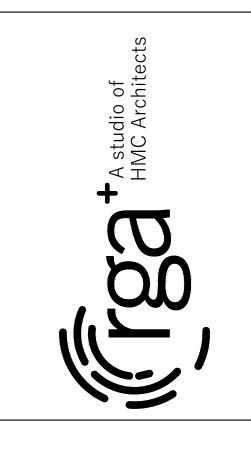
SHEET NOTES:

1. ALL EXISTING EQUIPMENT, DEVICES, CONDUIT AND WIRING, ETC., SHOWN ON PLANS ARE BASED ON AVAILABLE EXISTING DRAWINGS AND LIMITED SITE SURVEYS, AND SHOWN FOR CLARITY ONLY.

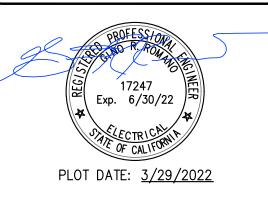
IDENTIFICATION STAMP DIV. OF THE STATE ARCHITECT APP: 02-119978 INC: REVIEWED FOR SS 🗹 FLS 🗹 ACS 🗹 DATE: 03/30/2022

KEYED NOTES:

1 MODIFIED PANEL SERVES EQUIPMENT BEING ADDED IN THIS PROJECT. SEE PANEL SCHEDULE ON THIS SHEET FOR REFERENCE.



PETERS engineering	7750 College Town Dr. ste.101 Sacramento, CA 95826 Tel (916) 447-2841 www.peterseng.com Job no. 22.010
	anical and electrical engineers



BIRNE STRUCTURE A

SHADE : Revision

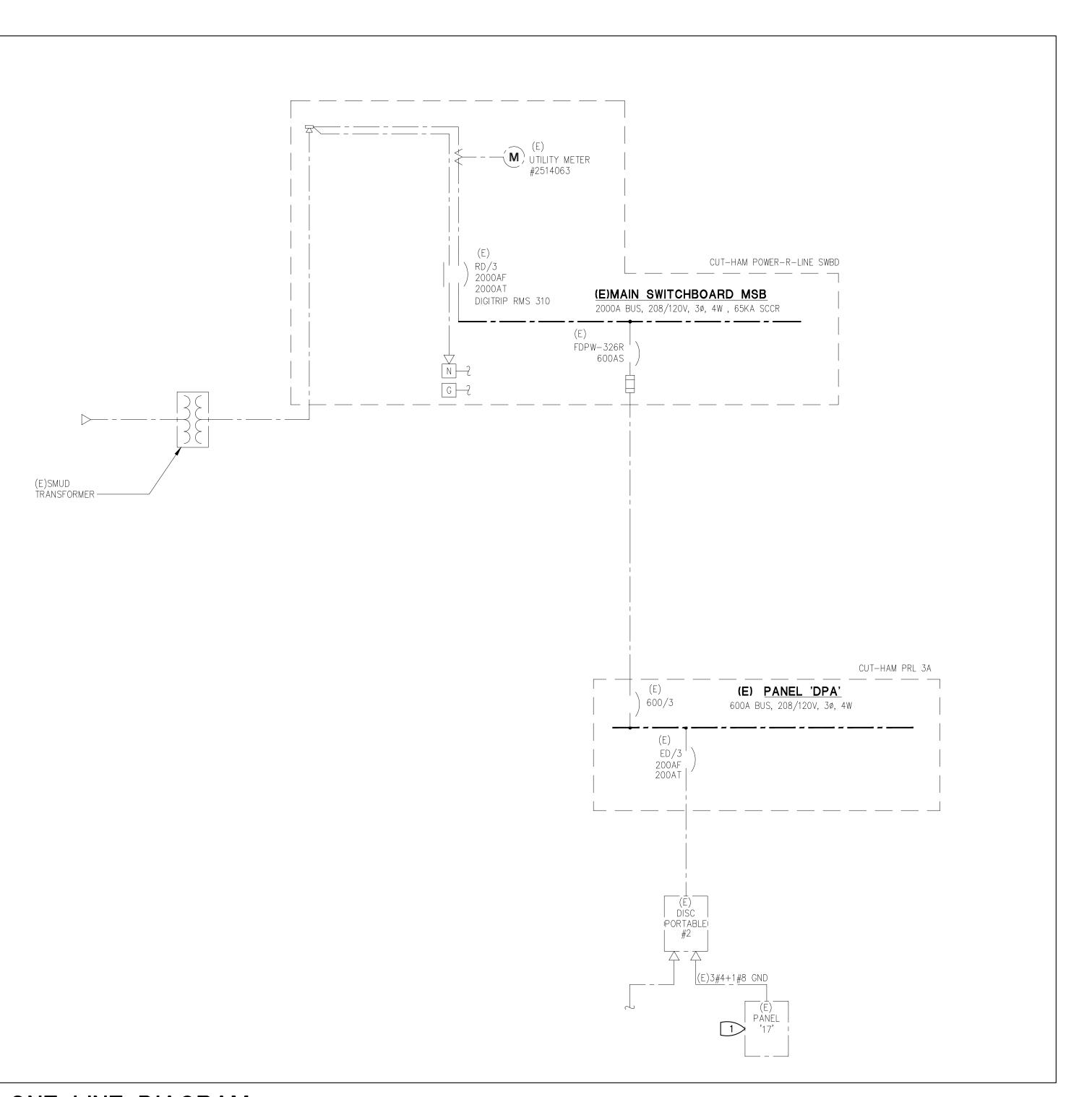
Copyright © 2021 Rainforth Grau Architects. All rights reserved. Reproduction of these plans, either in whole or in part, including any form of copying and/or preparation of derivative works thereof, for any reason without prior written permission, is strictly prohibited.

ONE LINE DIAGRAM

PROJECT NO.		21-1504.01
DATE:		3/1/22
SHEET		
	E2.1	

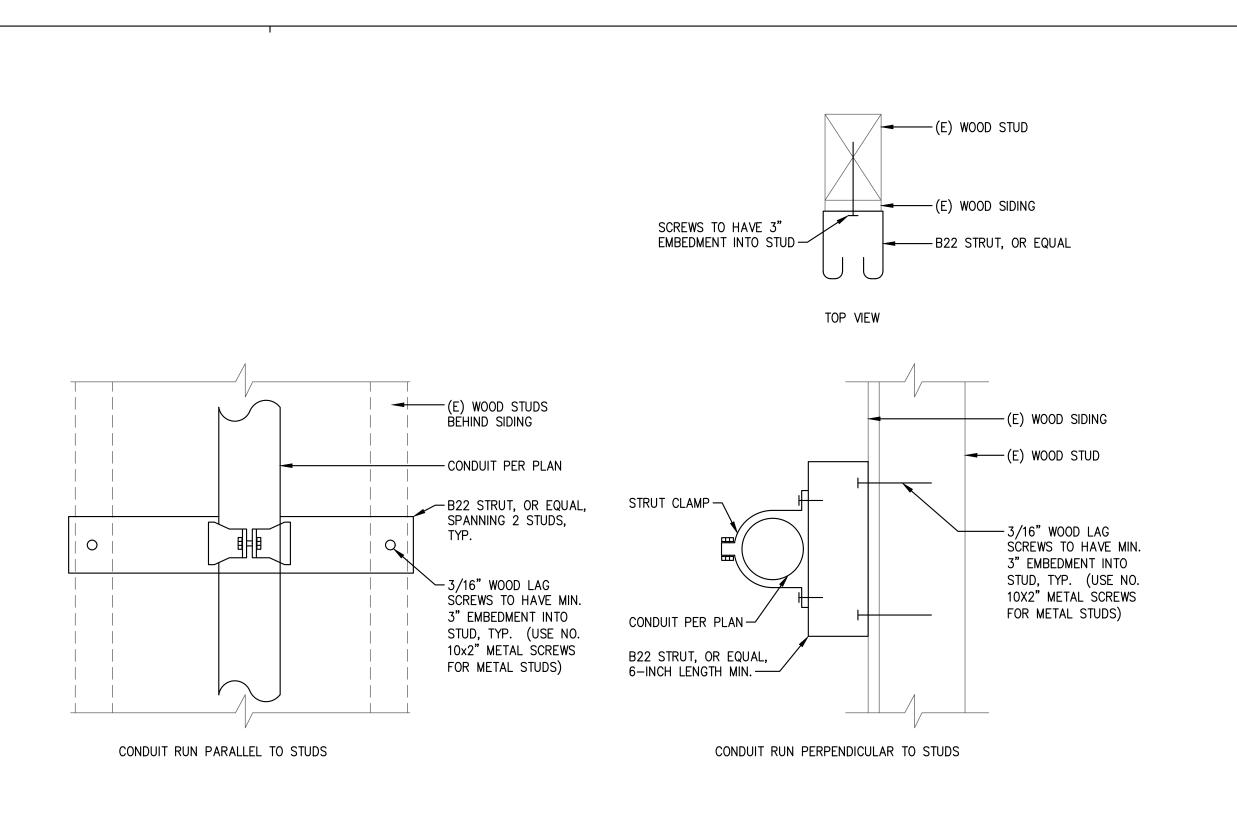
17	TYPE		: MLO		SER'	VICE:		MOUNT	ΓING:	ENCLOSURE:	10K	AIC
		: N12-1224RT BUSS FEEDER RATING		AMP AMP		/208 Ø, 3	VOLT	Γ	SURFACE	WIDTH: 15" DEPTH: 4.5"	100%	NEUT.
AØ	ВØ	DIRECTORY		BRKR	CKT		CKT	BRKR		DIRECTORY	AØ	BØ
•		DO NOT REMOVE THIS I	(.O.		1	•	2		DO NOT RE	MOVE THIS K.O.		•
		II .			3	•	4		"			
		N.L. [LIGHTS / PLUGS]		20/1	5	•	6	60/2	AC UNIT [N.	L.]		
		N.L. [LIGHTS / PLUGS]		20/1	7	•	8	-	"			
		N.L. [LIGHTS / PLUGS]		20/1	9	•	10	20/1	COMP 6			
		N.L. [LIGHTS / PLUGS]		20/1	11	•	12	20/1	COMP 8			
		N.L. [LIGHTS / PLUGS]		20/1	13	•	14	PFB	SPACE			
	360	RECEPTS - SHADE STR	20/1	15	•	16	PFB	SPACE				
		NEW LOAD		DEMAN	ID REA	DINGS		PEAK	DEMAND @	125% + (N) LOAD	TOTAL	DEMAND
		TOTAL PANEL VA	AMPS	AMPS	@1	25%			IPS	VA		DAD
Α	Ø =	0 VA	0.0	36.3		45.4		45.4		5445 VA	11325	5 VA
В	Ø =	360 VA	3.0	36.8		46.0		49.0	Α	5880 VA	49.0	AMPS

					Calcu						
Job Name:	Alice Birne	y Element	ary School	- Shade Stri	ucture					Job #:	22.01
Date:	2/22/2022										
	VOLTAGE:	120	PHASE:	1		POW/FR	FACTOR:	80%	CONDUIT:	Ş.	teel
	VOLIAGE.	120				TOVVEIX	ı Acıon.	0070	CONDOIT.		
	1										
FEEDER	AMPS AT	KVA	VOLTS	DISTANCE	DISTANCE	WIRES/	LOAD/	WIRE	WIRE	VOLTS	PERCENT
FEEDER NUMBER	AMPS AT LOAD	KVA TOTAL	VOLTS AT LOAD	DISTANCE FEET	DISTANCE TOTAL	WIRES/ PHASE	LOAD/ WIRE	WIRE SIZE	WIRE FACTOR	VOLTS DROP	
			AT LOAD	FEET	TOTAL	PHASE	· 1		''''-		PERCENT VOLT DRO



ONE LINE DIAGRAM

SCALE: NONE

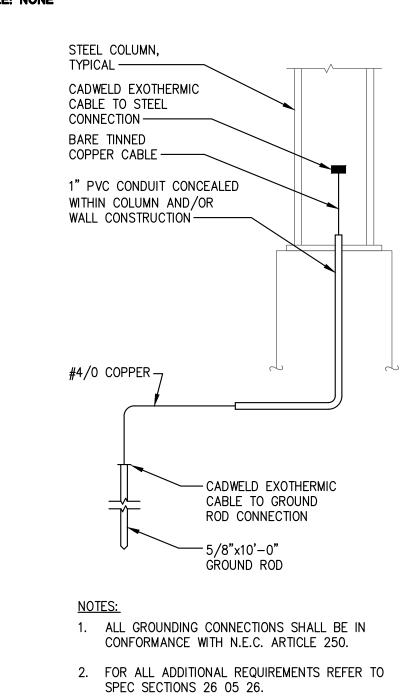


1. CONDUIT SHALL BE SUPPORTED AT INTERVALS NOT EXCEEDING TEN(10) FEET AND NOT MORE THAN THREE(3) FEET FROM THE OUTLET AND AT ANY POINT WHERE IT CHANGES DIRECTION. PERFORATED STRAP AND PLUMBER'S TAPE SHALL NOT BE PERMITTED. MAXIMUM CONDUIT AND CONDUCTOR WEIGHT IS 1.83LBS PER LINEAR FOOT.

7 CONDUIT MOUNTING DETAIL - STUD WALLS
SCALE: NONE

SHADE STRUCTURE WEATHERPROOF OUTLET BOX PER PLAN. SEE GENERAL NOTE #22 ON SHEET <u>**EO.1**</u> FOR" WEATHERPROOF GFCI RECEPTACLE RUN CONDUIT INTO REQUIREMENTS. -BACK BOX. SEE SHEET <u>E1.1</u> FOR MOUNTING HEIGHT. MOUNT TO COLUMN WITH (2) #10 SMS — PVC COATED RIGID STEEL CONDUIT STUB — _U.G. JUNCTION BOX U.H. PER PLAN (CHRISTY "N9") FINISH GRADE BASE(BELOW GRADE) TO NEXT RECEPTACLE -BRANCH CIRCUIT CONDUIT STANDARD RADIUS PVC COATED FROM PANEL BOARD. PVC COATED RIGID RIGID STEEL STEEL ELBOW, TYP. CONDUIT, TYP.

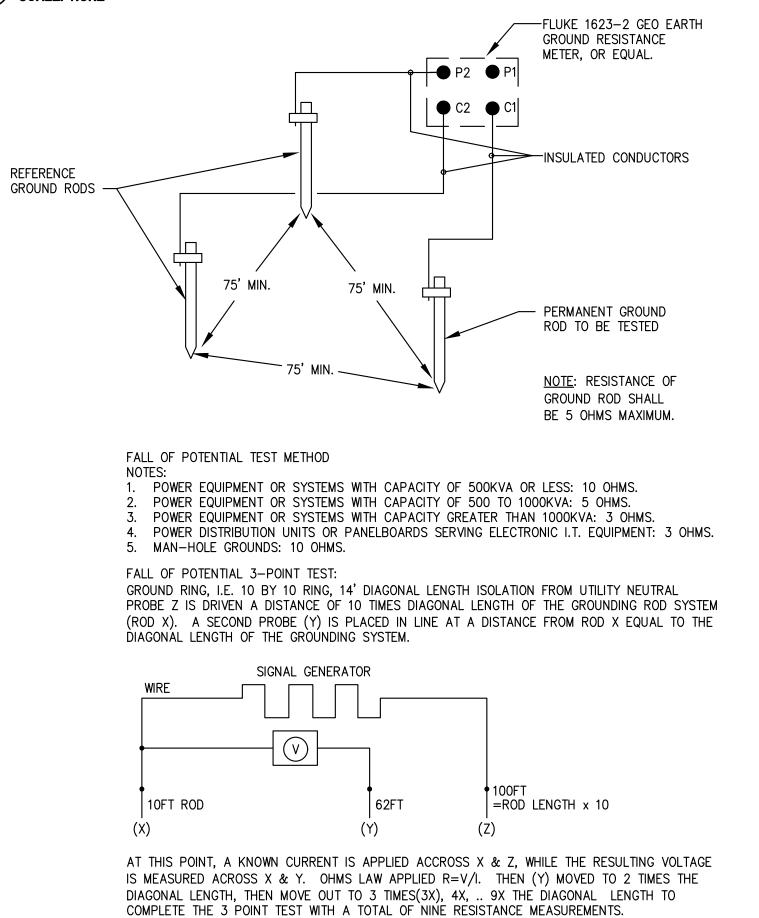
4 CONDUIT STUB IN POST DETAIL SCALE: NONE



TYPICAL STEEL COLUMN

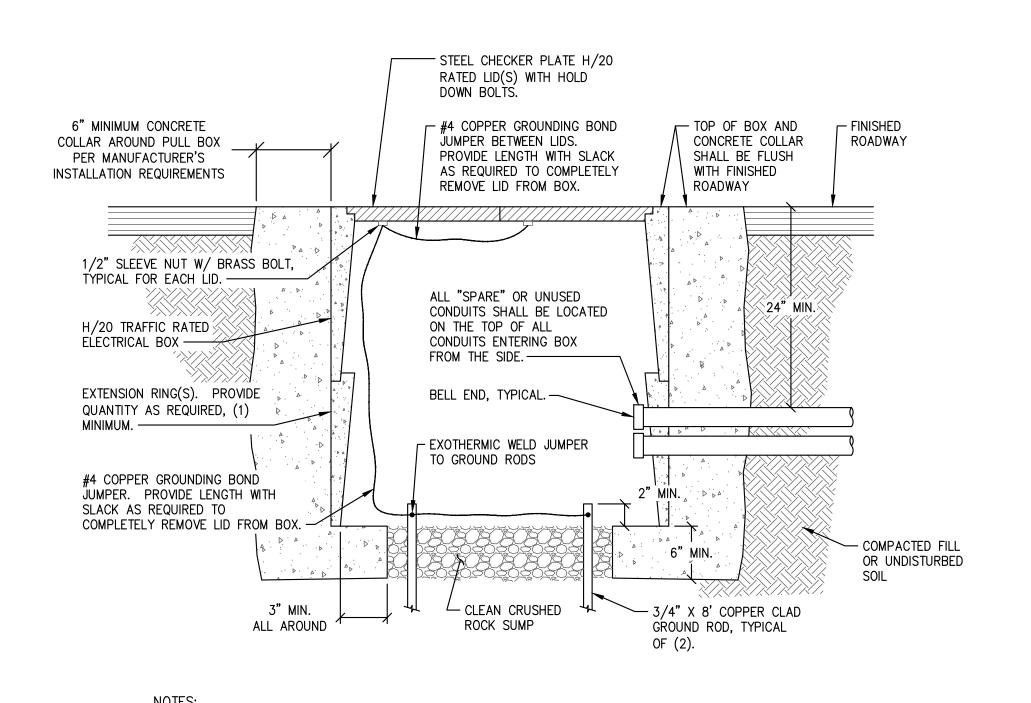
& REBAR GROUNDING DETAIL

SCALE: NONE



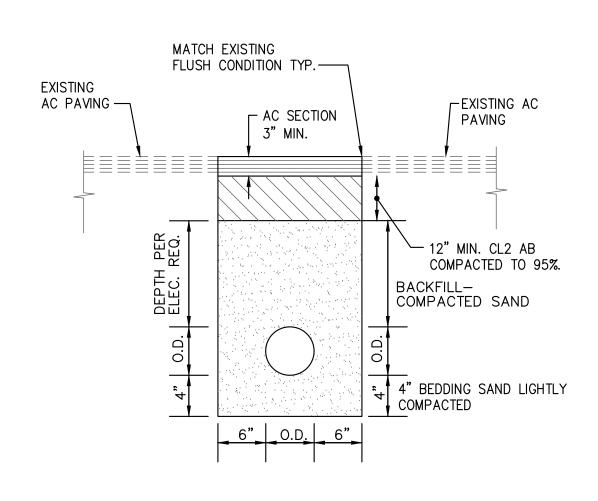
6 METHOD OF TESTING GROUND RODS DETAIL SCALE: NONE

DETAIL REMOVED SCALE: NONE



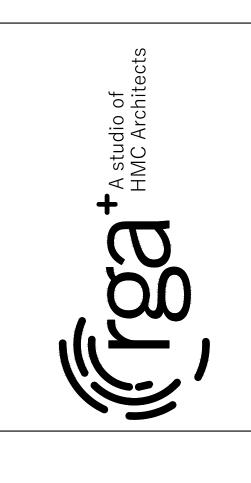
PROVIDE H/20 TRAFFIC RATED BOXES IN ALL LOCATIONS WITH VEHICLE TRAFFIC 2. CONTRACTOR SHALL PROVIDE THE MANUFACTURER'S INSTALLATION INSTRUCTIONS FOR H/20 TRAFFIC RATING REQUIREMENTS AS PART OF THE SUBMITTALS.

2 TYPICAL H/20 TRAFFIC RATED PULL BOX SCALE: NONE



3 TYPICAL TRENCH DETAIL
SCALE: NONE

IDENTIFICATION STAMP DIV. OF THE STATE ARCHITEC APP: 02-119978 INC: REVIEWED FOR SS 🗹 FLS 🗹 ACS 🗹 DATE: 03/30/2022







BIRNI

STRUCTURE A WALDORF K-SHADE : Revision

Copyright © 2021 Rainforth Grau Architects. All rights reserved. Reproduction of these plans, either in whole or in part, including any form of copying and/or preparation of derivative works thereof, for any reason without prior written permission, is strictly prohibited.

DETAILS

PROJECT NO.		21-1504.01
DATE:		3/1/22
SHEET	E0 4	
	E3. 1	

SOCF DEAD LOAD (SUPERIMPOSED ON FRAME) 5 PSF	
ROOF LIVE LOAD 20 P ROOF DEAD LOAD (SUPERIMPOSED ON FRAME) 5 PSF ROOF PANEL DEAD LOAD M=1.1 PSF, G=1.2 COLLATERAL DEAD LOAD M=3.9 PSF, G=3.1 ROOF SNOW LOAD. 20 P SROUND SNOW LOAD. Pg 20 P ROOF SNOW LOAD: SLOPED, Pg 20 P SITE APPLICATION DSA REVIEWER SHALL VERIFY THE STRUCTURE BE LOCATED AT LEAST 20 FEET FROM A SNOW LOAD LOPE FACTOR, Cg 11.6 SNOW EXPOSURE FACTOR, Cg 11.6 SNOW LOAD INPORTANCE FACTOR, Ig 1.1 HERRINA FACTOR, Cg 1.1 SASIC WIND SPEED (3 SECOND GUST), V _{cl} 10.0 RISK CATEGORY 0 SASIC WIND SPEED (3 SECOND GUST), V _{cl} 10.0 RISK CATEGORY 0 EXPOSURE CATEGORY 0 CACTORS, Kg, Kg, Sc 0.855, 1 Rg DOWER FASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED CASE A (1.1 / -1.2) G Cow, PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED CASE A (0.6 / -0.9) COMPONENTS & CLADDING - Cg (1 PRESSURE/SUCTION) CLEAR / OBSTRUCTED CASE A (0.6 / -0.9) COMPER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED CASE A (0.6 /	VALUES
SOSP EAD LOAD (SUPERIMPOSED ON FRAME) 5 PSF	DOE
ROOF PANEL DEAD LOAD	_
COLLATERAL DEAD LOAD ROOF SNOW LOAD	
ROOF SNOW LOAD ROOF SNOW LOAD ROOF SNOW LOAD ROOF SNOW LOAD ROOF SNOW LOAD SLOPED, P. 20 P RISK CATEGORY ■ ■ ROOF SNOW LOAD SLOPED, P. 20 P STITE APPLICATION DSA REVIEWER SHALL VERIFY THE STRUCTURE BE LOCATED AT LEAST 20 FEET FROM A SNOW LOAD SLOPE FACTOR, C. 1.0 1	
I ROBERT ROBER	,
ROOF SNOW LOAD: SLOPED, P ₈ STE APPLICATION DSA REVIEWER SHALL VERIFY THE STRUCTURE BE LOCATED AT LEAST 20 FEET FROM A SNOW LOAD SLOPE FACTOR, C ₉ 1.6 SNOW LOAD SLOPE FACTOR, C ₉ 1.7 SNOW LOAD SLOPE FACTOR, C ₉ 1.8 SNOW LOAD IMPORTANCE FACTOR, I ₈ 1.9 SASIC WIND SPEED (3 SECOND GUST), V _{ult} 100 M SASIC WIND SPEED (3 SECOND GUST), V _{ult} 100 M SISK CATEGORY 1.8 SASIC WIND SPEED (3 SECOND GUST), V _{ult} 100 M SISK CATEGORY 1.8 CATORS: K ₂ , K ₂ , K ₃ 1.9 1.9 SASIC WIND SPEED (3 SECOND GUST), V _{ult} 1.0 SASIC	PSF
SITE APPLICATION DSA REVIEWER SHALL VERIFY THE STRUCTURE BE LOCATED AT LEAST 20 FEET FROM A SNOW LOAD SLOPE FACTOR, C ₀ 1.0. SNOW EXPOSURE FACTOR, C ₀ 1.0. SNOW EXPOSURE FACTOR, C ₀ 1.1. SNOW EXPOSURE FACTOR, C ₀ 1.1. SNOW EXPOSURE FACTOR, C ₀ 1.2. SNOW EXPOSURE FACTOR, C ₀ 1.3. SNOW EXPOSURE FACTOR, C ₀ 1.4. SNOW EXPOSURE FACTOR, C ₀ 1.5. SASIC WIND SPEED (3 SECOND GUST), V _{olt} 1.6. SASIC WIND SPEED (3 SECOND GUST), V _{olt} 1.7. SASIC WIND SPEED (3 SECOND GUST), V _{olt} 1.8. SASIC WIND SPEED (3 SECOND GUST), V _{olt} 1.9. SASIC WIND SPEED (3 SECOND GUST), V _{olt} 1.0.	1
SNOW EXPOSURE FACTOR, C _e 1.1. SNOW LOAD IMPORTANCE FACTOR, I _e 1.2. THERMAL FACTOR, C ₁ 3. WIND DESIGN BASIC WIND SPEED (3 SECOND GUST), V _{ult} 1. BASIC WIND SPEED (3 SECOND GUST), V _{ult} 1. BASIC WIND SPEED (3 SECOND GUST), V _{ult} 1. 1. 1. 1. 1. 1. 1. 1. 1. 1	PSF
SNOW EXPOSURE FACTOR, C ₀ 1.1. SNOW LOAD IMPORTANCE FACTOR, I _k 1.1. THERMAL FACTOR, C ₁ BASIC WIND SPEED (3 SECOND GUST), V _{ult} 100 M RISK CATEGORY CACTORS: K ₀ , K _{th} , K _d Question (3 M Question (4 M Question	ADJACENT STRUCTURE
SENOW LOAD IMPORTANCE FACTOR, I ₁ THERMAL FACTOR, C ₁ MIND DESIGN BASIC WIND SPEED (3 SECOND GUST), V _{ult} 100 M RISK CATEGORY RISK CATEGORY CEXPOSURE CATEGORY COACTION A ₁ = 0.00256 K, K ₁ , K ₂ K ₃ B ₄ = 0.00256 K, K ₂ , K ₃ B ₄ = 0.00256 K, K ₂ , K ₃ B ₄ = 0.00256 K, K ₂ , K ₃ B ₄ = 0.00256 K, K ₂ , K ₃ B ₄ = 0.00256 K, K ₂ , K ₃ B ₄ = 0.00256 K, K ₂ , K ₃ B ₄ = 0.00256 K, K ₂ , K ₃ B ₄ = 0.00256 K, K ₂ , K ₃ B ₄ = 0.00256 K, K ₂ , K ₃ B ₄ = 0.00256 K, K ₂ , K ₃ B ₄ = 0.00256 K, K ₂ , K ₃ B ₄ = 0.00256 K, K ₂ , K ₃ B ₄ = 0.00256 K, K ₂ , K ₃ B ₄ = 0.00256 K, K ₃ B ₄ = 0.00256 K, K ₄ , K ₃ B ₄ = 0.00256 K, K ₂ , K ₃ B ₄ = 0.00256 K, K ₃ B ₄ = 0.00256 K, K ₄ , K ₃ B ₄ = 0.00256 K, K ₄ B ₅ = 0.00256 K, K ₄ B ₄ = 0.00256 K, K ₄ B ₅ = 0.00256 K, K ₄ B ₄ =	.0
THERMAL FACTOR, C, WIND DESIGN BASIC WIND SPEED (3 SECOND GUST), V _{ult} . 100 M RISK CATEGORY II EXPOSURE CATEGORY II EXPOSURE CATEGORY C FACTORS, K _x , K _x , K _x 0.85, 1 g _x = 0.00256 K, K _x , K _x 0.85, 1 g _x = 0.00256 K, K _x , K _x 0.85, 1 g _x = 0.00256 K, K _x 0.80 F ANGLE 18.43 - CLEAR / OBSTRUCTED CASE A (1.1 /-1.2) C _{ML} PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED CASE A (0.17 /-1.09) C _{ML} PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED CASE A (-0.17 /-1.09) C _{ML} PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED CASE A (-0.17 /-1.09) COMPONENTS & CLADDING - C _N (PRESSURE/SUCTION) CLEAR / OBSTRUCTED CONE 3 - (0.29 /- ZONE 2 - (1.77 /- ZONE 1 - (1.15 /- SEISMIC DESIGN STEEL - ORDINARY CA ANALYSIS PROCEDURE STEEL - ORDINARY CA SEISMIC IMPORTANCE FACTOR, Ie EQUIVALENT LA SEISMIC SITE CLASS D MCE _R SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S ₃ 0.9 SHORT PERIOD SITE COEFFICIENT, F _a 1.2 LONG PERIOD COEFFICIENT, F _a 1.7 FUNDAMENTAL PERIOD OF THE STRUCTURE, T 0.15 DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S _{DS} - USED 2.08 * 0.70 TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3) 1.0 DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S _{D1} 1.0 SEISMIC DESIGN CATEGORY 1.2 RESPONSE MODIFICATION FACTOR, R 1.2 OVERSTRENGTH FACTOR, Ω 1.2 RESDUNDANCY FACTOR, Ω 1.2 RESD	.0
THERMAL FACTOR, C, WIND DESIGN BASIC WIND SPEED (3 SECOND GUST), V _{ult} . 100 M RISK CATEGORY II EXPOSURE CATEGORY II EXPOSURE CATEGORY C FACTORS, K _x , K _x , K _x 0.85, 1 g _x = 0.00256 K, K _x , K _x 0.85, 1 g _x = 0.00256 K, K _x , K _x 0.85, 1 g _x = 0.00256 K, K _x 0.80 F ANGLE 18.43 - CLEAR / OBSTRUCTED CASE A (1.1 /-1.2) C _{ML} PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED CASE A (0.17 /-1.09) C _{ML} PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED CASE A (-0.17 /-1.09) C _{ML} PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED CASE A (-0.17 /-1.09) COMPONENTS & CLADDING - C _N (PRESSURE/SUCTION) CLEAR / OBSTRUCTED CONE 3 - (0.29 /- ZONE 2 - (1.77 /- ZONE 1 - (1.15 /- SEISMIC DESIGN STEEL - ORDINARY CA ANALYSIS PROCEDURE STEEL - ORDINARY CA SEISMIC IMPORTANCE FACTOR, Ie EQUIVALENT LA SEISMIC SITE CLASS D MCE _R SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S ₃ 0.9 SHORT PERIOD SITE COEFFICIENT, F _a 1.2 LONG PERIOD COEFFICIENT, F _a 1.7 FUNDAMENTAL PERIOD OF THE STRUCTURE, T 0.15 DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S _{DS} - USED 2.08 * 0.70 TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3) 1.0 DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S _{D1} 1.0 SEISMIC DESIGN CATEGORY 1.2 RESPONSE MODIFICATION FACTOR, R 1.2 OVERSTRENGTH FACTOR, Ω 1.2 RESDUNDANCY FACTOR, Ω 1.2 RESD	.0
WIND DESIGN BASIC WIND SPEED (3 SECOND GUST), V _{ol} R 100 M RISK CATEGORY 0.85, 1 EXPOSURE CATEGORY 0.85, 1 10 _a = 0.00256 K _x K _{xt} V _x FOR ALL EAVE HEIGHTS (8', 10' & 12') 18.50 0.2 _{MM} PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED CASE A (1.1 / -1.2) 0.8 _{ML} PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED CASE A (-0.17 / -1.09) 0.2 _{ML} PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED CASE A (-0.6 / -0.9) 0.00MPONENTS & CLADDING - C _N (PRESSURE/SUCTION) CLEAR / OBSTRUCTED CASE A (-0.6 / -0.9) 0.00MPONENTS & CLADDING - C _N (PRESSURE/SUCTION) CLEAR / OBSTRUCTED ZONE 3 - (2.29 / -2.20 / -2.2	2
BASIC WIND SPEED (3 SECOND GUST), V _{ult} RISK CATEGORY RISK CATEGORY CATEGORY CASEA (-0.85, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1,	
RISK CATEGORY EXPOSURE CATEGORY C EXPOSURE CATEGORY C DATE OF THE EXPOSURE CATEGORY RESPONSE MODIFICATION FACTOR, ρ RESPONSE MODIFICATION FOR TICK. 20' WIDE, 30' WIDE, 40' WIDE) 1.10 DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE) 1.11 DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE) 1.11 DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE) 1.12 DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE) 1.14 DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE) 1.15 DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE) 1.16 DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE) 1.17 PENDAMENTAL OF CATEGORY C DASS A (-0.0 f, 0.15) C ASE A (-0.1 f, -1.2)	MPH
EXPOSURE CATEGORY FACTORS: K ₂ , K ₂ , K ₃ 0.85, 1 The = 0.00256 K ₂ K ₂ K ₄ V ₂ Por All EAVE HEIGHTS (8', 10' & 12') 1.9, = 0.00256 K ₂ K ₂ K ₄ V ₂ Por All EAVE HEIGHTS (8', 10' & 12') 1.9, = 0.00256 K ₃ K ₄ K ₄ V ₂ Por All EAVE HEIGHTS (8', 10' & 12') 1.9, PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED 1.0, PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED 1.0, PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED 1.0, PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED 1.0, PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED 1.0, PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED 1.0, PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED 1.0, PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED 1.0, PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED 1.0, PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED 1.0, PER SUBJECT OBSTRUCT OBSTRUCTED 1.0, PER SUBJECT OBSTRUCT OB	
FACTORS: K ₂ , K ₂ , K ₃ Q ₁ = 0.00256 K ₂ K ₂ , K ₃ V ² FOR ALL EAVE HEIGHTS (8', 10' & 12') 18.50 C _{MW} PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED CASE A (1.1 / -1.2) C _{ML} PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED CASE A (-0.17 / -1.09) C _M PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C _N (PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C _N (PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C _N (PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C _N (PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C _N (PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C _N (PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C _N (PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C _N (PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C _N (PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C _N (PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C _N (PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLADDING - C _N (PRESSURE/SUCTION) CLEAR / OBSTRUCTED COMPONENTS & CLEAR - (OBSTRUCTED COMPONENTS & CLAD - (OBSTRUCTED COMPONENTS & CLACE COMPONENTS & COMPONENTS	
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	1, 0.85
C _{MM} PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED CASE A (-0.17 /-1.09) C _{NL} PER ASCE FIGURE 27.4-5 ROOF ANGLE 18.43 - CLEAR / OBSTRUCTED CASE A (-0.17 /-1.09) C _N PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED CASE A (-0.6 /-0.9) COMPONENTS & CLADDING - C _N (PRESSURE/SUCTION) CLEAR / OBSTRUCTED ZONE 3 - (2.29 /-2 ZONE 2 - (1.77 /-1 ZONE 1 - (1.15 /-1 SEISMIC DESIGN ZONE 1 - (1.15 /-1 LATERAL FORCE RESISTING SYSTEM STEEL - ORDINARY C/A ANALYSIS PROCEDURE EQUIVALENT LA SEISMIC IMORTANCE FACTOR, Ie 11.0 SEISMIC SITE CLASS D MCE _R SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S _s 2.6 MCE _R SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S _s 2.6 MCE _R SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S _s 1.2 LONG PERIOD COEFFICIENT, F _s 1.2 LONG PERIOD COEFFICIENT, F _s 1.2 DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S _{DS} - USED 2.08 * 0.70 DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S _{DS} - USED 2.08 * 0.70 DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S _{D1} 1.0 DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-	
$C_{NL} \ PER \ ASCE FIGURE 27.4-5 \ ROOF \ ANGLE 18.43 - CLEAR / OBSTRUCTED $	
$ \begin{array}{c} C_N \text{PER ASCE FIGURE 27.4-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED} & CASE A (-0.6 / -0.9) \\ \hline COMPONENTS & CLADDING - C_N (\text{PRESSURE/SUCTION}) \text{CLEAR / OBSTRUCTED}} & ZONE 3 - (2.29 / \cdot 20 \text{Ne} 2 - (1.77 / - 20 / - 20 \text{NE} 2 - (1.28 / - 20 , - 20 \text{NE} 2 $	· · · · · · · · · · · · · · · · · · ·
COMPONENTS & CLADDING - C_N (PRESSURE/SUCTION) CLEAR / OBSTRUCTED ZONE 3 - (2.29) / (2.29) / (2.20)	
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	· · · · · · · · · · · · · · · · · · ·
$ \begin{array}{c} \text{ZONE 1 - } 1.15 I - \\ \text{SEISMIC DESIGN} \\ \\ \text{LATERAL FORCE RESISTING SYSTEM} \\ \text{STEEL - ORDINARY CAMALYSIS PROCEDURE} \\ \text{SESIMIC IMORTANCE FACTOR, Ie} \\ \text{SEISMIC SITE CLASS} \\ \text{MCE}_R \text{ SPECTRAL RESPONSE ACCELERATION @ 0.2 \text{ s, S}_S } \\ \text{2.6} \\ \text{MCE}_R \text{ SPECTRAL RESPONSE ACCELERATION @ 0.2 \text{ s, S}_1 } \\ \text{3.1} \\ \text{3.2} \\ \text{4.1} \\ \text{5.2} \\ \text{5.2} \\ \text{5.3} \\ \text{5.4} \\ \text{5.2} \\ \text{6.4} \\ \text{6.2} \\ \text{5.2} \\ \text{6.4} \\ \text{6.2} \\ 6.2$	
LATERAL FORCE RESISTING SYSTEM ANALYSIS PROCEDURE EQUIVALENT LA SESIMIC IMORTANCE FACTOR, Ie SEISMIC SITE CLASS D MCE_R SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S _S $C.6$ MCE_R SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S ₁ $C.6$ SHORT PERIOD SITE COEFFICIENT, F _a $C.6$ LONG PERIOD COEFFICIENT, F _v $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S _{DS} $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S _{DS} $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S _{DS} $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S _{DS} $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S _{DS} $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S _{DS} $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S _{DS} $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S _{D1} $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S _{D1} $C.6$ DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S _{D1} $C.6$ DESIGN CATEGORY EQUIVALENT LA DESIGN CATEGORY EQUIVALENT LA DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S _{D1} $C.6$	-1.05) / (0.5 / -1.5)
ANALYSIS PROCEDURE SESIMIC IMORTANCE FACTOR, le 1.6 SEISMIC SITE CLASS MCE_R SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S _S 2.6 MCE_R SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S _S 3.7 SHORT PERIOD SITE COEFFICIENT, F _a 1.7 FUNDAMENTAL PERIOD OF THE STRUCTURE, T DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S _{DS} 2.0 DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S _{DS} - USED TO DETERMINE CS (WITH CAP PER ASCE-7 12.8.1.3) DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S _{D1} 1.0 SEISMIC DESIGN CATEGORY E RESPONSE MODIFICATION FACTOR, R 1.2 OVERSTRENGTH FACTOR, Ω 1.2 REDUNDANCY FACTOR, ρ 1.4 HORIZONTAL OR VERTICAL IRREGULARITIES SEISMIC RESPONSE COEFFICIENT, CS (20' WIDE, 30' WIDE, 40' WIDE) DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE) 1.2.73 PSF, 13.41	ANTILEVER COLUMN
SEISMIC SITE CLASS MCE_R SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S _S MCE_R SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S _S MCE_R SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S ₁ MCE_R SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S ₁ MCE_R SPECTRAL RESPONSE ACCELERATION @ 0.2 s, S ₁ MCE_R SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S _{DS} MCE_R SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S _{DS} MCE_R SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S _{DS} - USED MCE_R SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S _{DS} - USED MCE_R SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S _{D1} MCE_R SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S _{D1} MCE_R SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S _{D1} MCE_R SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S _{D1} MCE_R SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S _{D1} MCE_R SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S _{D1} MCE_R SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S _{D1} MCE_R SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S _{D1} MCE_R SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S _{D1} MCE_R SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S _{D1} MCE_R SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S _{D1} MCE_R SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S _{D1} MCE_R SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S _{D1} MCE_R SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S _{D2} MCE_R SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S _{D2} MCE_R SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S _{D2} MCE_R SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S _{D2} MCE_R SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S _{D2} MCE_R SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S _{D2} MCE_R SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S _{D2} MCE_R SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S _{D2} MCE_R SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S _{D2} MCE_R SPECTRAL RESPONSE ACCELERATION AT SHORT PERIODS, S _{D2} MCE_R SPECTRAL RESPONSE ACCELERATION AT S	
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$.0
MCE _R SPECTRAL RESPONSE ACCELERATION @ 0.2 s , S_1 SHORT PERIOD SITE COEFFICIENT, F_a LONG PERIOD COEFFICIENT, F_v 1.7 FUNDAMENTAL PERIOD OF THE STRUCTURE, T DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S_{DS} 2.0 DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S_{DS} - USED TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3) DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S_{D1} 1.0 SEISMIC DESIGN CATEGORY ERSPONSE MODIFICATION FACTOR, R OVERSTRENGTH FACTOR, D 1.2 REDUNDANCY FACTOR, D 1.4 HORIZONTAL OR VERTICAL IRREGULARITIES SEISMIC RESPONSE COEFFICIENT, Cs (D_{CO} WIDE, D_{CO} W)
SHORT PERIOD SITE COEFFICIENT, F_a LONG PERIOD COEFFICIENT, F_v 1.7 FUNDAMENTAL PERIOD OF THE STRUCTURE, T DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S_{DS} 2.0 DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S_{DS} - USED TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3) DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S_{D1} 1.0 SEISMIC DESIGN CATEGORY ERSPONSE MODIFICATION FACTOR, R OVERSTRENGTH FACTOR, Ω 1.2 REDUNDANCY FACTOR, ρ HORIZONTAL OR VERTICAL IRREGULARITIES NON SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE) DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE) 12.73 PSF, 13.41	60
LONG PERIOD COEFFICIENT, F_V FUNDAMENTAL PERIOD OF THE STRUCTURE, T 0.15. DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S_{DS} 2.0 DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S_{DS} - USED TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3) DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S_{D1} 1.0 SEISMIC DESIGN CATEGORY ERSPONSE MODIFICATION FACTOR, R 1.2 OVERSTRENGTH FACTOR, Ω 1.2 REDUNDANCY FACTOR, ρ 1.4 HORIZONTAL OR VERTICAL IRREGULARITIES NON SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE) DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE) 12.73 PSF, 13.41	90
FUNDAMENTAL PERIOD OF THE STRUCTURE, T 0.15. DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S_{DS} 2.0 DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S_{DS} - USED 2.08 * 0.70 TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3) DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S_{D1} 1.0 SEISMIC DESIGN CATEGORY E RESPONSE MODIFICATION FACTOR, R 1.2 OVERSTRENGTH FACTOR, Ω 1.2 REDUNDANCY FACTOR, ρ 1.0 HORIZONTAL OR VERTICAL IRREGULARITIES NON SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE) 1.16 DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE) 12.73 PSF, 13.41	20
FUNDAMENTAL PERIOD OF THE STRUCTURE, T 0.15. DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S_{DS} 2.0 DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S_{DS} - USED 2.08 * 0.70 TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3) DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S_{D1} 1.0 SEISMIC DESIGN CATEGORY E RESPONSE MODIFICATION FACTOR, R 1.2 OVERSTRENGTH FACTOR, Ω 1.2 REDUNDANCY FACTOR, ρ 1.0 HORIZONTAL OR VERTICAL IRREGULARITIES NON SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE) 1.16 DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE) 12.73 PSF, 13.41	70
DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S_{DS} 2.0 DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S_{DS} - USED 2.08 * 0.70 TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3) DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S_{D1} 1.0 SEISMIC DESIGN CATEGORY RESPONSE MODIFICATION FACTOR, R 1.2 OVERSTRENGTH FACTOR, Ω 1.2 REDUNDANCY FACTOR, ρ 1.0 HORIZONTAL OR VERTICAL IRREGULARITIES NON SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE) 1.16 DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE) 12.73 PSF, 13.41	52 s
DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, S_{DS} - USED TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3) DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S_{D1} SEISMIC DESIGN CATEGORY RESPONSE MODIFICATION FACTOR, R OVERSTRENGTH FACTOR, Ω REDUNDANCY FACTOR, ρ HORIZONTAL OR VERTICAL IRREGULARITIES SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE) DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE) 12.73 PSF, 13.41	
TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3) DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S _{D1} SEISMIC DESIGN CATEGORY RESPONSE MODIFICATION FACTOR, R OVERSTRENGTH FACTOR, Ω REDUNDANCY FACTOR, ρ HORIZONTAL OR VERTICAL IRREGULARITIES SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE) DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE) 12.73 PSF, 13.41	08
TO DETERMINE Cs (WITH CAP PER ASCE-7 12.8.1.3) DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S _{D1} SEISMIC DESIGN CATEGORY RESPONSE MODIFICATION FACTOR, R OVERSTRENGTH FACTOR, Ω REDUNDANCY FACTOR, ρ HORIZONTAL OR VERTICAL IRREGULARITIES SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE) DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE) 12.73 PSF, 13.41	
DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, S _{D1} SEISMIC DESIGN CATEGORY RESPONSE MODIFICATION FACTOR, R OVERSTRENGTH FACTOR, Ω REDUNDANCY FACTOR, ρ HORIZONTAL OR VERTICAL IRREGULARITIES SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE) DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE) 12.73 PSF, 13.41	7 0 = 1.456
SEISMIC DESIGN CATEGORY RESPONSE MODIFICATION FACTOR, R 1.2 OVERSTRENGTH FACTOR, Ω REDUNDANCY FACTOR, ρ HORIZONTAL OR VERTICAL IRREGULARITIES SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE) DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE) 12.73 PSF, 13.41	
SEISMIC DESIGN CATEGORY RESPONSE MODIFICATION FACTOR, R 1.2 OVERSTRENGTH FACTOR, Ω REDUNDANCY FACTOR, ρ HORIZONTAL OR VERTICAL IRREGULARITIES SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE) DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE) 12.73 PSF, 13.41	
RESPONSE MODIFICATION FACTOR, R OVERSTRENGTH FACTOR, Ω REDUNDANCY FACTOR, ρ HORIZONTAL OR VERTICAL IRREGULARITIES SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE) DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE) 12.73 PSF, 13.41	
OVERSTRENGTH FACTOR, Ω 1.2 REDUNDANCY FACTOR, ρ 1.0 HORIZONTAL OR VERTICAL IRREGULARITIES NON SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE) 1.10 DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE) 12.73 PSF, 13.41	
REDUNDANCY FACTOR, ρ HORIZONTAL OR VERTICAL IRREGULARITIES SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE) DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE) 12.73 PSF, 13.41	
HORIZONTAL OR VERTICAL IRREGULARITIES SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE) DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE) 12.73 PSF, 13.41	
SEISMIC RESPONSE COEFFICIENT, Cs (20' WIDE, 30' WIDE, 40' WIDE) DESIGN BASE SHEAR, V (20' WIDE, 30' WIDE, 40' WIDE) 1.10 1.2.73 PSF, 13.41	
ALLOWABLE SOIL BEARING FOR FOUNDATIONS VARIES - SEE FOUN	· ·
ALLOWABLE SOIL BEARING FOR FOUNDATIONS VARIES - SEE FOUN	
	NDATION CHARTS
FLOOD DESIGN - DESIGN IS ASSUMED TO NOT BE IN FLOOD HAZARD AREA	
IF PROJECT IS LOCATED IN A FLOOD ZONE OTHERTHAN ZONE X, A LETTER STAMPED & SIGNED FROM A SOILS ENGINEER IS REQUIRED TO VALIDATE THE	

ALL DEFLECTIONS SHOWN ALSO INCLUDE THE P-DE	LTA ROTATION PER IR PC-7	DEFLECT	IONS ARE FOR (1) STI	RUCTURE
		SOIL	CLASSES PER CBC TABLE 18	06A.2
MAXIMUM DRIFT δ_{max} SIDE COLUMNS		Soil Class 5	Soil Class 4	Soil Class 3
20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.40	2.55	2.65
30' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.25	2.35	2.45
40' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT) MINIMUM SEPARATION $(\delta_m = C_d \ \delta_{max})$ $C_d = 1.25$	(INCHES)	2.20	2.25	2.20
20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	3.00	3.19	3.31
30' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.81	2.94	3.06
40' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.75	2.81	2.75
MAXIMUM DRIFT δ_{max} CORNER COLUMNS		Soil Class 5	Soil Class 4	Soil Class 3
20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.20	2.30	2.40
30' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.30	2.45	2.50
40' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT) MINIMUM SEPARATION $(\delta_m = C_d \ \delta_{max})$ $C_d = 1.25$	(INCHES)	2.40	2.55	2.65
20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.75	2.88	3.00
30' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.88	3.06	3.13
40' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	3.00	3.19	3.31
MAXIMUM DRIFT δ_{max} END COLUMNS		Soil Class 5	Soil Class 4	Soil Class 3
20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	1.60	1.70	1.75
30' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.00	2.45	2.25
40' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT) MINIMUM SEPARATION $(\delta_m = C_d \ \delta_{max})$ $C_d = 1.25$	(INCHES)	2.50	2.30	2.80
20' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.00	2.13	2.19
30' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	2.50	3.06	2.81
40' WIDE (8' EAVE HT, 10' EAVE HEIGHT, 12' EAVE HT)	(INCHES)	3.13	2.88	3.50

STRUCTURAL SEPARATION

ARCHITEC TURAL REQUIREMENTS						
DESC RIPTION	DESIGN VAULES					
TYPE OF CONSTRUCTION	II-B					
OCCUPANCY CLASSIFICATION	A-3					
NUMBER OF STORIES	1					
FIRE SPRINKLER SYSTEM	NOT BY ICON/WEIGHT NOT INCLUDED IN DESIGN					

RELATED BUILDING CODES AND STANDARDS

TITLE 24 CODES:

2019 CALIFORNIA ADMINISTRATIVE CODE (CAC).. ..(PART 1, TITLE 24, CCR) 2019 CALIFORNIA BUILDING CODE (CBC), VOLUMES 1, AND 2.(PART 2, TITLE 24,

2019 CALIFORNIA ELECTRICAL CODE. .(PART 3, TITLE 24, CCR) 2019 CALIFORNIA MECHANICAL CODE (CMC). .(PART 4, TITLE 24, CCR) (PART 5, TITLE 24, CCR) 2019 CALIFORNIA PLUMBING CODE (CPC).. 2019 CALIFORNIA ENERGY CODE. (PART 6, TITLE 24, CCR) 2019 CALIFORNIA FIRE CODE (CFC) .(PART 9, TITLE 24, CCR` 2019 CALIFORNIA GREEN BUILDING STANDARDS CODE.....(PART 11, TITLE 24, CCR) 2019 CALIFORNIA REFERENCE STANDARDS CODE.. ..(PART 12, TITLE 24, CCR)

REFERENCE CODE SECTIONS FOR APPLICABLE STANDARDS: 2019 CBC, CHAPTER 35

2019 CFC, CHAPTER 80 SCOPE OF WORK NARRATIVE

THESE DRAWINGS ILLUSTRATE THE FABRICATION AND INSTALLATION REQUIREMENTS FOR A FREE-STANDING PREFABRIC ATED STEEL SHADE STRUCTURE. THE ENTIRE STRUCTURAL SYSTEM IS COMPRISED OF HOLLOW STRUCTURAL STEEL MEMBERS SUPPORTED BY CONCRETE FOUNDATIONS. THE FLEXIBILITY INCLUDED HEREIN

ALLOWS THE STRUCTURE TO COMPLY WITH A WIDE VARIETY OF PROJECT SITES AND LOADING REQUIREMENTS.

<u>GENERAL:</u>

- 1. GENERAL NOTES AND TYPICAL DETAILS SHALL APPLY TO ALL PARTS OF THE JOB EXCEPT WHERE THEY MAY CONFLICT WITH DETAILS AND NOTES ON OTHER SHEETS. WHERE CONDITIONS ARE NOT SPECIFICALLY INDICATED BUT ARE OF SIMILAR CHARACTER TO DETAILS SHOWN, SIMILAR DETAILS OF CONSTRUCTION SHALL BE USED SUBJECT TO REVIEW BY THE STRUCTURAL ENGINEER FOR THIS PROJECT
- 2. WORK SHALL CONFORM TO THE REQUIREMENTS, AS AMENDED TO DATE, OF THE LATEST ADOPTED EDITION OF THE CBC, C.A.C. TITLE 24, AND ALL OTHER LOCAL, STATE AND FEDERAL REGULATIONS.
- 3. OMISSIONS OR CONFLICTS BETWEEN THE VARIOUS ELEMENTS OF THE WORKING DRAWINGS AND/OR SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER FOR THIS PROJECT PRIOR TO PROCEEDING
- 4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING THE WORK OF ALL TRADES AND SHALL CHECK ALL DIMENSIONS, ALL DISCREPANCIES SHALL BE CALLED TO THE ATTENTION OF THE STRUCTURAL ENGINEER FOR THIS PROJECT AND BE RESOLVED BEFORE PROCEEDING WITH THE WORK.
- 5. THESE CONSTRUCTION DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE AND DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND SHALL BE SOLELY RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES, INCLUDING, BUT NOT LIMITED TO, BRACING, TEMPORARY SUPPORTS, AND SHORING. OBSERVATION VISIT TO THE SITE BY FIELD REPRESENTATIVES OF THE ARCHITECT/ENGINEER SHALL NOT INCLUDE INSPECTIONS OF THE PROTECTIVE MEASURES OR THE CONSTRUCTION PROCEDURES. ANY SUPPORT SERVICES PERFORMED BY THE ARCHITECT/ENGINEER DURING THE CONSTRUCTION SHALL BE DISTINGUISHED FROM CONSTRUCTION AND DETAILED INSPECTION SERVICES WHICH ARE FURNISHED BY OTHERS. THESE SUPPORT SERVICES PERFORMED BY THE ARCHITECT/ENGINEER, WHETHER OF MATERIAL OR WORK, ARE FOR THE PURPOSE OF ASSISTING IN QUALITY
- CONTROL AND IN ACHIEVING CONFORMANCE WITH CONTRACT DOCUMENTS, BUT DO NOT GUARANTEE CONSTRUCTION. 6. ASTM DESIGNATIONS AND ALL STANDARDS REFER TO THE LATEST AMENDMENTS.
- 7. CONFORM TO APPLICABLE CAL/OSHA CONSTRUCTION SAFETY REGULATIONS FOR ALL WORK PERFORMED DURING CONSTRUCTION. JOB SITE SAFETY IS STRICTLY THE RESPONSIBILITY OF THE CONTRACTOR AND NOT THE ARCHITECT/ENGINEER OR OWNER.
- 8. THE ENGINEER AND THEIR CONSULTANTS SHALL HAVE NO RESPONSIBILITY FOR THE DISCOVERY, HANDLING, REMOVAL OR DISPOSAL OF HAZARDOUS MATERIALS AT THE PROJECT SITE, INCLUDING BUT NOT LIMITED TO
- ASBESTOS, ASBESTOS PRODUCTS, POLYCHLORINATED BIPHENYL (PCB) OR OTHER TOXIC SUBSTANCES. 9. SHOULD ANY CONDITIONS DEVELOP NOT COVERED BY THE CONTRACT DOCUMENTS, OR IF A CHANGE IN THE SCOPE OF WORK IS PROPOSED, A CONSTRUCTION CHANGE DOCUMENT DETAILING AND SPECIFYING THE REQUIRED
- CHANGE(S) SHALL BE SUBMITTED TO AND APPROVED BY DSA BEFORE PROCEEDING WITH THE WORK. 10. THE SCHOOL DISTRICT INSPECTOR ON RECORD SHALL INSPECT AND APPROVE THE ERECTED FRAME PRIOR TO ROOF
- 11. SEE REQUIREMENTS FOR LOCATION IN ANY FIRE HAZARD SEVERITY ZONE FOR WILDLAND URBAN INTERFACE AREAS (WUI) AS SPECIFIED IN THE APPLICABLE VERSION OF THE CALIFORNIA BUILDING CODE. PROVIDE PROTECTION AND DETAILS OF ALL AREAS COMPLYING WITH THE WUI REQUIREMENTS.
- 12. LOCATING THIS STRUCTURE CLOSER THAN 20 FEET TO OTHER STRUCTURES MAY AFFECT THE ALLOWABLE AREA
- FOR THE EXISTING CONSTRUCTION PER THE APPLICABLE VERSION OF THE CALIFORNIA BUILDING CODE. 13. VIEWS AND DETAILS ARE NOT DRAWN TO SCALE (UNLESS NOTED OTHERWISE). DO NOT SCALE THESE DRAWINGS.

STRUCTURAL AND MISCELLANEOUS STEEL:

- 1. ALL STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE AMERICAN INSTITUE OF STEEL CONSTRUCTION (AISC) SPECIFICATION MANUAL REFERENCED BY THE LATEST EDITION OF THE CALIFORNIA BUILDING CODE.
- 2. PIPE SECTIONS SHALL CONFORM TO ASTM A53, Fy = 35 KSI, GRADE B OR A501 UNLESS NOTED OTHERWISE. 3. STRUCTURAL TUBING (HSS SHAPES) SHALL CONFORM TO ASTM A-500, GRADE B (OR C), Fy = 46 KSI (MIN).
- 4. IF MATERIAL AVAILABILITY IS LIMITED, MEMBER THICKNESS CAN BE INCREASED BEYOND WHAT IS SHOWN IN THESE DRAWINGS (MAXIMUM INCREASE OF 1/8").
- 5. ALL CHANNELS, ANGLES, AND MISC. STEEL SHALL CONFORM TO ASTM A-36, Fy =36 KSI.
- 6. ALL PLATE STEEL SHALL CONFORM TO ASTM A-572, Fy= 50 KSI.
- 7. ALL COLD FORM STEEL SHALL CONFORM TO ASTM A-653, CS = TYPE B, Fy = 50 KSI. 8. STRUCTURAL STEEL AND DECK SHALL BE IDENTIFIED FOR CONFORMITY PER CBC 2202A.1.
- 9. ALL ROOF DECKS SHALL HAVE KYNAR 500 METAL COATING.
- 10.ALL ROOF DECKS SHALL CONFORM TO ASTM A-792, Fy = 50 KSI.

INSTRUCTIONS FOR ARCHITECTS SUBMITTING THESE PRE-CHECKED DRAWING TO DSA: BEFORE SUBMITTING THESE PRE-CHECKED DRAWINGS FOR YOUR PROJECT, FOLLOW THE

STEP 1: SELECT FRAME DIMENSIONS FOR YOUR PROJECT -GABLE STRUCTURES UP TO 20' WIDE USE THE "RG 20" BASE FRAME -GABLE STRUCTURES UP TO 30' WIDE USE THE "RG 30" BASE FRAME

STEPS BELOW TO PROPERLY DEFINE THE APPROVED OPTIONS:

-GABLE STRUCTURES UP TO 40' WIDE USE THE "RG 40" BASE FRAME -MAXIMUM WIDTH IS 40' (SEE "ARCHITECTURAL VIEWS" SHEET FOR REFERENCE) -THE 24', 44', 64', 84' AND 104' LENGTHS ARE SUGGESTED BECAUSE THEY ARE THE MOST COMMON

(20' BAYS ARE THE MOST ECONOMICAL) -FRAME LENGTHS ASSUME 2' OVERHANGS (UNO BY ARCHITECT - 2' MAX DIMENSION)

STEP 2: SELECT ROOF DECK FOR YOUR PROJECT -"M" REPRESENTS McELROY METAL "MULTI-RIB" ROOF PANEL

-"G" REPRESENTS McELROY METAL "MEGA-RIB" ROOF PANEL -"S" REPRESENTS MCELROY METAL "MEDALLION-LOK" 16" STANDING SEAM ROOF PANEL

STEP 3: IDENTIFY THE Ss ACCELERATION (q) FOR YOUR PROJECT

-Ss VALUE DETERMINES THE REQUIRED SEISMIC DESIGN FORCES -Ss VALUE DEPENDS ON THE PROJECTS GEOGRAPHICAL LOCATION (VALUES RANGE FROM 0.00 TO 3.73)

STEP 4: IDENTIFY THE Ss REGION FOR YOUR PROJECT

CONSTRUCTION.

-THE REGIONS ARE DEPENDANT ON THE Ss VALUE DETERMINED IN STEP 3 -THE SS REGION DICTATES THE MAXIMUM DEAD LOAD PERMITTED ON THE FRAME (SEE TABLE TO RIGHT)

STEP 5: IDENTIFY THE ROOF DEAD LOAD FOR YOUR PROJECT -THE ROOF DECK DEAD LOAD WILL ALWAYS BE INCLUDED -THE COLLATERAL LOAD REPRESENTS ADDITIONAL LOAD THAT CAN BE SUPPORTED BY THE FRAME -BE SURE THE TOTAL ROOF DEAD LOAD FOR YOUR PROJECT IS LESS THAN OR EQUAL TO THE MAX

-MARK UP PC DRAWINGS WITH SIZE AND LOCATION OF CUTOUTS BEFORE SUBMITTING TO DSA

DEAD LOAD SHOWN IN STEP 4 FOR YOUR SE VALUE -Sds value used in calculation is the capped Sds (see design criteria) STEP 6: IDENTIFY THE FOUNDATION REQUIREMENTS FOR YOUR PROJECT

-IDENTIFY SOIL CLASS FOR PROJECT SITE PER SITE SPECIFIC SOIL CONDITIONS -USE THIS TO SELECT CORRECT FOUNDATION SIZE ON FOUNDATION SHEET STEP 7: SELECT MISCELLANEOUS OPTIONS FOR YOUR PROJECT -MAXIMUM CLEAR HEIGHT IS 12'-0"; (SEE "ARCHITECTURAL VIEWS" SHEET FOR REFERENCE)

STEP 8: SELECT APPLICABLE SHEET INDEX FOR YOUR PROJECT -REFERENCE THE BASE FRAME (STEP 1) AND THE ROOF PANEL TYPE (STEP 2) -IDENTIFY THE APPLICABLE SHEÈT INDEX

STEP 9: INCLUDE APPLICABLE SHEETS WITH YOUR DSA SUBMITTAL -INCLUDE 'MISC DESIGN OPTIONS' SHEET FOR PROJECTS WITHOUT ELECTRICAL CUTOUTS OR GUTTERS

NOTICE OF DISCLAIMER FOR STRUCTURAL ENGINEERING RESPONSIBILITY

- 1. PER TITLE 24, PART 1, SECTION 4-316(e) OF THE CALIFORNIA CODE OF REGULATIONS, THIS NOTICE SHALL
- BE GIVEN TO DSA PRIOR TO THE APPROVAL OF PLANS AND SPECIFICATIONS. 2. FOR THE SITE SPECIFIC PROJECT, J. R. MILLER & ASSOCIATES IS NOT THE DESIGN PROFESSIONAL IN
- GENERAL RESPONSIBLE CHARGE. 3. FOR THE SITE SPECIFIC PROJECT, J.R. MILLER & ASSOCIATES' RESPONSIBILITY IS LIMITED TO THE PREPARATION OF THE PLANS AND SPECIFICATIONS FOR THE SHELTERS OF THIS PC ONLY.
- 4. STRUCTURAL OBSERVATION OF CONSTRUCTION IS SPECIFICALLY EXCLUDED FROM J.R. MILLER & ASSOCIATES' RESPONSIBILITY FOR THE SITE SPECIFIC PROJECT. 5. ALL CONSTRUCTION ACTIVITIES RELATED TO STRUCTURAL ENGINEERING SHALL BE DELEGATED TO A QUALIFIED

ENGINEER BY THE DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE CHARGE. THESE ACTIVITIES INCLUDE,

BUT ARE NOT LIMITED TO. APPROVAL OF INSPECTOR QUALIFICATIONS. STRUCTURAL OBSERVATION OF CONSTRUCTION, REVIEW OF INSPECTION REPORTS, AND SIGNING OFF OF THE VERIFIED REPORT FOR COMPLETED WORK. 6. J.R. MILLER & ASSOCIATES WILL BE RESPONSIBLE FOR RESPONDING TO QUESTIONS PERTAINING TO THE PLANS

AND SPECIFICATIONS FOR THE SHELTERS OF THIS PC WHICH ARISE DURING PLAN REVIEW AND

- 1. ALL WELDING SHALL COMPLY WITH AWS D1.1 SPECIFICATIONS AND SHALL BE DONE BY AWS QUALIFIED WELDERS CERTIFIED FOR THE TYPE OF WELDING TO BE PERFORMED AS REQUIRED BY DSA.
- 2. ALL WELDING SHALL BE DONE BY GAS METAL ARC PROCESS WITH E70XX ELECTRODES. FLUX CORE ARC WELD

3. ALL WELDING SHALL BE DONE IN THE SHOP WITH REQUIRED INSPECTION, PRE-APPROVED BY DSA, TO ENSURE

- SHALL CONFORM TO CHARPY NOTCH TOUGHNESS RATING OF 20 ft-16 @ (0° F).
- PROPER MATERIAL ID AND WELDING.
- 4. WELD FILLER METAL MANUFACTURER SHALL PROVIDE WRITTEN CERTIFICATION OF COMPLIANCE WITH CODE AND SPECIFIC ATIONS.

- 1. ALL BOLTS SHOWN ON THESE DRAWINGS ARE ASTM F3125 GRADE A325 HIGH STRENGTH BOLTS (UNO), WITH THE NUTS CONFORMING TO ASTM A-563.
- 2. HIGH STRENGTH BOLTS SHALL BE VERIFIED AND INSPECTED PER CBC 1705A2.1
- 3. BEFORE ERECTING THE FRAME, VERIFY ALL BOLTS AND NUTS ARE CLEAN OF DEBRIS AND BURRS INCLUDING THE HARDWARE ALREADY FASTENED INSIDE THE MEMBERS. CHASING SOME OF THE BOLTS AND NUTS MAY BE
- 4. HARDENED STEEL WASHERS SHALL CONFORM TO ASTM F-436.
- 5. THE BOLTING INSTALLATION REQUIREMENTS OUTLINED BELOW ARE CRITICAL TO THE STRUCTURE'S DESIGN AND PERFORMANCE. THE INSTALLER IS REQUIRED TO COORDINATE THIS PHASE OF CONSTRUCTION WITH THE SPECIAL BOLTING INSPECTOR AND THE INSPECTOR OF RECORD PRIOR TO THE ERECTION OF THE FRAME. ALL BOLTS SHALL BE INSTALLED AND INSPECTED PER THE APPLICABLE VERSION OF AISC'S "SPECIFICATION FOR STRUCTURAL JOINTS
- USING HIGH-STRENGTH BOLTS", CBC 1705A.2.1; AISC 341-16 J7; AISC 360-16 N5.6. A)PRETENSIONED JOINTS MUST BE INSTALLED AND INSPECTED TO MEET ONE OF THE FOLLOWING REQUIREMENTS:
 - 1. TURN-OF-NUT PRETENSIONING
 - 2. CALIBRATED WRENCH PRETENSIONING 3. DIRECT-TENSION-INDICATOR PRETENSIONING (CONTRACTOR RESPONSIBLE FOR PURCHASE OF

- 1. ALLOWABLE SOIL PRESSURES ASSUME CLASS 5 SOIL CLASSIFICATION PER CBC TABLE 1806A, UNLESS NOTED
- 2. PER CBC SECTION 1803A.2, GEOTECHNICAL REPORTS ARE NOT REQUIRED FOR ONE-STORY LIGHT-STEEL FRAME BUILDINGS OF TYPE II CONSTRUCTION AND 4,000 SQUARE FOOT OR LESS IN FLOOR AREA AND NOT LOCATED WITHIN EARTHQUAKE FAULT ZONESOR SIESMIC HAZARD ZONES AS SHOWN ON THE MOST RECENT MAPS PUBLISHED BY THE CGS. ALLOWABLE FOUNDATION AND LATERAL SOIL PRESSURE VALUES MAY BE DETERMINED FROM TABLE 1806A.2.
- 3. FILL AND BACKFILL SHALL BE COMPACTED TO 95% OF MAX. DENSITY IN ACCORDANCE WITH ASTM TEST METHOD D-1557 OR AS RECOMMENDED BY THE GEO-TECH ENGINEER. FLOODING NOT PERMITTED.
- 4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SHORING, ETC. NECESSARY TO SUPPORT CUT AND/OR FILL BANKS DURING EXCAVATION, AND FORMING AND PLACEMENT OF CONCRETE.
- 5. MINIMUM SETBACK FROM TOE OF SLOPE ON AN ASCENDING SLOPE SHALL BE 15 FEET AND MINIMUM SETBACK
- FROM TOE OF SLOPE ON A DESCENDING SLOPE SHALL BE 40 FEET 6. PER CBC SECTION 1803A.6, GEOHAZARD REPORTS ARE NOT REQUIRED FOR ONE-STORY LIGHT-STEEL FRAME BUILDINGS OF TYPE II CONSTRUCTION AND 4,000 SQUARE FOOT OR LESS IN FLOOR AREA AND NOT LOCATED WITHIN EARTHQUAKE FAULT ZONESOR SIESMIC HAZARD ZONES AS SHOWN ON THE MOST RECENT MAPS PUBLISHED BY THE CGS.
- 7. GEOHAZRD REPORTS ARE TO COMPLY WITH DSA IR A-4 PER IR-7 SECTION 1.8
- 8. SITE SPECIFIC GEOTECHNICAL REPORT IS REQUIRED AT THE TIME OF SITE APPLICATION IS USING OTHER THAN
- 9. LATERAL BEARING HAS BEEN INCREASED PER CBC 1806A.3.4 & HAS BEEN DESIGNED FOR P-DELTA EFFECTS

1. MIX DESIGN REQUIREMENTS: (NORMAL WEIGHT CONCRETE)

STRENGTH Pc (28 DAYS)	W/C RATIO (NON—AIR ENTRAINED)	W/C RATIO (AIR ENTRAINED)	SLUMP (±1")	UNIT WEIGHT (NORMAL WEIGHT)
4500 PSI	0.44	0.35	150 PCF	
2 CONCRETE MIX DESIG	27.		5 5 5 5 5 5	IDU PCF

ENTRAINMENT FOR THESE CATEGORIES SHALL BE AS FOLLOWS: F0-0, F1-4.5, F2-6 3. AGGREGATES SHALL CONFORM TO THE ASTM C-33 WITH PROVEN SHRINKAGE CHARACTERISTICS OF LESS THAN 0.005.

SCHOOL DISTRICT:

- MAX AGGREGATE SIZE = 1". 4. CEMENT SHALL CONFORM TO ASTM C-150 (TYPE V) UNLESS NOTED OTHERWISE ON THE DRAWINGS.
- 5. CONCRETE SHALL BE MAINTAINED IN A MOIST CONDITION FOR A MINIMUM OF FIVE DAYS AFTER PLACEMENT.
- ALTERNATE METHODS WILL BE APPROVED IF SATISFACTORY PERFORMANCE CAN BE ASSURED. 6. CONCRETE SHALL NOT FREE FALL MORE THAN FIVE FEET.
- 7. CONCRETE DURABILITY SHALL BE PER CBC 1904A.1 & ACI 318-14 CHAPTER 19.
- 8. CONCRETE SHALL BE TESTED PER CBC 1903A, TABLE 1705A.3. AND ACI 318-14 SECTION 26.12.

STEP 10: IDENTIFY PROJECT NAME AND SCHOOL DISTRICT

CONSTRUCTION NOTES

TESTS AND INSPECTIONS FOR THE PROJECT.

SHALL COMPLY WITH ALL LOCAL ORDINANCES

PROJECT NAME:

			FRAME	DIMENSION	 S	
-			SUG	GESTED		OTHER
STE	FRAME WIDTH	[] 20'	3 0'	[] 40'		[] (40' MAX)
	FRAME LENGTH	[] 44'	М 64'	[]84'	[] 104'	[] (NO MAX)

7	ROOF PANEL								
STEP	ROOF PANEL TYPE	⋈ м [] G [] S							
3 E P	P	ROJECT SITE — Ss ACCELERATION (g)							
ST		<u>0.642</u>							

		Ss REGION		
			Ss REGIONS	MAX DEAD LOAD
4		X	0 < Ss <= 2.14	5 PSF
STEP			2.14 < Ss <= 2.50	5 PSF
S	DESCRIPTION		2.50 < Ss <= 2.75	5 PSF
			2.75 < Ss <= 3.00	4 PSF
			Ss > 3.73 MAX	3 PSF

		TOTAL ROOF DEAD LOA	AD
		DEAD LOAD	EXAMPLES
٦ 5	ROOF DECK	<u>1.1</u> PSF	M=1.1PSF; G=1.2PSF;S=1.3PSF (SEE STEP 2)
STE	COLLATERAL	<u>0</u> PSF	LIGHTING, ETC
	TOTAL	<u>1.1</u> PSF	ADD ROOF DECK AND COLLATERAL LOADS (MAX 5 PSF)

1. A DSA-CERTIFIED CLASS 3 PROJECT INSPECTOR IS REQUIRED FOR THIS PROJECT.

BEFORE PROCEEDING WITH THE WORK, (SECTION 4-317(c), PART 1, TITLE 24, CCR)

2. CHANGES TO THE APPROVED DRAWINGS AND SPECIFICATIONS SHALL BE MADE BY ADDENDA OR CONSTRUCTION CHANGE

CONTINUOUS INSPECTION OF WORK, THE DUTIES OF THE INSPECTOR ARE DEFINED IN SECTION 4-342, PART 1, TITLE 24, CCR.

RECONSTRUCTION IS TO BE IN ACCORDANCE WITH TITLE 24, CCR. SHOULD ANY EXISTING CONDITIONS SUCH AS DETERIORATION

FINISHED WORK WILL NOT COMPLY WITH TITLE 24, CCR, A CONSTRUCTION CHANGE DOCUMENT (CCD), OR A SEPARATE SET OF PLANS AND SPECIFICATIONS, DETAILING AND SPECIFYING THE REQUIRED WORK SHALL BE SUBMITTED TO AND APPROVED BY DSA

OR NON-COMPLYING CONSTRUCTION BE DISCOVERED WHICH IS NOT COVERED BY THE CONTRACT DOCUMENTS WHEREIN THE

6. GRADING PLANS, DRAINAGE IMPROVEMENTS, ROAD AND ACCESS REQUIREMENTS AND ENVIRONMENTAL HEALTH CONSIDERATIONS

4. A DSA ACCEPTED TESTING LABORATORY DIRECTLY EMPLOYED BY THE DISTRICT (OWNER) SHALL CONDUCT ALL THE REQUIRED

5. THE INTENT OF THESE DRAWINGS AND SPECIFICATIONS ARE THAT ALL THE WORK OF THE ALTERATION. REHABILITATION OR

 $\,$ 3. A "DSA CERTIFIED" PROJECT INSPECTOR EMPLOYED BY THE DISTRICT (OWNER) AND APPROVED BY DSA SHALL PROVIDE

DOCUMENT (CCD) APPROVED BY DSA, AS REQUIRED BY SECTION 4-338, PART 1, TITLE 24, CCR.

REINFORCING STEEL:

- 1. REINFORCING STEEL SHALL BE DEFORMED STEEL CONFORMING TO THE REQUIREMENTS OF ASTM A-615,

 - GR 60: (#4 BARS AND LARGER) GR 40: (#3 BARS)
- DETAILING, FABRICATION, AND ERECTION OF REINFORCING BARS SHALL CONFORM TO THE ACL "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCING CONCRETE STRUCTURES."
- 3. MIN. COVER FOR CAST-IN-PLACE CONCRETE SHALL BE AS FOLLOWS:
- A. CAST AGAINST EARTH B. CAST AGAINST FORM BELOW GRADE
- C. FORMED SLABS (#11 BAR & SMALLER)......3/4"
- D. SLABS ON GRADE (FROM TOP OF SLAB)......1" 4. BARS SHALL BE CLEAN OF RUST, GREASE OR OTHER MATERIAL LIKELY TO IMPAIR BOND.
- BENDS SHALL BE MADE COLD.

MINIMUM MISC ELLANEOUS

- REINFORCING SHALL BE LAP SPLICED PER ACI 318-14 SECTION 25.5. 6. PRIOR TO PLACING OF CONCRETE, REINFORCING STEEL AND EMBEDDED ITEMS SHALL BE WELL SECURED IN POSITION.
- 7. WELDING OF REINFORCING IS NOT ALLOWED.

8. REINFORCING STEEL SHALL BE INSPECTED PER CBC 1705A.3.

- POWDER-COAT FINISH SYSTEM: ALL BUILDINGS THAT HAVE A POWDER-COATED FINISH SHALL MEET THE FOLLOWING SPECIFICATIONS:
- 1. THE STEEL FRAME SHALL BE SHOT-BLASTED TO A NEAR WHITE CONDITION PER SSPC-10 SPECIFICATIONS. 2. THE STEEL SHALL BE WASHED IN A ZINC PHOSPHATE IN AN MINIMUM EIGHT STAGE ELECTRO DEPOSITION
- 3. IMMEDIATELY FOLLOWING PRE-TREATMENT THE STEEL SHALL BE TOTALLY IMMERSED IN A LIQUID EPOXY
- PRIMER(E-COAT) AND COATED TO A UNIFORM THICKNESS OF A MINIMUM OF 0.7 TO 0.9 MILS. THE E-COATING SHALL PROVIDE A MINIMUM OF 1000 HOURS OF SALT SPRAY CORROSION PROTECTION TO THE STEEL.
- 4. THE STEEL SHALL THEN HAVE A TGIC POLYESTER COLOR COAT APPLIED OVER THE E-COATED SURFACE.
- 5. THE COLOR COAT SHALL THEN HAVE A CLEAR TGIC COATING APPLIED TO SEAL IN THE COLOR COAT AND RESIST
- ULTRAVIOLET LIGHT, TO HELP PREVENT FADING. 6. THE FINISH THICKNESS OF THESE THREE APPLICATIONS SHALL BE A MINIMUM OF 8 TO 12 MILS
- 7. ALL CARBON STEEL MEMBERS (COLUMNS, BEAMS, PLATES, ETC.) NOT POWDER-COATED SHALL BE PAINTED WITH PRIME COAT PER THE "AISC CODE OF STANDARD PRACTICE" AND THE "AISC SPECIFICATION SECTION M3" (UNLESS NOTED

OTHERWISE).	
ABBREVIATIONS:	

ABBREVI	ATIONS:_		
ACI	AMERICAN CONCRETE INSTITUTE	MPH	MILES PER HOUR
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION	М	MULTI-RIB ROOF PANEL (MCELROY)
ASM	ASSEMBLY (INTERNAL REFERENCE)	NTS	NOT TO SCALE
ASTM	AMERICAN SOCIETY FOR TESTING AND MAT'LS	NO	NUMBER
AWS	AMERICAN WELDING SOCIETY	ОС	ON CENTER
CBC	CALIFORNIA BUILDING CODE	OSHA	OCCUPATIONAL HEALTH AND SAFETY ADMIN
CJP	COMPLETE JOINT PENETRATION	PCF	POUNDS PER CUBIC FOOT
CLR	CLEAR	PJ	PRETENSIONED JOINT
DEG	DEGREE	PLCS	PLACES
DIA	DIAMETER	PLT	PLATE
DIM	DIMENSION	PSF	POUNDS PER SQUARE FOOT
DSA	DIVISION OF THE STATE ARCHITECT	PSI	POUNDS PER SQUARE INCH
EQ	EQUAL	QTY	QUANTITY
FT	FEET	REF	REFERENC E
GA	GAGE	SQ	SQUARE
IN	INCHES	SS	STANDING SEAM ROOF PANEL (MCELROY)
KSI	KIPS PER SQUARE INCH	TYP	TYPIC AL
MAX	MAXIMUM	UNO	UNLESS NOTED OTHERWISE

USGS

U.S. GEOLOGIC AL SURVEY

DIV. OF THE STATE ARCHITECT APP: 04-120013 PC REVIEWED FOR SS I FLS I ACS I CG DATE: 08/06/2021

DRAWN BY

DATE

REV

REV DATE

ARCHITECTS ENGINEERS

2700 SATURN STIBREA, CA 92821

. 714.524.1870 | F. 714.524.1875

ANGEL

4/2/202

FOUNDATION REQUIREMENTS SOIL CLASS 5 (BEARING)-1500 PSF 📈 | SOIL CLASS 4 (BEARING)-2000 PSF [] SOIL CLASS 3 (BEARING)-3000 PSF [SOIL CLASS 5 (LATERAL BEARING)-100 PSF | SOIL CLASS 4 (LATERAL BEARING)-150 PSF | SOIL CLASS 3 (LATERAL BEARING)-200 PSF

MISC ELLANEOUS DESIGN OPTIONS CLEAR HEIGHT []8' 🔀 10' []12' [] ' (12' MAX) ELECTRICAL CUTOUTS **⋈** YES [] NO GUTTERS 🔀 YES [] NO

				SHEET	INDEX						
	BASE FRAME		RG 20				RG 30			RG 40	
	ROOF PANEL TYPE	М	G	S		М	G	S	М	G	S
	SELECT ONE	[]	[]	[]		[]	[]	[X]	[]	[]	[]
	GENERAL NOTES	LS1.0	LS1.0	LS1.0		LS1.0	LS1.0	LS1.0	LS1.0	LS1.0	LS1.0
	DSA 103 EXAMPLE	LS1.1	LS1.1	LS1.1		LS1.1	LS1.1	LS1.1	LS1.1	LS1.1	LS1.1
ω _	FOUNDATION PLAN	LS2.0	LS2.0	LS2.0		LS3.0	LS3.0	LS3.0	LS4.0	LS4.0	LS4.0
STEP	FRAMING PLAN	LS2.1	LS2.1	LS2.1		LS3.1	LS3.1	LS3.1	LS4.1	LS4.1	LS4.1
	FRAME CONNECTION DETAILS	LS2.1	LS2.1	LS2.1		LS3.1	LS3.1	LS3.1	LS4.2	LS4.2	LS4.2
	ROOFING LAYOUT & DETAILS	LS2.2	LS2.3	LS2.4		LS3.2	LS3.3	LS3.4	LS4.3	LS4.4	LS4.5
	MISC DESIGN OPTIONS	LS5.0	LS5.0	LS5.0		LS5.0	LS5.0	LS5.0	LS5.0	LS5.0	LS5.0

COPYRIGHT 2004, ICON SHELTER SYSTEMS, INC. 1455 LINCOLN AVE HOLLAND MI, 49423

PRE-CHECK (PC) DOCUMENT Code: 2019 CBC A separate project application for construction is required.

ISTINCTIVE STEEL SHELTERS

616.396.0919 800.748.0985

PRINTED ON:

616.396.0944 FX

SA 103-19: LISTING	OF STRUCTURAL TESTS & SPECIAL	INSPECTIONS, 2019 CBC		IG OF STRUCTURAL TES	STS & SPECIAL II	ISPECTIO		DSA 103-19: LISTI Application Number:	NG OF STRUCTURAL TESTS School Name:	5 & SPECIAL INSP	School District:	DSA 10
Application Number: 04-000000	School Name: ICON Shelter Systems	School District: PC Submittal	Application Number: 04-000000 DSA File Number:	School Name: ICON Shelter Systems Increment Number:			School District: PC Submittal Date Created:	04-000000 DSA File Number:	ICON Shelter Systems Increment Number:		PC Submittal Date Created:	04-00000 DSA File
A File Number:	Increment Number:	Date Created: 2021-07-14 05:50:33					2021-07-14 05:50:33				2021-07-14 05:50:33	
												5. Te
	2	MA ORO	Geotechnical Reports:	: Project has a geotechn ⁵	ical report, or CDs i	ndicate soil	s special inspection is required by GE	C. Compaction test	ing.	Test LOI	(Refer to specific items identified in the Appendix for exemptions where	□ a.
IMPORTANT: Th		119 CBC tests and some of the special inspections required for the project.	1. GENERAL:	<u> </u>	Table 1705A.6						soils testing may be conducted under the supervision of a geotechnical engineer or LOR's engineering manager. In such cases, the LOR's form DSA 291 shall satisfy the soil test reporting requirements for the exempt items.)	\ \ \
Generally, the structur	ral tests and special inspections noted on	his form are those that will be performed by the Geotechnical Engineer complete test and inspection program must be performed as detailed	Test or Special Inspe	ection	Type P. B	erformed Co y	de References and Notes				251 shall satisfy the 301 test reporting requirements for the exempt items,	b. de
on the DSA approved	documents. The appendix at the bottom	f this form identifies work NOT subject to DSA requirements for special ible for providing inspection of all facets of construction, including but		ared properly prior to placement o	Periodic of		by geotechnical engineer or his or her qualified representative. ee Appendix for exemptions.)	4. CAST-IN-PLACE	DEEP FOUNDATIONS (PIERS):	Table 1705A.8		C. ur
not limited to, special in	nspections not listed on this form such as	tructural wood framing, high-load wood diaphragms, cold-formed steel ents, etc., per Title 24, Part 2, Chapter 17A (2019 CBC).	Foundation excavation	excavations for foundations. tions are extended to proper dept	oth			Test or Special Ins	pection	Type Perfor	med Code References and Notes	□ d.
		in this document are from the CBC, or California Building Code.	and have reached pro • Materials below foot design bearing capaci	tings are adequate to achieve the	e			 ☑ a. Inspect drilling of and accurate record 	perations and maintain complete	Continuous GE	* * By geotechnical engineer or his or her qualified representative. (See Appendix for exemptions.)	e.
TO COLUMNS	ichnica section and table references round	The this document are from the ebe, of camornia building code.		·				b. Verify pier location	ons, diameters, plumbness, bell cable), lengths and embedment into	Continuous GE		6. Te
1. TYPE		2. PERFORMED BY	2. SOIL COMPACTION Test or Special Inspe		Table 1705A.6	erformed Co	de References and Notes		ole); record concrete or grout		(See Appendix for exemptions.)	
ntinuous – Indicates that a	a continuous special inspection is	GE – Indicates that the special inspection shall be performed by a registered geotechnical engineer or his or her authorized representative.	□ a. Perform classification	ion and testing of fill materials.	B Test	y LOR* * U	Under the supervision of the geotechnical engineer.	C. Confirm adequat	e end strata bearing capacity.	Continuous GE	* By geotechnical engineer or his or her qualified representative. (See Appendix for exemptions.)	
uired		LOR – Indicates that the test or special inspection shall be performed by a testing		er materials, densities and	Continuous	GE* * 6	By geotechnical engineer or his or her qualified representative. (Refer to ecific items identified in the Appendix for exemptions where soils SI and	d. Concrete piers.		Provide tests and inspe	ections per CONCRETE section below.	b.
iodic – Indicates that a per	riodic special inspection is required	laboratory accepted in the DSA Laboratory Evaluation and Acceptance (LEA) Program. See CAC Section 4-335.	during placement of f	es, placement and compaction fill.		te	sting may be conducted under the supervision of a geotechnical gineer or LOR's engineering manager. In such cases, the LOR's form DSA					□ C.
		PI – Indicates that the special inspection may be performed by a project inspector when specifically approved by DSA.				29 ite	1 shall satisfy the soil SI and test reporting requirements for the exempt ms.)					
– Indicates that a test is re	required	SI – Indicates that the special inspection shall be performed by an appropriately						DGS DSA 103-19 (Revised 07/	(16/2020)			
		qualified/approved special inspector.						DIVISION OF THE STATE ARCH	НТЕСТ	DEPARTMENT OF GENE Page 3 of		IIA DIVISION O
SA 103-19 (Revised 07/16/20)	,		DGS DSA 103-19 (Revised 07/16/ DIVISION OF THE STATE ARCHITE	•	DEPARTMENT OF	GENERAL SERVI	CES STATE OF CALIFORNIA					
ON OF THE STATE ARCHITECT		OF GENERAL SERVICES STATE OF CALIFORNIA age 1 of 11				2 of 11						
					TESTS & SPECIAL	INSPECTION	DNS (Concrete), 2019 CBC		ING OF STRUCTURAL TEST	S & SPECIAL INSF	PECTIONS (Concrete), 2019 CBC	DSA 10 Table 1705
103-19: LISTING cation Number:	OF STRUCTURAL TESTS & SPECIA School Name: ICON Shelter Systems	L INSPECTIONS (SOILS), 2019 CBC School District: PC Submittal	Table 1705A.3; ACI 318-1 Application Number: 04-00000	-14 Sections 26.12 & 26.13 School Name: ICON Shelter System	ms		School District: PC Submittal	Application Number: 04-000000 DSA File Number:	School Name: ICON Shelter Systems Increment Number:		School District: PC Submittal Date Created:	Application 04-000000 DSA File N
ile Number:	ICON Shelter Systems Increment Number:	Date Created: 2021-07-14 05:50:33	DSA File Number:	Increment Number			Date Created: 2021-07-14 05:50:33	DSA FIIE NUMBER:	nici ement Number:		Date Created: 2021-07-14 05:50:33	
7. CAST-IN-PLACE CON	ICRE		17. STRUCTURAL Material Verification and	L STEEL, COLD-FORMED STEEL A	AND ALUMINUM USED F	OR STRUCTU	AL PURPO		gth bolts, nuts and washers.	Test LO	DR Table 1705A.2.1 Item 1c, 2213A.1; RCSC 2014 Section 7.2; DSA IR 17-8.	19.
Test or Special Inspecti		Performed By Code References and Notes	Test or Special Ins		Туре	Performed (Code References and Notes	Inspection of High-Stre				Tes
rial Verification and Test a. Verify use of required	3	SI Table 1705A.3 Item 5, 1910A.1.		ation of all materials and: ndicate material properties that co	Periodic		Table 1705A.2.1 Item 3a–3c. 2202A.1; AISI S100-16 Section A3.1 & A3.2, AISI S240-15 Section A3 & A5, AISI S220-15 Sections A4 & A6. * By special	C. Bearing-type ("s		Periodic S	Table 1705A.2.1 Item 2a, 1705A.2.6, 2204A.2; AISC 360-16 J3.1, J3.2, M2.5 & N5.6; RCSC 2014 Section 9.1; DSA IR 17-9.	fille
b. Identifiy, sample, and	test reinforcing steel. Test	LOR 1910A.2; ACI 318-14 Section 26.6.1.2; DSA IR 17-10. (See Appendix for exemptions.)	with requirements. • Material sizes, typ		Simple		nspector or qualified technician when performed off-site.	d. Pretensioned a	nd slip-critical connections.	*	Table 1705A.2.1 Items 2b & 2c, 1705A.2.6, 2204A.2; AISC 360-16 J3.1, J3.2, M2.5 & N5.6; RCSC 2014 Sections 9.2 & 9.3; DSA IR 17-9. * "Continuous" or "Periodic" depends on the tightening method used.	dec
c. During concrete place for strength tests, perfor	ement, fabricate specimens Test rm slump and air content	LOR Table 1705 A.3 Item 6; ACI 318-14 Sections 26.5 & 26.12.	requirements. b. Test unidentified	ed materials	Test	LOR 2	2202A.1.					
tests, and determine the concrete.	·		c. Examine seam w	velds of HSS shapes	Periodic	SI	DSA IR 17-3.	19. WELDI		D1.2 for Aluminum; A	DSA.2.1 Items 4 & 5; AWS D1.1 and AWS D1.8 for structural steel; AWS WS D1.3 for cold-formed steel; AWS D1.4 for reinforcing steel; DSA IR 17-	☐ d. V
d. Test concrete (f'c).	Test	LOR 1905A.1.15; ACI 318-14 Section 26.12.	'	ıment steel fabrication per DSA-ap ıments.	pproved Periodic		lot applicable to cold-formed steel light-frame construction, except for russes (1705A.2.4).	Verification of Material	s, Equipment, Welders, etc.:	3 (See Appendix for e.	xemptions.)	
e. Batch plant inspection	n: See Notes	SI Default of 'Continuous' per 1705A.3.3. If approved by DSA, batch plant inspection may be reduced to 'Periodic' subject to requirements in	18. HIGH-STRENG				assisting.	Test or Special In	spection	Type Perfo By	rmed Code References and Notes	23.
		Section 1705A.3.3.1, or eliminated per 1705A.3.3.2. (See Appendix for exemptions.)		nd Testing of High-Strength Bolt	ts, Nuts and Washers:				r material identification markings per listed on the DSA-approved documents	Periodic S	DSA IR 17-3.	Tes
f. Welding of reinforcing	g steel. Provide spe	cial inspection per STEEL, Category 19.1(d) & (e) and/or 19.2(g) & (h) below.	Test or Special Ins	·	31	Ву	Code References and Notes		r material manufacturer's certificate of	Periodic S	DSA IR 17-3.	—
			certificates of com	ation markings and manufacturer's npliance conform to ASTM standar SA-approved documents.			Fable 1705A.2.1 Items 1a & 1b, 2202A.1; AISC 360-16 Section A3.3, J3.1, and N3.2; RCSC 2014 Section 1.5 & 2.1; DSA IR 17-8 & DSA IR 17-9.	c. Verify WPS, wel	der qualifications and equipment.	Periodic S	DSA IR 17-3.	□ b . T
											I	
SA 103-19 (Revised 07/16/202			DGS DSA 103-19 (Revised 07/	,	DEPARTMENT (DE GENERAL SER	VICES STATE OF CALIFORNIA	DGS DSA 103-19 (Revised 07 DIVISION OF THE STATE ARC	•	DEPARTMENT OF GEN	ERAL SERVICES STATE OF CALIFORN	DIVISION OF
N OF THE STATE ARCHITECT		OF GENERAL SERVICES STATE OF CALIFORN Page 5 of 11	IIA DIVISION OF THE STATE ARCT	AITECT		ge 6 of 11	VICES STATE OF CALIFORNIA			Page 7 of	11	
		L INSPECTIONS (Steel and Aluminum), 2019 CBC	DSA 103-19: LIST Application Number:	TING OF STRUCTURAL 1 School Name:	TESTS & SPECIAL	INSPECTI	ONS(SIGNATURE), 2019 CBC School District:	DSA 103-19: LIST Application Number: 04-000000	OF REQUIRED VERIFIED RE School Name: ICON Shelter Systems	PORTS, CBC 2019	School District: PC Submittal	
.1, Table 1705A.2.1; AIS tion Number: 00	SC 303-16, AISC 341-16, AISC 358-16, AISC 36 School Name: ICON Shelter Systems	0-16; AISI S100-16 School District: PC Submittal	04-000000 DSA File Number:	ICON Shelter Systen Increment Number			PC Submittal Date Created: 2021-07-14 05:50:33	DSA File Number:	Increment Number:		Date Created: 2021-07-14 05:50:33	
Number:	Increment Number:	Date Created: 2021-07-14 05:50:33						-				
			Name of Architect or Enginee	eer in general responsible charge:				1. Soils Testing and	Inspection: Geotechnical Verified Re	eport Form DSA 293		
3. ANCHOR BOLTS AND	O ANCHOR RODS:		Name of Structural Engineer	r (When structural design has been dele	egated):				g and Inspection: Laboratory Verified	•		
est or Special Inspectio		Performed Code References and Notes						^{3.} DSA 292			ndependently contracting SI, Special Inspection Verified Report Form	
, Anchor Bolts and Ancho	or Rods Tes t	LOR Sample and test anchor bolts and anchor rods not readily identifiable per procedures noted in DSA IR 17-11.	Signature of Architect or Stru	uctural Engineer:	Date:			4. High-Strength Bo Report Form DSA		y Verified Report Form	DSA 291, or, for independently contracting SI, Special Inspection Verified	
o. Threaded rod not used	for foundation anchorage. Test	LOR Sample and test threaded rods not readily identifiable per procedures noted in DSA IR 17-11.	Motor T. C. W. C. C.	SA alactronic 111	ification stems	n DCA	pends against using secured all streams or district to					
			NOTE: To facilitate DS.	.A electronic mark-ups and identif	uncation stamp applicatio	טא recomr, טאר,	nends against using secured electronic or digital signatures. DSA STAMP]				
							DOV OLVINIL	-				
								I .				
								J				
			DGS DSA 103 10 / Doctored 43					Des Des 100 10 to 1 to 1	16/2020)			
A 103-19 (Revised 07/16/2020 N OF THE STATE ARCHITECT		*OF GENERAL SERVICES STATE OF CALIFOI	DGS DSA 103-19 (Revised 07 DMSION OF THE STATE ARC			OF GENERAL SER	EVICES STATE OF CALIFORNIA	DGS DSA 103-19 (Revised 07/		DEPARTMENT OF GENER		
	DEPARTMEN	OF GENERAL SERVICES STATE OF CALIFOR	DIVISION OF THE STATE ARC			OF GENERAL SER age 10 of 11	VICES STATE OF CALIFORNIA			DEPARTMENT OF GENER Page 11 of		

Application Number: 04-000000 ICON Shelter Systems Increment Number: DSA File Number: Date Created: 5. RETAINING WALLS: Type Performed Code References and Notes Test or Special Inspection Continuous GE* 1705A.6.1. * By geotechnical engineer or his or her qualified representative. (See Section 2 above). a. Placement, compaction and inspection of backfill. □ b. Placement of soil reinforcement and/or drainage Continuous | GE* | * By geotechnical engineer or his or her qualified representative Continuous

By geotechnical engineer or his or her qualified representative See DSA IR 16-3. c. Segmental retaining walls; inspect placement of units, dowels, connectors, etc. d. Concrete retaining walls. Provide tests and inspections per CONCRETE section below. e. Masonry retaining walls. Provide tests and inspections per MASONRY section below. 6. OTHER SOIL Test or Special Inspection Type Performed Code References and Notes a. Soil Improvements Test GE* Submit a comprehensive report documenting final soil improvements constructed, construction observation and the results of the confirmation testing and analysis to CGS for final acceptance. * By geotechnical engineer or his or her qualified representative b. Inspection of Soil Improvements Continuous GE* * By geotechnical engineer or his or her qualified representative DGS DSA 103-19 (Revised 07/16/2020) INIA DIVISION OF THE STATE ARCHITECT DEPARTMENT OF GENERAL SERVICES STATE OF CALIFORNIA Page 4 of 11 DSA 103-19: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (Concrete), 2019 CBC Table 1705A.3; ACI 318-14 Sections 26.12 & 26.13
Application Number: School Name: PC Submittal ICON Shelter Systems Date Created: 2021-07-14 05:50:33 DSA File Number: Increment Number: 19.1 SHOP WELDING: Type Performed Code References and Notes Test or Special Inspection a. Inspect groove welds, multi-pass fillet welds, single pass | Continuous fillet welds > 5/16", plug and slot welds. applicable); DSA IR 17-3. ✓ b. Inspect single-pass fillet welds ≤ 5/16", floor and roof
 Periodic
 SI
 1705A.2.2, Table 1705A.2.1 Items 5a.5 & 5a.6; AISC 360-16 (and AISC) 341-16 as applicable); DSA IR 17-3. deck welds. c. Inspect welding of stairs and railing systems. Periodic SI 1705A.2.1; AISC 360-16 (and AISC 341-16 as applicable); AWS D1.1 & D1.3; d. Verification of reinforcing steel weldability Periodic SI 1705A.3.1; AWS D1.4; DSA IR 17-3. Verify carbon equivalent reported on other than ASTM A706. Continuous SI Table 1705A.2.1 Item 5b, 1705A.3.1, Table 1705A.3 Item 2, 1903A.8; AWS D1.4; DSA IR 17-3. e. Inspect welding of reinforcing steel. 23. ANCHOR BOLTS AND ANCHOR RODS: Type Performed Code References and Notes Test or Special Inspection ☑ a. Anchor Bolts and Anchor Rods LOR Sample and test anchor bolts and anchor rods not readily identifiable per procedures noted in DSA IR 17-11. LOR Sample and test threaded rods not readily identifiable per procedures noted in DSA IR 17-11. **b.** Threaded rod not used for foundation anchorage.

DEPARTMENT OF GENERAL SERVICES

STATE OF CALIFORNIA

DSA 103-19: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (SOILS), 2019 CBC

DIV. OF THE STATE ARCHITECT APP: 04-120013 PC REVIEWED FOR SS 🗹 FLS 🗹 ACS 🗹 CG 🗌 DATE: 08/06/2021

RH/DSA-PC

ANGEL

4/2/2021

ARCHITECTS ENGINEERS

2700 SATURN ST I BREA, CA 92821

T. 714.524.1870 | F. 714.524.1875 WWW.JRMA.COM

ICON STD

DRAWN BY

DATE

REV

REV DATE

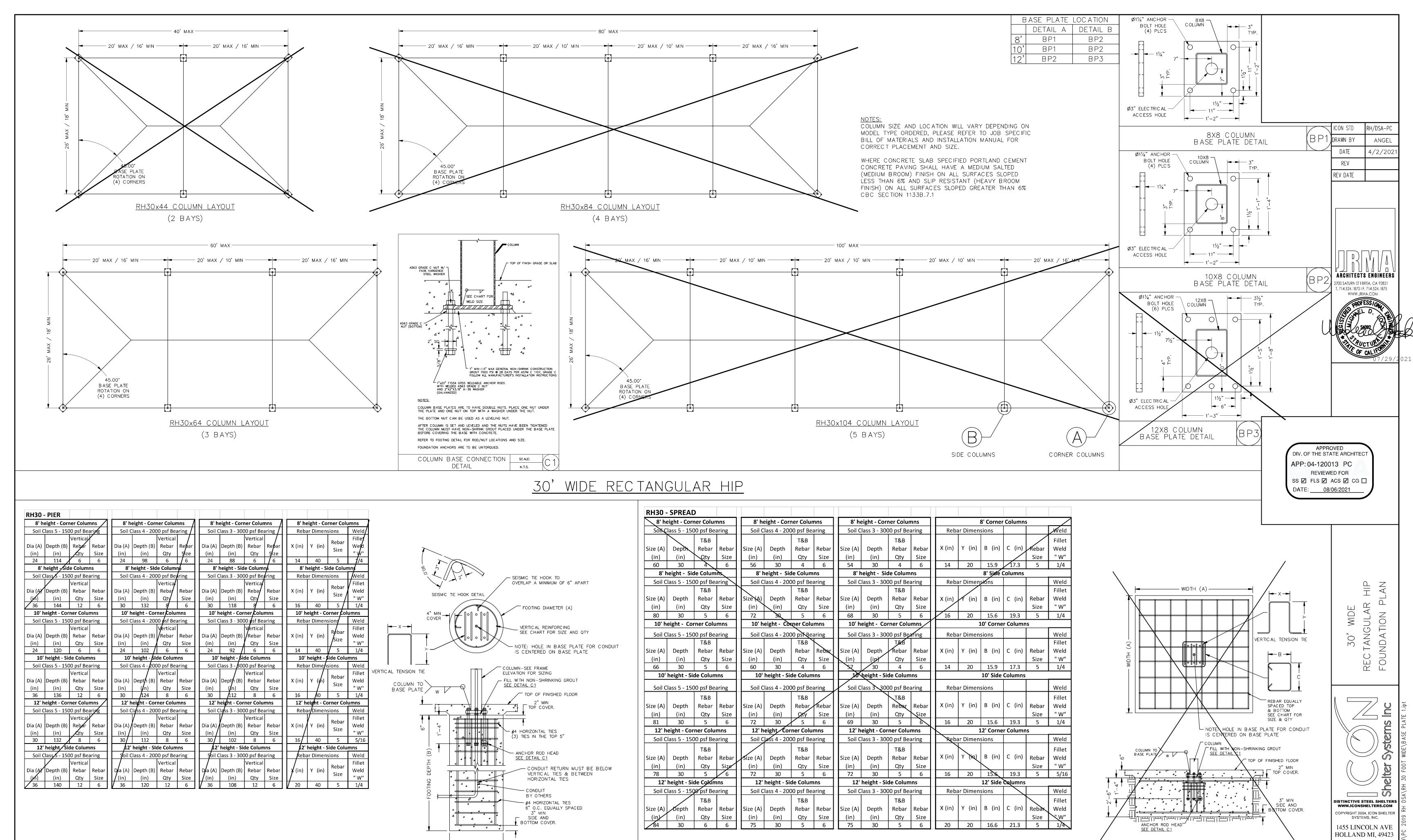
103 DSA

DISTINCTIVE STEEL SHELTERS COPYRIGHT 2004, ICON SHELTER 1455 LINCOLN AVE

HOLLAND MI, 49423

616.396.0919 800.748.0985 616.396.0944 FX

PRE-CHECK (PC) DOCUMENT Code: 2019 CBC A separate project application for construction is required.



FOOTING DIAMETER (A)

SEE DETAILS BP1. BP2 OR BP3 FOR ANCHOR BOLT PATTERNS

BP1 & BP2 ARE (4) BOLT PATTERN WHILE B3 IS A (6) BOLT

PRE-CHECK (PC) DOCUMENT
Code: 2019 CBC
A separate project application for construction is required.

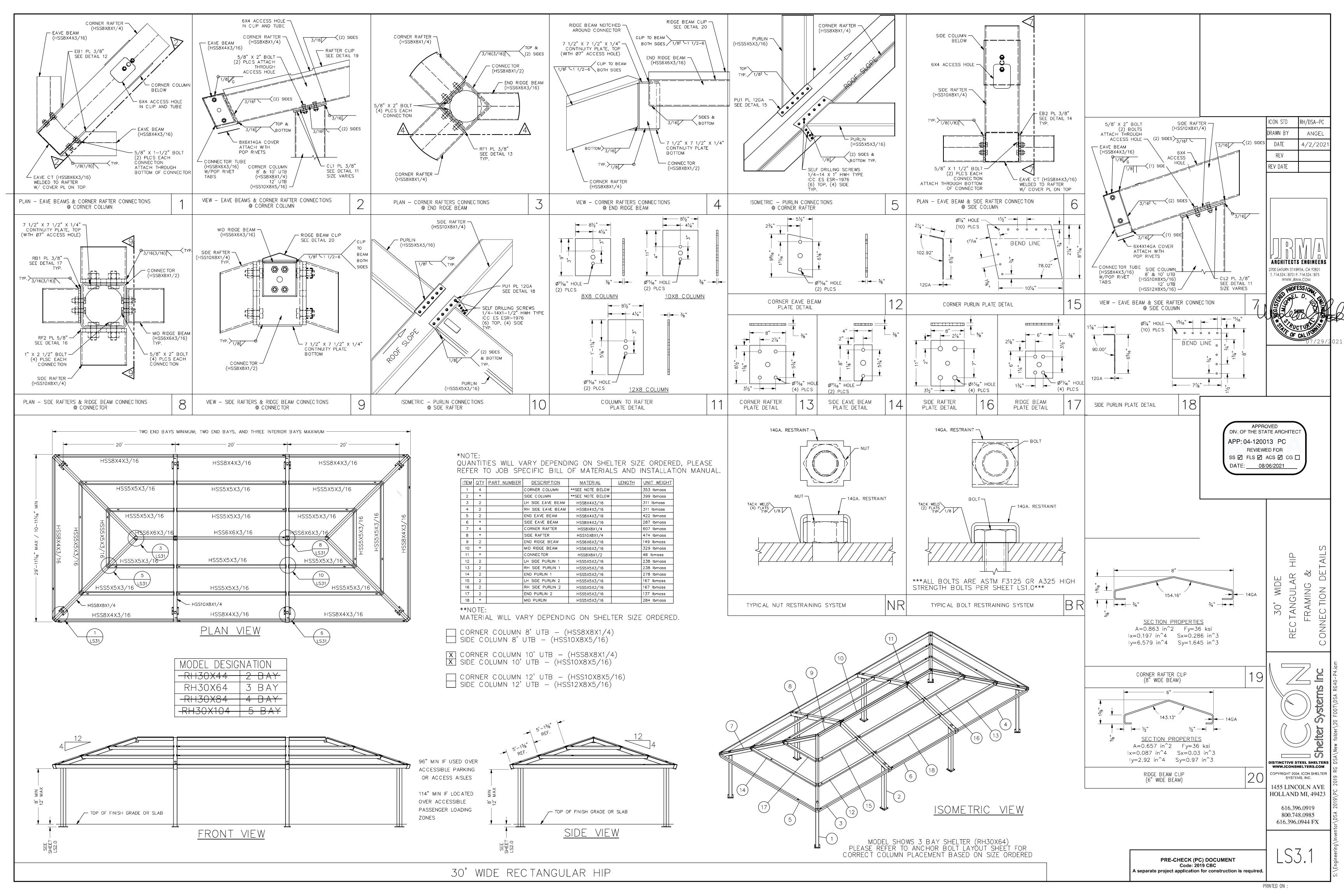
DETAILS BP1. BP2 OR BP3 FOR ANCHOR BOLT PAT**N**ERNS

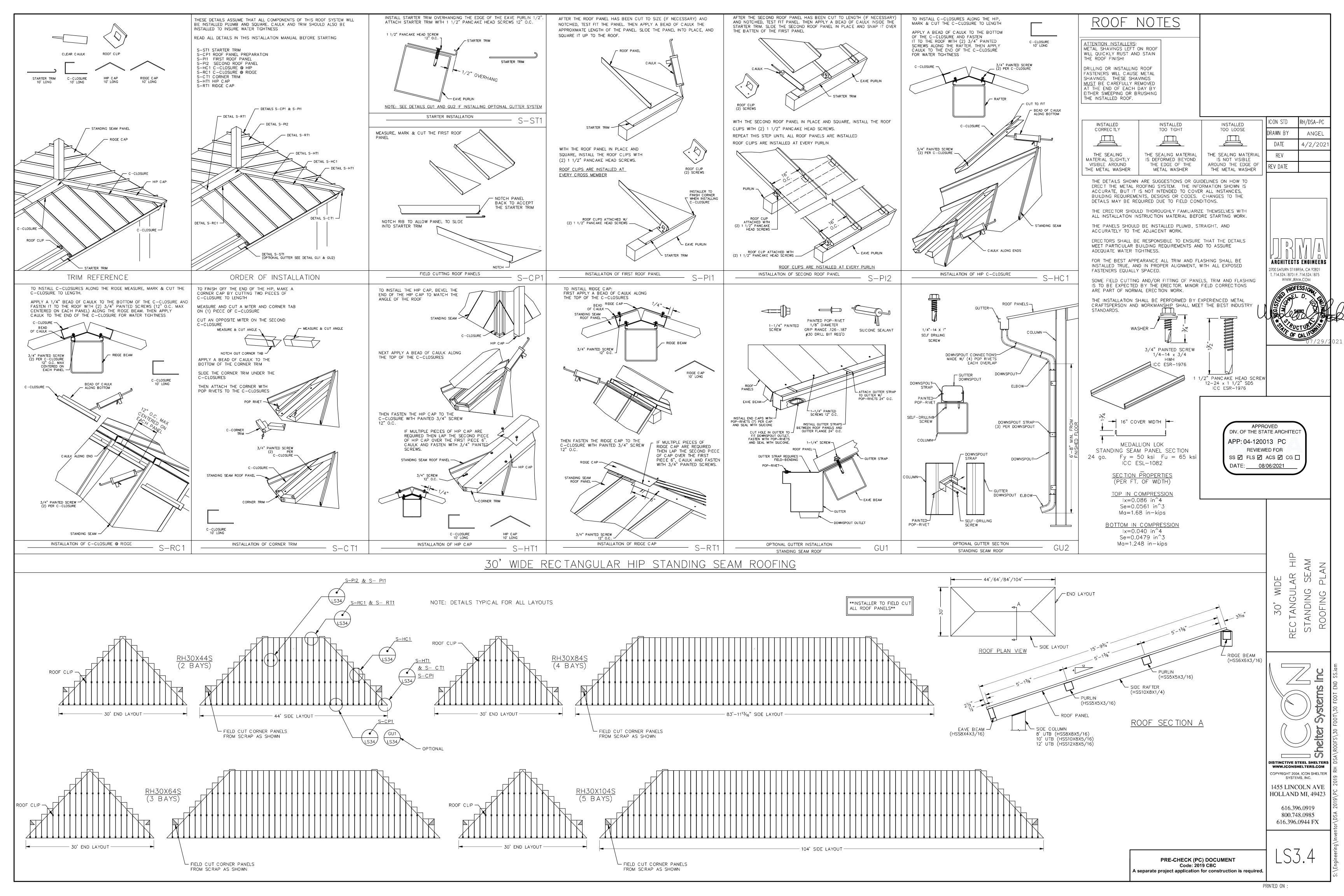
 $t ar{eta}$ P1 & BP2 are (4) bolt pattern while b3 is a (6) b δ L^

PRINTED ON :

616.396.0919 800.748.0985

616.396.0944 FX



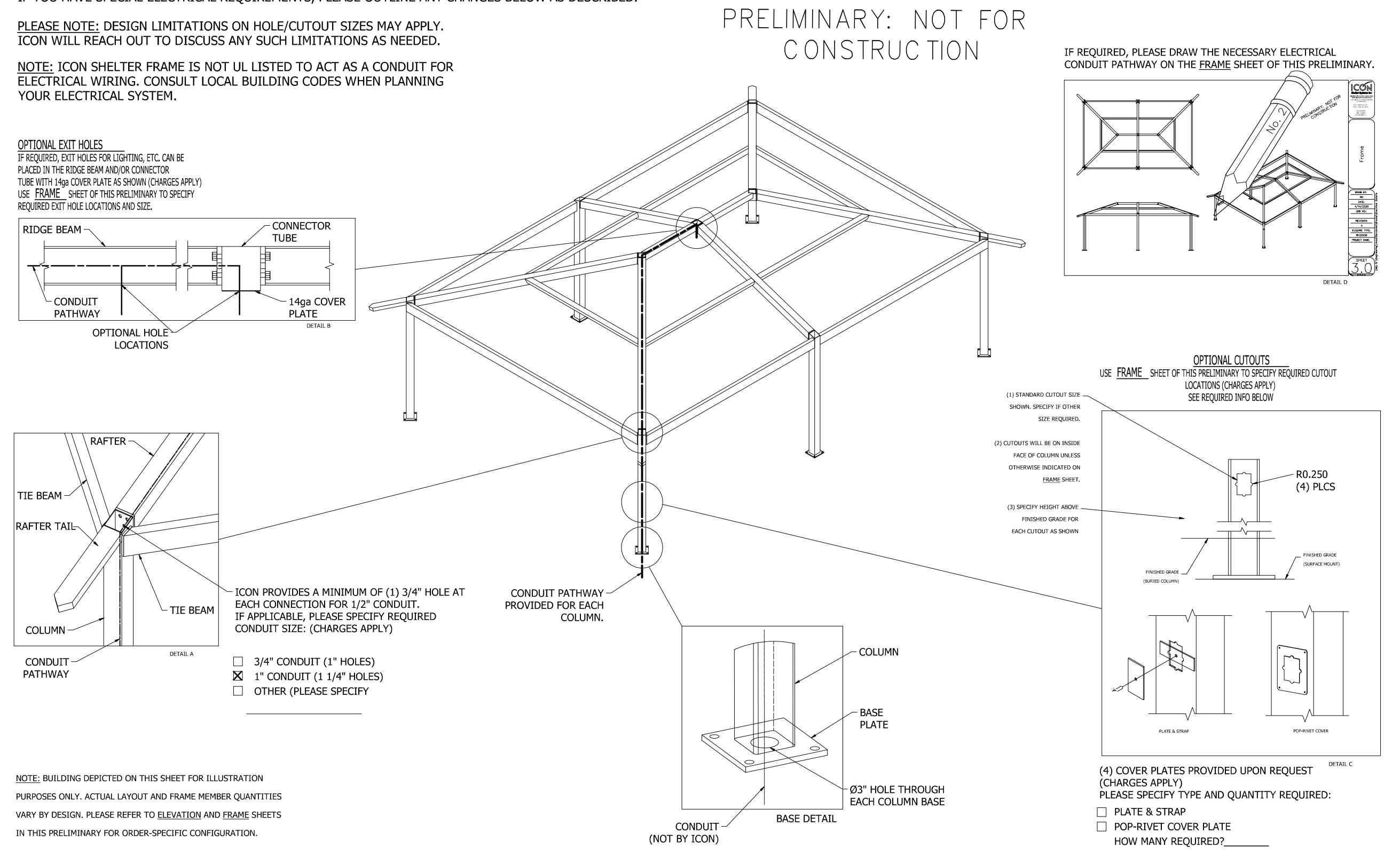


ELECTRICAL INFORMATION - RECTANGULAR HIP

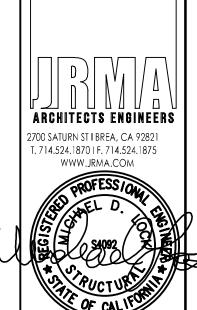
ICON'S STANDARD ELECTRICAL IS DESIGNED TO ACCOMMODATE Ø1/2" CONDUIT WITH A Ø3" INLET HOLE ON THE BOTTOM OF EACH COLUMN. THE CONDUIT PATHWAY RUNS THROUGH THE COLUMN, RAFTER, AND RIDGE BEAM THROUGH ALL BOLTED CONNECTIONS AS SHOWN. IF YOU HAVE SPECIAL ELECTRICAL REQUIREMENTS, PLEASE OUTLINE ANY CHANGES BELOW AS DESCRIBED.

2. ELECTRICAL EXIT HOLES (DETAIL B) 3. ELECTRICAL ACCESS & COVER PLATES (DETAIL C) 4. ELECTRICAL CONDUIT PATHWAY (DETAIL D)

1. CONDUIT HOLE SIZE (DETAIL A)

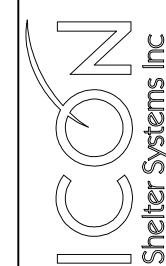


REV DATE



APPROVED DIV. OF THE STATE ARCHITEC APP: 04-120013 PC SS 🗹 FLS 🗹 ACS 🗹 CG 🗌

 \bigcirc



COPYRIGHT 2004, ICON SHELTER 1455 LINCOLN AVE HOLLAND MI, 49423 616.396.0919

> 800.748.0985 616.396.0944 FX

PRE-CHECK (PC) DOCUMENT Code: 2019 CBC

A separate project application for construction is required.